CLARE E. CONNORS 7936

Attorney General State of Hawai'i

STUART N. FUJIOKA
RYAN W. ROYLO
MELISSA J. KOLONIE
CARTER K. SIU

4223
6329
10209
7313

Deputy Attorneys General

235 S. Beretania Street, Room 304

Honolulu, Hawai'i 96813 Telephone: (808) 586-1255 Facsimile: (808) 586-1488

Email: Stuart.N.Fujioka@hawaii.gov

Ryan.W.Roylo@hawaii.gov Melissa.J.Kolonie@hawaii.gov Carter.K.Siu@hawaii.gov

Attorneys for Petitioner
DEPARTMENT OF EDUCATION,
STATE OF HAWAI'I



BEFORE THE LAND USE COMMISSION

OF THE STATE OF HAWAI'I

In the Matter of the Petition of

DEPARTMENT OF EDUCATION, STATE OF HAWAI'I,

To Amend the Agricultural Land Use District Boundaries into the Urban Land Use District for Approximately 77.2 acres of land at Kihei, Maui, Hawai'i, Maui Tax Map Key Nos. 2-2-02: 81 and 83. DOCKET NO. A11-794

SUPPLEMENTAL EXHIBIT TO PETITIONER DEPARTMENT OF EDUCATION, STATE OF HAWAII'S MOTION TO AMEND THE LAND USE COMMISSION'S FINDINGS OF FACT, CONCLUSIONS OF LAW AND DECISION AND ORDER FILED JULY 29, 2013; EXHIBIT "38"; CERTIFICATE OF SERVICE

SUPPLEMENTAL EXHIBIT TO PETITIONER DEPARTMENT OF EDUCATION, STATE OF HAWAII'S MOTION TO AMEND THE LAND USE COMMISSION'S FINDINGS OF FACT, CONCLUSIONS OF LAW AND DECISION AND ORDER FILED JULY 29, 2013

Through counsel and in further support of its Motion to Amend the Land Use Commission's Findings of Fact, Conclusions of Law and Decision and Order Filed July 29, 2013, Petitioner submits the following supplemental exhibit:

Exhibit 38 Traffic Impact Report for Kihei High School prepared September 2011, revised April 2012. (Attached as Exhibit G to the Final Environmental Impact Statement.)

DATED: Honolulu, Hawai'i, August 31, 2021.

/s/ Stuart N. Fujioka
STUART N. FUJIOKA
RYAN W. ROYLO
MELISSA J. KOLONIE
CARTER K. SIU
Deputy Attorneys General
Attorneys for Petitioner

DEPARTMENT OF EDUCATION, STATE OF HAWAI'I

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Traffic Impact Report



Prepared by: Wilson Okamoto Corporation

September 2011 Revised April 2012

TRAFFIC IMPACT REPORT

FOR

KIHEI HIGH SCHOOL

Prepared for:

Group 70 International, Inc. 925 Bethel Street, Fifth Floor Honolulu, Hawaii 96813

Prepared by:

Wilson Okamoto Corporation 1907 S. Beretania Street, Suite 400 Honolulu, Hawaii 96826 WOC Ref #7854-01

> September 2011 Revised April 2012

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Traffic Impact Report for Kihei High School

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I. INTRODUCTION

A. Purpose of Study

The purpose of this study is to identify and assess the traffic impacts resulting from the proposed Kihei High School in Kihei on the island of Maui. High school students who reside in Kihei currently attend Maui High School in Kahului or Baldwin High School in Wailuku. The proposed school will allow these and future high school students from Kihei to attend a school within their region.

B. Scope of Study

This report presents the findings and conclusions of the traffic study, the scope of which includes:

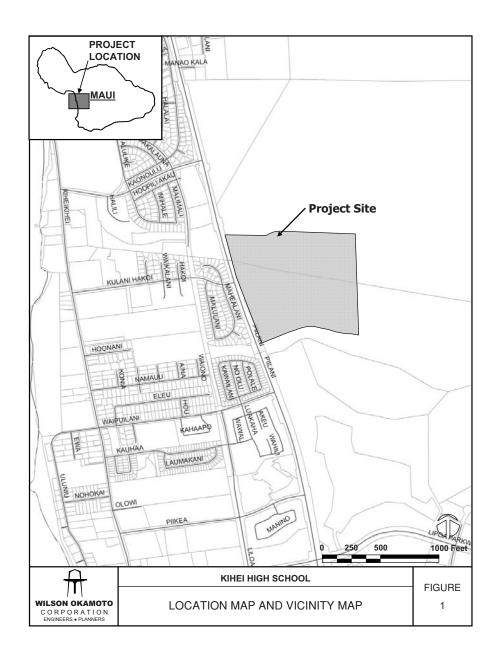
- Description of the proposed project.
- 2. Evaluation of existing roadway and traffic operations in the vicinity.
- Analysis of future roadway and traffic conditions without the proposed project.
- Analysis and development of trip generation characteristics for the proposed project.
- 5. Superimposing site-generated traffic over future traffic conditions.
- The identification and analysis of traffic impacts resulting from the proposed project.
- Recommendations of improvements, if appropriate, that would mitigate the traffic impacts resulting from the proposed project.

II. PROJECT DESCRIPTION

A. Location

The Kihei High School will be located on an over 70-acre site adjacent to Piilani Highway in Kihei on the island of Maui (see Figure 1). The proposed project site is further identified as Tax Map Keys (TMKs): 2-2-002: 15 (por) and 54 (por). The site is bounded by Piilani Highway to the west and agricultural lands to the north, east, and south. Vehicular access to the proposed school will be provided via a new access roadway off Piilani Highway at the intersection with Kulanihakoi Street.

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B. Project Characteristics

The State of Hawaii Department of Education (DOE) has plans to construct a new high school in Kihei adjacent to Piilani Highway. The proposed high school will include the following:

- Classrooms
- Library
- Auditorium
- Cafeteria
- · Administration building
- · Industrial arts building
- ROTC facility
- · PE and athletic buildings
- Gymnasium
- · Swimming pool
- · Football/track stadium
- · Tennis courts
- Grassed playfields
- · Track and field appurtenances
- · Softball and baseball fields
- · Parking areas

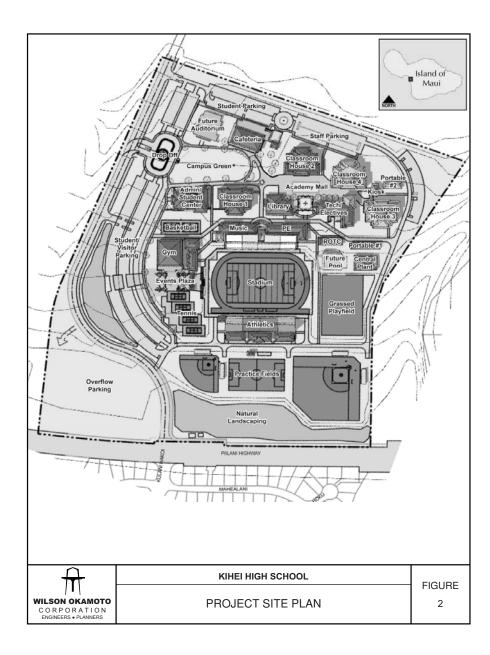
Access to the new high school will be provided via a new access roadway off Piilani Highway at the intersection with Kulanihakoi Street. The majority of the facilities at the high school are expected to be completed when the school opens in the Year 2015. The school is initially expected to accommodate approximately 800 students with a full enrollment of 1,650 students expected by the Year 2025. Figure 2 shows the proposed project site plan.

III. EXISTING TRAFFIC CONDITIONS

A. Area Roadway System

In the vicinity of the project, Piilani Highway is a predominantly four-lane, two-way roadway generally oriented in the north-south direction that provides access through Kihei. North of the project site, Piilani Highway intersects Kaonoulu Street. At this unsignalized T-intersection, the northbound approach of the highway has an exclusive left-turn lane and two through lanes while the southbound approach has two through lanes and an exclusive right-turn lane. Kaonoulu Street is generally oriented

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in the east-west direction and serves as a connector roadway between South Kihei Road and Piilani Highway. At the intersection with Piilani Highway, the Kaonoulu Street approach has two stop-controlled lanes that serve left-turn and right-turn traffic movements. In addition, a refuge lane is provided within the median along the highway to assist vehicles turning left from Kaonoulu Street.

South of the intersection with Kaonoulu Street, Piilani Highway intersects
Kulanihakoi Street. At this unsignalized T-intersection, the northbound approach of
the highway has an exclusive left-turn lane and two through lanes while the
southbound approach has two through lanes and an exclusive right-turn lane.
Kulanihakoi Street is generally oriented in the east-west direction and serves as a
connector roadway between South Kihei Road and Piilani Highway. At the
intersection with Piilani Highway, the Kulanihakoi Street approach has two stopcontrolled lanes that serve left-turn and right-turn traffic movements. In addition, a
refuge lane is provided within the median along the highway to assist vehicles turning
left from Kulanihakoi Street.

Further south, Piilani Highway intersects E. Waipuilani Road. At this unsignalized T-intersection, the northbound approach of the highway has two through lanes while the southbound approach has two through lanes and an exclusive right-turn lane. E. Waipuilani Road is generally oriented in the east-west direction and serves as a connector roadway between South Kihei Road and Piilani Highway. At the intersection with Piilani Highway, the E. Waipuilani Road approach has one stop-controlled lane that serves right-turn traffic movements only.

At the southern end of the study area, Piilani Highway intersects Piikea Avenue. At this signalized intersection, the northbound approach of the highway has an exclusive left-turn lane and two through lanes while the southbound approach has two through lanes and an exclusive right-turn lane. Piikea Avenue is generally oriented in the east-west direction and serves as a connector roadway between South Kihei Road and Piilani Highway. At the intersection with Piilani Highway, the Piikea Avenue approach has exclusive left-turn and right-turn lanes.

Traffic Impact Report for Kihei High School

West of the intersection with Piilani Highway, Kulanihakoi Street intersects South Kihei Road. At this unsignalized T-intersection, the Kulanihakoi Street approach has two stop-controlled lanes that serve left-turn and right-turn traffic movements. South Kihei Road runs parallel to Piilani Highway and serves as an alternate north-south route through Kihei. At the intersection with Kulanihakoi Street, the northbound approach of South Kihei Road has a shared through and right-turn lane while the southbound approach has an exclusive left-turn lane and one through lane.

B. Traffic Volumes and Conditions

1. General

a. Field Investigation

Field investigations were conducted on January 26-27, 2011, and consisted of manual turning movement count surveys in the project vicinity. The manual turning movement count surveys were conducted between the morning peak hours of 6:00 AM and 9:00 AM, and the afternoon peak hours of 3:00 PM and 6:00 PM at the following intersections:

- · Piilani Highway and Kaonoulu Street
- · Piilani Highway and Kulanihakoi Street
- · Piilani Highway and E. Waipuilani Road
- · Piilani Highway and Piikea Avenue
- Kulanihakoi Street and South Kihei Road

Appendix A includes the existing traffic count data.

b. Capacity Analysis Methodology

The highway capacity analysis performed in this study is based upon procedures presented in the "Highway Capacity Manual", Transportation Research Board, 2000, and the "Synchro" software, developed by Trafficware. The analysis is based on the concept of Level of Service (LOS).

LOS is a quantitative and qualitative assessment of traffic operations. Levels of Service are defined by LOS "A" through "F";

LOS "A" representing ideal or free-flow traffic operating conditions and LOS "F" unacceptable or potentially congested traffic operating conditions.

"Volume-to-Capacity" (v/c) ratio is another measure indicating the relative traffic demand to the road carrying capacity. A v/c ratio of one (1.00) indicates that the roadway is operating at or near capacity. A v/c ratio of greater than 1.00 indicates that the traffic demand exceeds the road's carrying capacity. The LOS definitions are included in Appendix B.

2. Existing Peak Hour Traffic

a. General

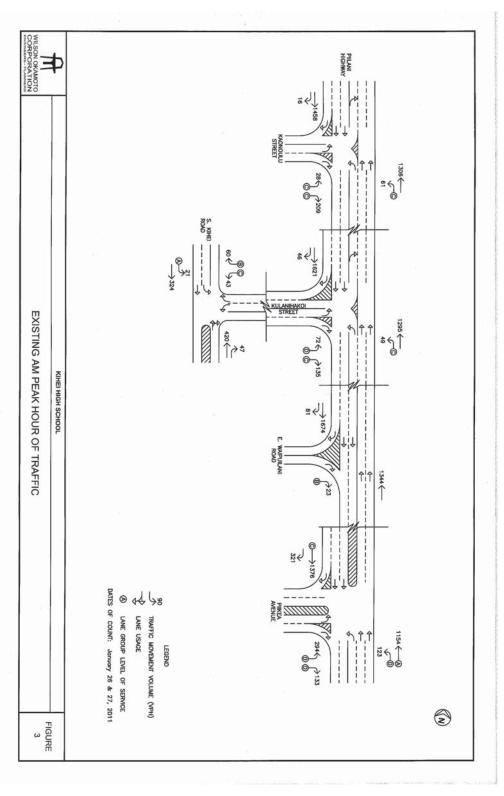
Figures 3 and 4 show the existing AM and PM peak hour traffic volumes and operating traffic conditions. The AM peak hour of traffic generally occurs between the hours of 7:15 AM and 8:15 AM while the PM peak hour of traffic generally occurs between the hours of 3:45 PM and 4:45 PM. The analysis is based on these peak hour time periods for each intersection to identify the traffic impacts resulting from the proposed project. LOS calculations are included in Appendix C.

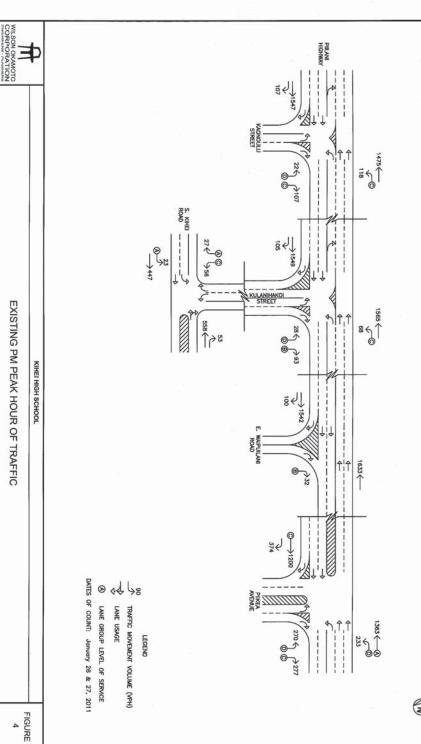
b. Piilani Highway and Kaonoulu Street

At the intersection with Kaonoulu Street, Piilani Highway carries 1,367 vehicles northbound and 1,474 vehicles southbound during the AM peak hour of traffic. During the PM peak period, traffic volumes are higher with 1,593 vehicles traveling northbound and 1,654 vehicles traveling southbound. The critical movement on the highway approaches of the intersection is the northbound left-turn traffic movement which operates at LOS "C" during both peak periods.

The Kaonoulu Street approach of the intersection carries 237 vehicles and 129 vehicles eastbound during the AM and PM peak

eak
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periods, respectively. The left-turn traffic movement on this approach operates at LOS "D" and LOS "C" during the AM and PM peak periods, respectively, while the right-turn traffic movement operates at LOS "C" during both peak periods. Traffic queues periodically formed on this approach with the average queue lengths of 2-3 vehicles observed during both peak periods.

Piilani Highway and Kulanihakoi Street

At the intersection with Kulanihakoi Street, Piilani Highway carries 1,344 vehicles northbound and 1,667 vehicles southbound during the AM peak hour of traffic. During the PM peak period, the overall traffic volume is higher with 1,633 vehicles traveling northbound and 1,654 vehicles traveling southbound. The critical movement on the highway approaches of the intersection is the northbound left-turn traffic movement which operates at LOS "C" during both peak periods.

The Kulanihakoi Street approach of the intersection carries 207 vehicles and 121 vehicles eastbound during the AM and PM peak periods, respectively. The left-turn traffic movement on this approach operates at LOS "C" during both peak periods while the right-turn traffic movement operates at LOS "C" and LOS "B" during the AM and PM peak periods, respectively. Traffic queues periodically formed on this approach with the average queue lengths of 2-4 vehicles observed during both peak periods.

Piilani Highway and E. Waipuilani Road

At the intersection with E. Waipuilani Road, Piilani Highway carries 1,344 vehicles northbound and 1,755 vehicles southbound during the AM peak hour of traffic. During the PM peak period, the overall traffic volume is higher with 1,633 vehicles traveling northbound and 1,642 vehicles traveling southbound.

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The E. Waipuilani Road approach of the intersection carries 23 vehicles and 32 vehicles eastbound during the AM and PM peak periods, respectively. This approach operates at LOS "B" during both peak periods.

e. Piilani Highway and Piikea Avenue

At the intersection with Piikea Avenue, Piilani Highway carries 1,277 vehicles northbound and 1,697 vehicles southbound. During the PM peak period, the overall traffic volume is higher with 1,596 vehicles traveling northbound and 1,574 vehicles traveling southbound. The critical movements on the highway approaches of the intersection are the northbound left-turn traffic movement which operates at LOS "D" during both peak periods and the southbound through traffic movement which operates at LOS "C" during both peak periods. Traffic queues periodically formed on the highway approaches of the intersection with the most significant queuing occurring during the PM peak periods. During this period, average queue lengths of 7-9 vehicles were observed on both approaches. These queues were observed to clear the intersection after each traffic signal cycle change.

The Piikea Avenue approach of the intersection carries 427 vehicles and 547 vehicles eastbound during the AM and PM peak periods, respectively. The left-turn traffic movement on this approach operates at LOS "D" during both peak periods while the right-turn traffic movement operates at LOS "D" and LOS "C" during the AM and PM peak periods, respectively. Traffic queues periodically formed on the Piikea Avenue approach of the intersection with average queue lengths of 7-9 vehicles observed during both peak periods.

Occasionally, queues extended through the upstream intersection with the Piilani Village Shopping Center, but most of these queues were observed to clear the intersection after each traffic signal cycle change.

f. Kulanihakoi Street and South Kihei Road

At the intersection with South Kihei Road, the Kulanihakoi Street approach of the intersection carries 103 vehicles and 83 vehicles westbound during the AM and PM peak periods, respectively. The critical movement on the Kulanihakoi Street approach is the left-turn traffic movement which operates at LOS "C" during both peak periods.

The South Kihei Road approaches of the intersection carry 467vehicles northbound and 342 vehicles southbound during the AM peak hour of traffic. During the PM peak period, traffic volumes are higher with 611 vehicles traveling northbound and 470 vehicles traveling southbound. The critical movement on the South Kihei Road approaches of the intersection is the southbound left-turn traffic movement which operates at LOS "A" during both peak periods.

IV. PROJECTED TRAFFIC CONDITIONS

A. Site-Generated Traffic

1. Trip Generation Methodology

The trip generation methodology used in this study is based upon generally accepted techniques developed by the Institute of Transportation Engineers (ITE) and published in "Trip Generation, 8th Edition," 2008. The ITE trip generation rates are developed empirically by correlating the vehicle trip generation data with various land use characteristics such as the number of vehicle trips generated per student. As previously stated, high school students from Kihei currently attend high schools in Kahului and Wailuku and these students are expected to transfer to the new high school once it is opened. As provided by the Department of Education (DOE), there are currently 704 students from Kihei attending high schools in other regions. Table 1 summarizes the project site trip generation characteristics applied to the AM and PM peak hours of traffic to measure the impact resulting from the proposed Kihei High School.

Table 1: Peak Hour Trip Generation

	YEAR 20	15
HIGH SCHOOL (EX	ISTING STUDENTS	5)
NDEPENDENT VAR	IABLE:	Students = 704 (Existing)
		PROJECTED TRIP ENDS
AM PEAK	ENTER	201
	EXIT	95
	TOTAL	296
PM PEAK	ENTER	43
	EXIT	49
	TOTAL	92
HIGH SCHOOL (NE	W STUDENTS)	***
NDEPENDENT VAR	IABLE:	Students = 96 (New)
		PROJECTED TRIP ENDS
AM PEAK	ENTER	27
1	EXIT	13
	TOTAL	40
PM PEAK	ENTER	6
	EXIT	6
	TOTAL	12
YEAR 2015 SUBTOT	AL	
NDEPENDENT VAR	IABLE:	Students = 800
		PROJECTED TRIP ENDS
· AM PEAK	ENTER	228
	EXIT	108
	TOTAL	336
PM PEAK	ENTER	49
	EXIT	55
	TOTAL	104
	YEAR 20	25
HIGH SCHOOL (NE	W STUDENTS)	
INDEPENDENT VAR	IABLE:	Students = 850 (New)
AM PEAK	ENTER	243
	EXIT	114
	TOTAL	357
PM PEAK	ENTER	52
	EXIT	59
	TOTAL	111

Table 1: Peak Hour Trip Generation (Cont'd)

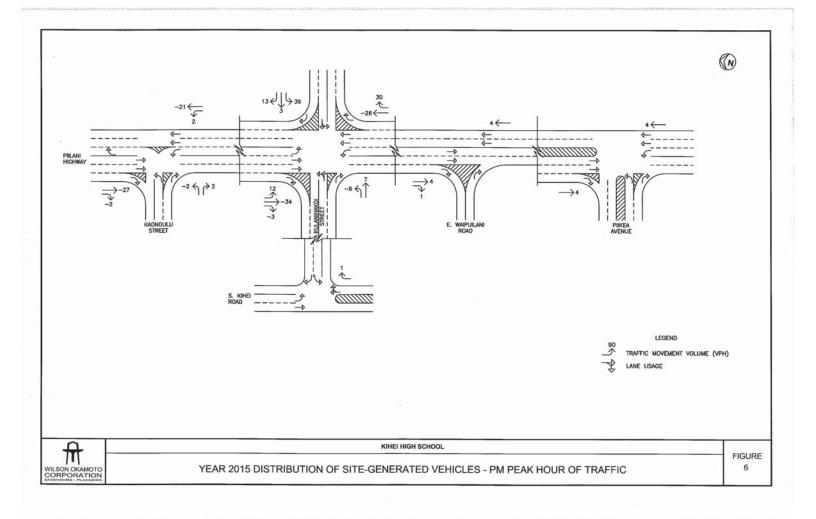
		PROJECTED TRIP ENDS
AM PEAK	ENTER	471
	EXIT	222
	TOTAL	693
PM PEAK	ENTER	101
	EXIT	114
	TOTAL	215

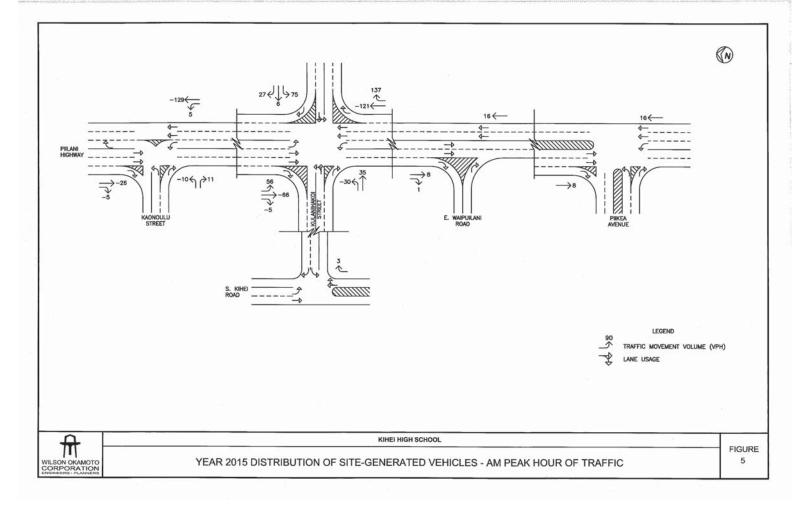
2. Trip Distribution

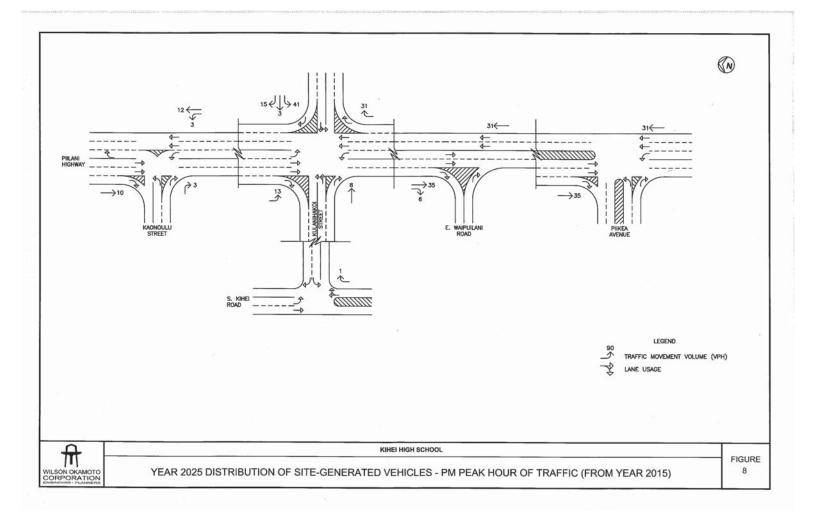
Figures 5 to 8 show the distribution of site-generated vehicular trips at the study intersections during the Year 2015 and Year 2025 AM and PM peak periods. Access to Kihei High School will be provided via new access road off Pillani Highway at the intersection with Kulanihakoi Street. High School students from Kihei currently attending other schools in Kahului and Wailuku are assumed to already be utilizing Pillani Highway to travel to/from Kihei. As such, trips associated with existing students were reassigned from Pillani Highway to the new high school access. The directional distribution of existing and new trips to/from the high school was based upon the relative distribution of households within the Kihei and Wailea areas and the available routes to/from the new school.

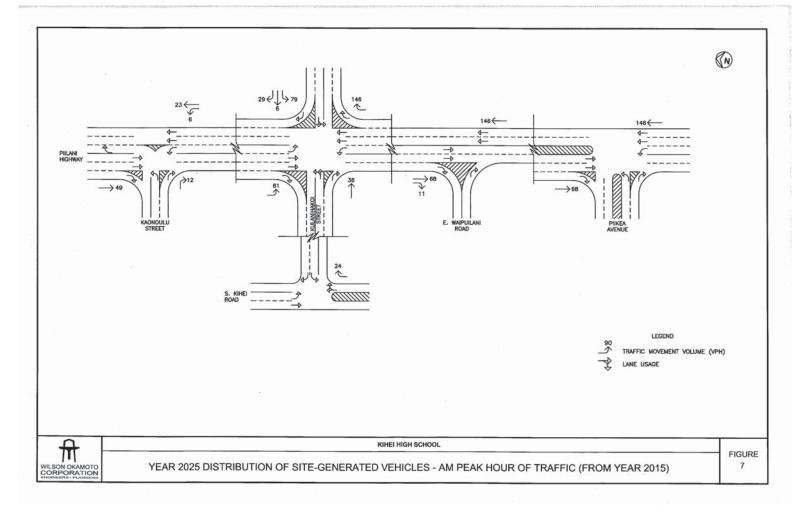
B. Through Traffic Forecasting Methodology

Historical traffic count data obtained from the State Department of Transportation (SDOT), Highway Division survey stations in the vicinity of the project site indicates traffic volumes have remained relatively stable and, as such, an annual traffic growth rate of approximately 1.0% per year was conservatively assumed along Piilani Highway and South Kihei Road in the project vicinity. Using 2011 as the Base Year, growth factors of 1.04 and 1.14 were applied to the existing through traffic demands along those roadways to achieve the projected Year 2015 and Year 2025, respectively traffic demands.









C. Other Considerations

Kihei Mauka

The agricultural lands surrounding the project site for the proposed high school are owned by Kaonoulu Ranch and Haleakala Ranch. The ranches have future plans to develop these lands (currently referred to as "Kihei Mauka") that will include residential, commercial, and industrial uses. The project development plan and implementation schedule for this project are not known at this time and, as such, the Kihei Mauka development is not incorporated into projected conditions. It should be noted that the ranches plans currently include connection to the access roadway for the high school. However, once the details of the Kihei Mauka development are known, the ranches should be undertaking a traffic study to assess the development's impact on the surrounding roadways.

2. Piilani Promenade and Maui Outlets Center

The Piilani Promenade and Maui Outlets Center will be located adjacent to Piilani Highway north of the proposed Kihei High School. The two projects are expected to include over 703,000 square feet of retail and restaurant space and include the extension of Kaonoulu Street further east. The project development plan and implementation schedule for these projects are not known at this time and, as such, the Piilani Promenade and Maui Outlets Center are not incorporated into projected conditions. It should be noted once the project details are known, the project developers should be undertaking traffic studies to assess the impact of the projects on the surrounding roadways.

3. Maui Research and Tech Park

The existing Maui Research and Tech Park is located east of Piilani
Highway near the intersection with Lipoa Street. The proposed project entails
the expansion of the existing tech park, as well as, development of other
residential and commercial uses in the surrounding areas. The project
development plan and implementation schedule for this project are not known

at this time and, as such, the Maui Research and Tech Park expansion is not incorporated into projected conditions. It should be noted once the project details are known, the project developers should be undertaking traffic studies to assess the impact of the projects on the surrounding roadways.

4. Honua'ula Development

The Honua'ula development will be located on an approximately 670 acre parcel near the end of Piilani Highway. The proposed development will include a maximum of 1,400 residential units (mix of single- and multi-family units), mixed use areas, two golf courses, and a variety of public and private amenities. The project development plan and implementation schedule for this development are not known at this time and, as such, the Honua'ula development is not incorporated into projected conditions.

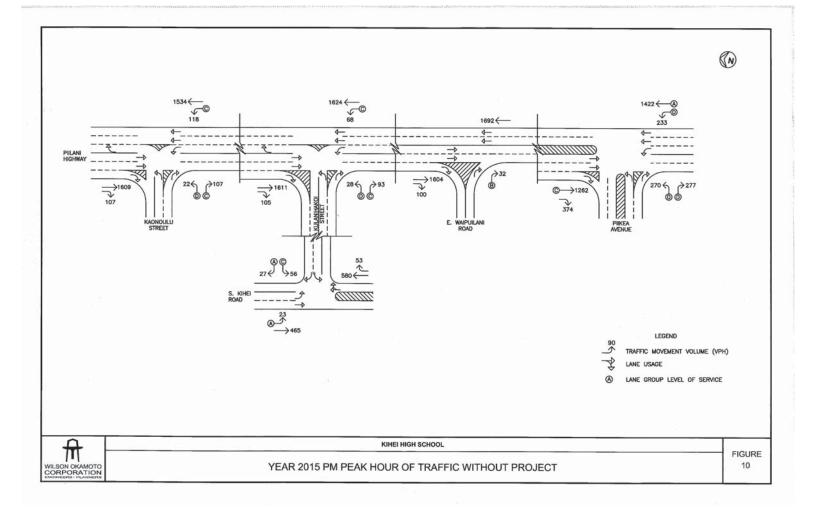
D. Year 2015 Total Traffic Volumes

1. Without Project

The projected Year 2015 peak hour traffic volumes and operating conditions in the project vicinity without the proposed Kihei High School are shown on Figures 9 and 10, and summarized in Table 2. The existing levels of service are provided for comparison purposes. LOS calculations are included in Appendix D.

Table 2: Existing and Projected Year 2015 (Without Project) LOS
Traffic Operating Conditions

Intersection	Critical Tr	affic	A	M	P	M	
	Movemer Approac	777	Exist	Year 2015 w/out Proj	Exist	Year 2015 w/out Proj	
Piilani Hwy/	Eastbound	LT	С	С	D	D	
Kaonoulu St		RT	С	С	С	С	
	Northbound	LT	С	C	С	С	
Piilani Hwy/	Eastbound	Eastbound LT		D	D	D	
Kulanihakoi St		RT	C	С	В	С	
	Northbound	LT	С	С	С	С	



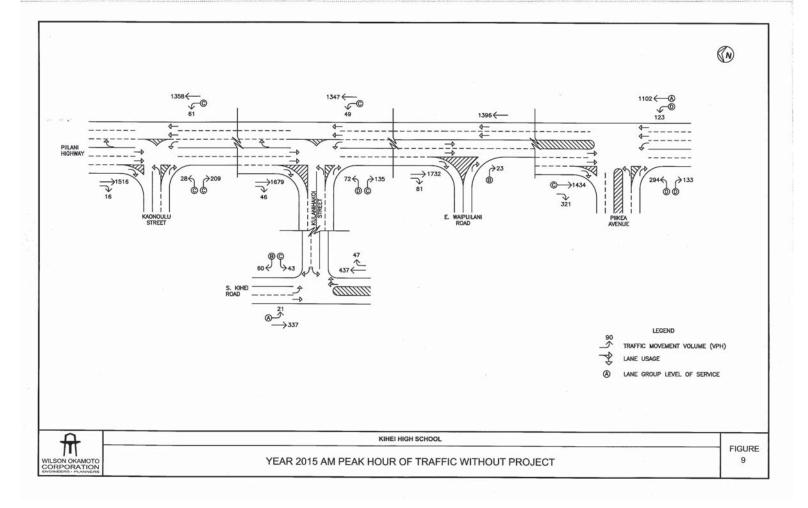


Table 2: Existing and Projected Year 2015 (Without Project) LOS Traffic Operating Conditions (Cont'd)

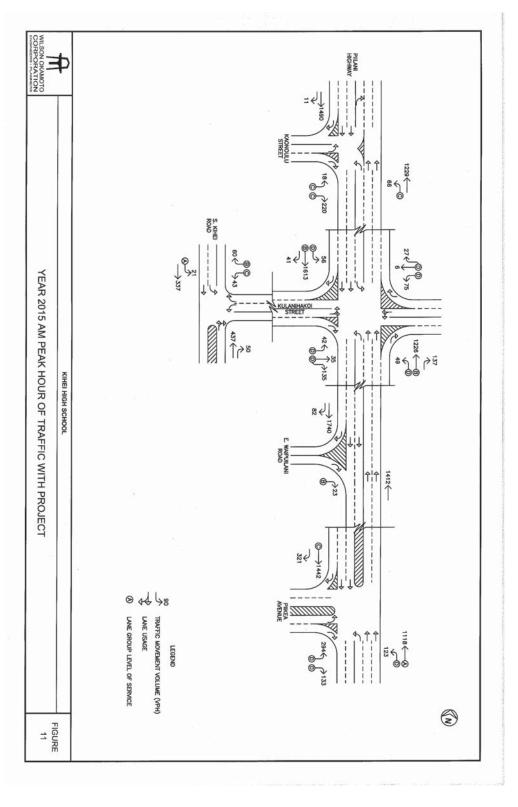
Intersection	Critical Tr	affic	A	M	PM		
	Movement/ Approach		Exist	Year 2015 w/out Proj	Exist	Year 2015 w/out Proj	
Piilani Hwy/ E. Waipuilani Rd	Eastbound	RT	В	В	В	В	
Piilani Hwy/	Eastbound	LT	D	D	D	D	
Piikea Ave		RT	D	D	C	D	
	Northbound	LT	D	D	D	D	
	Southbound	TH	С	С	С	С	
Kulanihakoi St/	Westbound	LT	С	C	С	С	
South Kihei Rd	Southbound	LT	В	В	A	A	

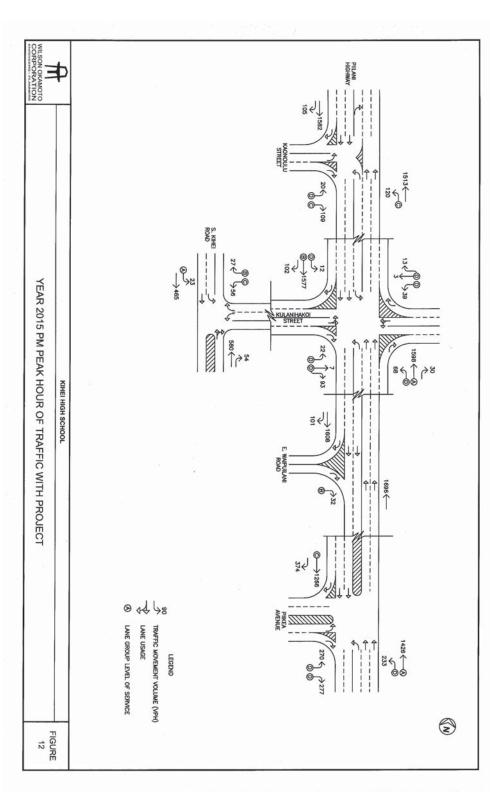
Under Year 2015 without project conditions, traffic operations in the project vicinity are expected to deteriorate slightly from existing conditions due to ambient growth in traffic along the surrounding roadways. The eastbound right-turn traffic movement at the intersection of Piilani Highway and Kulanihakoi Street is expected to deteriorate from LOS "B" to LOS "C" during the PM peak period while the eastbound right-turn traffic movement at the intersection of Piilani Highway and Piikea Avenue is expected to deteriorate from LOS "C" to LOS "D" during the PM peak period. The remaining critical movements at these intersections, as well as, the other study intersections are expected to continue operating at levels of service similar to existing conditions.

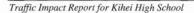
2. With Project

The Year 2015 cumulative peak hour traffic conditions with the proposed Kihei High School are shown in Figures 11 and 12, and summarized in Table 3. The cumulative volumes consist of site-generated traffic superimposed over Year 2015 projected traffic demands. Due to the anticipated increases in traffic at the intersection of Pillani Highway and

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Kulanihakoi Street due to ambient growth in traffic and the inclusion of the proposed access for the Kihei High School, a Traffic Signal Warrant Study was undertaken for that intersection (see Appendix E) to determine if a traffic signal system was warranted. Based on existing and projected traffic volumes, the study recommends the installation of a traffic signal system at that intersection. As such, a traffic signal system is assumed to be installed in conjunction with the Kihei High School project by the Year 2015. The projected Year 2015 (Without Project) operating conditions are provided for comparison purposes. LOS calculations are included in Appendix F.

Table 3: Projected Year 2015 (Without and With Project) LOS
Traffic Operating Conditions

Intersection		Critical Traffic AM					
		Movement/ Approach			Year 2015 w/out Proj	Year 2015 w/ Proj	
Piilani Hwy/	Eastbound	LT	C	C	D	D	
Kaonoulu St		RT	С	С	С	C	
	Northbound	LT	С	C	С		
Piilani Hwy/	Eastbound	LT ·	D	D	D	D	
Kulanihakoi St*		TH	-		-		
		RT	С	D	С	D	
	Westbound	LT-TH	-	D	-	D	
		RT	-	D	-	D	
	Northbound	LT	С	D	С	D	
		TH	-	В	-	A	
	Southbound	LT	-	D	-	D	
		TH	-	В	-	В	
Piilani Hwy/ E. Waipuilani Rd	Eastbound	RT	В	В	В	В	

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Table 3: Projected Year 2015 (Without and With Project) LOS Traffic Operating Conditions (Cont'd)

Intersection	Critical Tr	affic	Al	М	P	PM	
	Moveme Approac		Year 2015 w/out Proj	Year 2015 w/ Proj	Year 2015 w/out Proj	Year 2015 w/ Proj	
Piilani Hwy/	Eastbound	LT	D	D	D	D	
Piikea Ave		RT	D	D	D	D	
	Northbound	LT	D	D	D	D	
	Southbound	TH	C	С	С	C	
Kulanihakoi St/	Westbound	LT	С	С	С	С	
South Kihei Rd	Southbound	LT	В	В	A	В	

*Traffic signal system installed in conjunction with the proposed high school.

Under Year 2015 with project conditions, traffic operations in the project vicinity are generally expected to remain similar to without project conditions despite the addition of site-generated vehicles to the surrounding roadways. Along Piilani Highway, the critical movements at the intersection with Kaonoulu Street are expected to continue operating at LOS "C" or better during the AM peak period and LOS "D" or better during the PM peak period while those at the intersection and Piikea Avenue are expected to continue operating at LOS "D" during both peak periods. At the intersection of the highway with E. Waipuilani Road, the eastbound approach is expected to continue operating at LOS "B" during both peak periods while the critical movements at the intersection of Kulanihakoi Street and South Kihei Road area expected to continue operating at LOS "C" or better during both peak periods. At the intersection of Piilani Highway and Kulanihakoi Street, the critical movements are expected to operate at LOS "D" or better during both peak periods primarily due to the installation of a traffic signal system at that intersection.

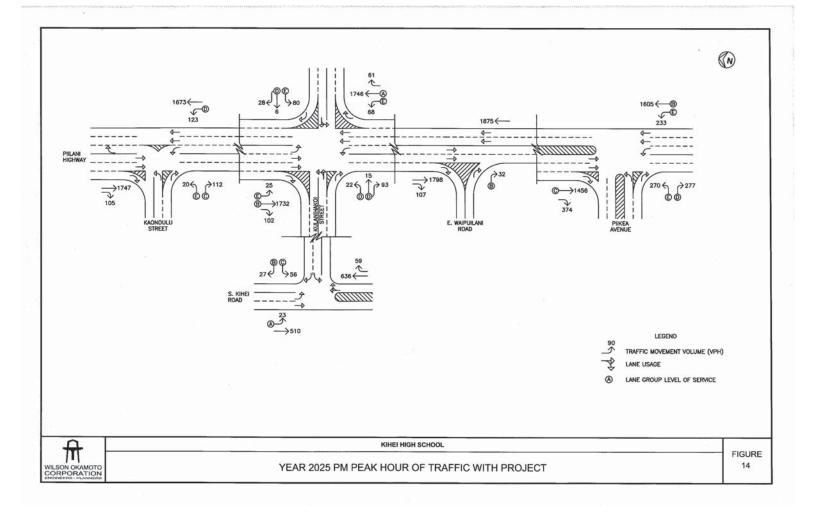
E. Year 2025 Total Traffic Volumes

The Year 2025 cumulative peak hour traffic conditions with the proposed Kihei High School are shown in Figures 13 and 14, and summarized in Table 4. The cumulative volumes consist of site-generated traffic superimposed over Year 2025 projected traffic demands. The projected Year 2015 (With Project) operating conditions are provided for comparison purposes. LOS calculations are included in Appendix G.

Table 4: Projected Year 2015 (With Project) and Year 2025 (With Project) LOS
Traffic Operating Conditions

Intersection	Critical T		A	M	PM		
	Movement/ A	Movement/ Approach		Year 2025 w/ Proj	Year 2015 w/ Proj	Year 2025 w/ Proj	
Piilani Hwy/	Eastbound	LT	Proj C	D	D	Е	
Kaonoulu St		RT	С	С	С	С	
	Northbound	LT	С	С	С	D	
Piilani Hwy/	Eastbound	LT-TH	D	D	D	D	
Kulanihakoi St*		RT	D	D	D	D	
	Westbound	LT-TH	D	Е	D	Е	
		RT	D	D	D	D	
	Northbound	LT	D	E	D	Е	
		TH	В	С	A	A	
	Southbound	LT	D	Е	D	Е	
		TH	В	С	В	В	
Piilani Hwy/ E. Waipuilani Rd	Eastbound	RT	В	С	В	В	
Piilani Hwy/	Eastbound	LT	D	Е	D	Е	
Piikea Ave		RT	D	D	D	D	
	Northbound	LT	D	Е	D	Е	
	Southbound	TH	C	С	С	С	

^{*}Traffic signal system installed in conjunction with the proposed high school.



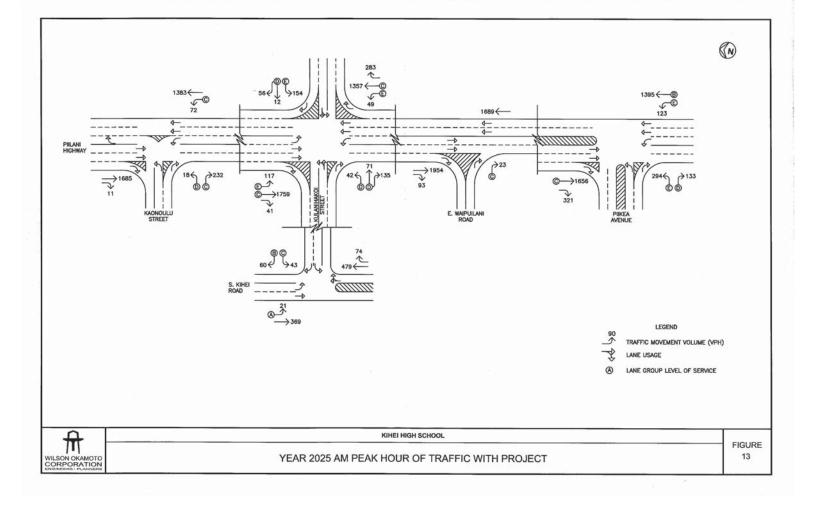


Table 4: Projected Year 2015 (With Project) and Year 2025 (With Project) LOS
Traffic Operating Conditions (Cont'd)

Intersection	Critical Tr	CONTRACTOR OF THE PARTY OF THE	1.57	M	PM		
	Movement/ A	Year 2015 w/ Proj	Year 2025 w/ Proj	Year 2015 w/ Proj	Year 2025 w/ Proj		
Kulanihakoi St/	Westbound	LT	C	C	C	C	
South Kihei Rd	Southbound	В	В	В	В		

Under Year 2025 with project conditions, traffic operations in the project vicinity are expected to deteriorate slightly from Year 2015 with project conditions primarily due to ambient growth in traffic along the surrounding roadways. Along Pillani Highway, the critical movements at the intersection with Kaonoulu Street are expected to operate at LOS "D" or better during the AM peak period and LOS "E" or better during the PM peak period while those at the intersections with Kulanihakoi Street and Pilkei Avenue are expected to operate at LOS "E" or better during both peak periods. At the intersection of the highway with E. Waipuilani Road, the eastbound approach is expected to operate at LOS "C" and LOS "B" during the AM and PM peak periods, respectively. Along South Kihei Road, the critical movements at the intersection with Kulanihakoi Street are expected to operate at LOS "C" or better during both peak periods.

V. RECOMMENDATIONS

Based on the analysis of the traffic data, the following are the recommendations of this study to be implemented prior to the opening of Kihei High School in the Year 2015:

- Maintain sufficient sight distance for motorists to safely enter and exit all project roadways.
- Provide adequate on-site loading and off-loading service areas and prohibit off-site loading operations.
- Provide adequate turn-around area for service, delivery, and refuse collection vehicles to maneuver on the project site to avoid vehicle-reversing maneuvers onto public roadways.

Traffic Impact Report for Kihei High School

- Provide sufficient turning radii at all project roadways to avoid or minimize vehicle encroachments to oncoming traffic lanes.
- Provide an exclusive right-turn lane and shared left-turn and through lane on the
 access road approach from the high school at the intersection with Piilani Highway.
 The layout and dimension of these lanes should be determined during the design
 phase of the project.
- Provide a channelized northbound deceleration lane along Pillani Highway at the intersection with the access road for the high school. The layout and dimension of these lanes should be determined during the design phase of the project.
- Provide a channelized northbound acceleration lane along Piilani Highway at the intersection with the access road for the high school. The layout and dimension of these lanes should be determined during the design phase of the project.
- Provide an exclusive southbound left-turn lane along Piilani Highway at the intersection with the access road for the high school. The layout and dimension of these lanes should be determined during the design phase of the project.
- Provide two eastbound departure lanes along the access road for the high school from the intersection with Piilani Highway. The layout and dimension of these lanes should be determined during the design phase of the project.
- 10. Modify the eastbound approach of Kulanihakoi Street at the intersection with Piilani Highway and the access road for the high school to provide an exclusive right-turn lane and a shared left-turn and through lane. The layout and dimension of these lanes should be determined during the design phase of the project.
- 11. Install at traffic signal system at the intersection of Pilani Highway with Kulanihakoi Street and the access road for the high school. The layout, phasing, and timing of this signal system should be determined during the design phase of the project.
- Prepare a Traffic Management Plan for the high school to minimize the impact of school related vehicles on the surrounding roadways. This plan should address daily school and special event traffic.
- 13. Consider preparing Traffic Assessment Reports periodically (every 5 years at a minimum) once the high school is opened to verify projected traffic conditions in the vicinity and assess the effectiveness of traffic management strategies implemented by the high school.

VI. CONCLUSION

High school students that reside in Kihei currently have to attend Maui High School in Kahului or Baldwin High School in Wailuku. The proposed Kihei High School will allow these students to attend a high school in their district. The proposed high school will include classrooms, support facilities, and athletic facilities to support an initial enrollment of 800 students with an ultimate enrollment of 1,650 students expected within 10 years. With the development of the proposed high school, traffic operations upon opening are expected to remain similar to without project conditions primarily due to the provision of turning lanes and a traffic signal system at the intersection of Piilani Highway with Kulanihakoi Street and the access road for the high school. By the Year 2025, traffic operations in the vicinity are expected to deteriorate slightly primarily due to ambient growth in traffic along the surrounding roadways. As such, the preparation of a Traffic Management Plan for the high school is recommended to minimize the impact of school related traffic on the surrounding roadways.

APPENDIX A EXISTING TRAFFIC COUNT DATA

Counter:5671, 5674 Counted By:DB and DF Weather:Clear

File Name : KaopiiPM Site Code : 00000006 Start Date : 1/26/2011

Page No : 1

-							Groups	Printed- I	Jnshifted									
		Pillani Highway Southbound				Westboun	Pillani Highway Northbound						onoulu Stre Eastbound		rt			
Start Time	Left	Thru	Right	Peds	App. Total	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Tota	
02:00 PM	0	340	13	0	353	0	23	328	0	0	351	6	0	31	0	37	74	
02:15 PM	0	312	11	0	323	0	22	438	0	0	460	7	0	25	0	32	81	
02:30 PM	0	306	10	0	316	0	28	471	0	0	499	3	0	37	0	40	858	
02:45 PM	0	352	9	0	361	0	38	393	0	0	431	1	0	67	0	68	860	
Total	0	1310	43	0	1353	0	111	1630	0	0	1741	17	0	160	0	177	327	
03:00 PM	0	414	9	0	423	0	17	351	0	0	368	3	0	27	0	30	821	
03:15 PM	0	388	12	0	400	0	19	387	0	0	406	4	0	30	0	34	840	
03:30 PM	0	393	12	0	405	0	27	433	0	0	460	7	0	29	0	36	901	
03:45 PM	0	401	4	0	405	0	36	335	0	0	371	5	0	33	0	38	814	
Total	0	1596	37	0	1633	0	99	1506	0	0	1605	19	0	119	0	138	337	
04:00 PM	0	456	16	0	472	0	23	346	0	0	369	3	0	23	0	26	86	
04:15 PM	0	367	8	0	375	0	28	431	0	0	459	8	0	19	0	27	861	
04:30 PM	0	379	11	0	390	0	32	371	0	0	403	6	0	36	0	42	83	
04:45 PM	0	395	13	0	408	0	22	265	0	0	287	3	0	30	0	33	72	
Total	0	1597	48	0	1645	0	105	1413	0	0	1518	20	0	108	0	128	329	
05:00 PM	0	363	12	0	375	0	30	320	0	0	350	4	0	22	0	26	75	
05:15 PM	0	369	11	0	380	0	18	328	0	0	346	5	0	26	0	31	75	
05:30 PM	0	305	13	0	318	0	22	294	0	0	316	7	0	30	0	37	67	
05:45 PM	0	318	10	0	328	0	18	223	0	0	241	4	0	32	0	36	608	
Total	0	1355	46	0	1401	0	88	1165	0	0	1253	20	0	110	0	130	278	
Grand Total	0	5858	174	0	6032	0	403	5714	0	0	6117	76	0	497	0	573	1272	
Approh %	0	97.1	2.9	0			6.6	93.4	0	0		13.3	0	86.7	0			
Total %	0	46	1.4	0	47.4	0	3.2	44.9	0	0	48.1	0.6	0	3.9	0	4.5		

			ani Highwo outhbound				Westboun			lani Highw Northbound					onoulu Str Eastbound		re Laboratorio	
Start Time	Left	Thru	Right	Peds	App.	Total	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Tota
eak Hour Analysis Fr	om 02:00	PM to 05:4	5 PM - Pe	ak 1 of 1														
Peak Hour for Entire In	tersection	Begins at	03:30 PM	1177700000														
03:30 PM	0	393	12	0		405	0	27	433	0	0	460	7	0	29	0	36	901
03:45 PM	0	401	4	0		405	0	36	335	0	0	371	5	0	33	0	38	814
04:00 PM	0	456	16	0		472	0	23	346	0	0	369	3	0	23	0	26	867
04:15 PM	0	367	8	0		375	0	28	431	0	0	459	8	0	19	0	27	861
Total Volume	0	1617	40	0		1657	0	114	1545	0	0	1659	23	0	104	0	127	3443
% App. Total	0	97.6	2.4	0				6.9	93.1	0	0		18.1	0	81.9	0		
PHF	.000	.887	.625	.000	(S.FReco.)	.878	.000	.792	.892	.000	.000	.902	.719	.000	.788	.000	.836	.955

Wilson Okamoto Corporation 1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:5671/5674 Counted By:DB and DF Weather:Clear File Name : KaopiiAM Site Code : 00000006 Start Date : 1/26/2011

Page No : 1

										s Printed-	Unshifte	d									
	- 20200		ilani High Southbou					inoulu Si Vestbou				Pii	lani High Iorthbou	way nd				inoulu Si Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	In
06:00 AM	0	114	2	0	116	0	0	0	0	0	4	106	0	0	110	3	0	5	0	8	23
06:15 AM	0	180	5	0	185	0	0	0	0	0	1	131	0	0	132	9	0	16	0	25	34
06:30 AM	0	209	0	0	209	0	0	0	0	0	5	222	0	0	227	3	0	17	0	20	45
06:45 AM	0	235	7	0	242	0	0	0	0	0	8	282	0	0	290	6	0	33	0	39	57
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07:00 AM	0	268	0	0	268	0	0	0	0	0	11	292	0	0	303	7	0	29	0	36	607
07:15 AM	0	393	6	0	399	0	0	0	0	0	11	328	0	0	339	6	0	63	0	69	803
07:30 AM	0	403	4	0	407	0	0	0	0	0	18	341	0	0	359	10	0	61	0	71	83
07:45 AM	0	327	4	0	331	0	0	0	0	0	21	335	0	0	356	5	0	42	0	47	734
Total	0	1391	14	0	1405	0	0	0	0	0	61	1296	0	0	1357	28	0	195	0	223	298
08:00 AM	0	345	2	0	347	0	0	0	0	0	11	299	0	0	310	7	0	45	0	52	70
08:15 AM	0	286	5	0	291	0	0	0	0	0	13	258	0	0	271	8	0	37	0	45	60
08:30 AM	0	270	6	0	276	0	0	0	0	0	13	329	0	0	342	4	0	31	0	35	65
08:45 AM	0	267	7	0	274	0	0	0	0	0	11	296	0	0	307	7	0	40	0	47	621
Total	0	1168	20	0	1188	0	0	0.	0	0	48	1182	0	0	1230	26	0	153	0	179	259
Grand Total	0	3297	48	0	3345	0	0	0	0	01	127	3219	0	0	3346	75	0	419	0	494	7188
Approh %	0	98.6	1.4	0		0	0	0	0		3.8	96.2	0	0		15.2	0	84.8	0		
Total %	0	45.9	0.7	0	46.6	0	0	0	0	0	1.8	44.8	0	0	46.6	1	0	5.8	0	6.9	

			ani High outhbou					inoulu Si Vestbou		103			lani High Vorthbou					inoulu St Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int Tota
Peak Hour Analys	is From	06:00 AM	If to 08:4	5 AM - P	eak 1 of 1										-						
Peak Hour for Ent	ire Inters	section B	egins at	07:15 AM	1																
07:15 AM	0	393	6	0	399	0	0	0	0	0	11	328	0	0	339	6	0	63	0	69	807
07:30 AM	0	403	4	0	407	0	0	0	0	0	18	341	0	0	359	10	0	61	0	71	837
07:45 AM	0	327	4	0	331	0	0	0	0	0	21	335	0	0	356	5	0	42	0	47	734
08:00 AM	0	345	2	0	347	0	0	0	0	0	11	299	0	0	310	7	0	45	0	52	709
Total Volume	0	1468	16	0	1484	0	0	0	0	0	61	1303	0	0	1364	28	0	211	0	239	3087
% App. Total	0	98.9	1.1	0	1.00	0	0	0	0	2.27	4.5	95.5	0	0	3,00000	11.7	0	88.3	0	0000	
PHF	.000	.911	.667	.000	.912	.000	.000	.000	.000	.000	.726	.955	.000	.000	.950	.700	.000	.837	.000	.842	.922

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1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:3890/5675 Counted By:SH and NH Weather:Clear File Name: KulpilPM Site Code: 00000007 Start Date: 1/26/2011

Page No : 1

			lani High louthbou					nihakoi : Vestbou					lani High Iorthbou					nihakoi : Eastbour			2000
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	,Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	In Tota
02:00 PM	0	328	24	0	352	0	0	0	0	0	11	311	0	0	322	30	0	21	0	51	72
02:15 PM	0	387	24	0	411	0	0	0	0	0	13	403	0	0	416	29	0	14	0	43	87
02:30 PM	0	365	29	0	394	0	0	0	0	0	16	392	0	0	408	36	0	16	0	52	85
02:45 PM	0	416	30	0	446	0	0	0	0	0	10	405	0	0	415	39	0	37	0	76	93
Total	0	1496	107	0	1603	0	0	0	0	0	50	1511	0	0	1561	134	0	88	0	222	338
03:00 PM	0	414	29	0	443	0	0	0	0	0	12	325	0	0	337	23	0	19	0	42	82
03:15 PM	0	408	20	0	428	0	0	0	0	0	16	398	0	0	414	7	0	19	0	26	86
03:30 PM	0	393	18	0	411	0	0	0	0	0	17	413	0	0	430	7	0	31	0	38	87
03:45 PM	0	403	29	0	432	0	0	0	0	0	13	418	0	0	431	9	0	31	0	40	90
Total	0	1618	96	0	1714	0	0	0	0	0	58	1554	0	0	1612	46	0	100	0	146	347
04:00 PM	0	469	23	0	492	0	0	0	0	- 0	19	344	0	0	363	7	0	18	0	25	88
04:15 PM	0	347	28	0	375	0	0	0	0	0	19	438	0	0	457	6	0	23	0	29	86
04:30 PM	0	404	25	0	429	0	0	0	0	0	17	448	0	0	465	6	0	21	0	27	92
04:45 PM	0	376	25	0	401	0	0	0	0	0	20	310	0	0	330	9	0	27	0	36	76
Total	0	1596	101	0	1697	0	0	0	0	0	75	1540	0	0	1615	28	0	89	0	117	342
05:00 PM	0	386	22	0	408	0	0	0	0	0	10	311	0	0	321	9	0	18	0	27	75
05:15 PM	0	390	26	0	416	0	0	0.	0	0	17	363	0	0	380	8	0	33	0	41	83
05:30 PM	0	317	18	0	335	0	0	0	0	0	23	291	0	0	314	12	0	22	0	34	68
05:45 PM	0	330	27	0	357	0	0	0	0	0	27	271	0	0	298	7	0	23	0	30	68
Total	0	1423	93	0	1516	0	0	0	0	0	77	1236	0	0	1313	36	0	96	0	132	296
06:00 PM	0	0	0	0	0	0	0	0	0	0	5	65	0	0	70	0	0	0	0	0	. 7
Grand Total	0	6133	397	0	6530	0	0	0	0	0	265	5906	0	0	6171	244	0	373	0	617	1331
Apprch %	0	93.9	6.1	0		0	0	0	0		4.3	95.7	0	0		39.5	0	60.5	0		
Total %	0	46.1	3	0	49	0	0	0	0	0	2	44.3	. 0	0	46.3	1.8	0	2.8	0	4.6	

			lani High outhbou					nihakoi : Vestbou					lani High Iorthbou					nihakoi : Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
eak Hour Analys	is From	02:00 PI	M to 05:3	0 PM - Pe	eak 1 of 1						-				-						-
Peak Hour for Ent	ire Inters	ection B	legins at	03:45 PM	1																
03:45 PM	0	403	29	0	432	0	0	0	0	0	13	418	0	0	431	9	0	31	0	40	903
04:00 PM	0	469	23	0	492	0	0	0	0	0	19	344	0	0	363	7	0	18	0	25	880
04:15 PM	0	347	28	0	375	0	0	0	0	0	19	438	0	0	457	6	0	23	0	29	861
04:30 PM	0	404	25	0	429	0	0	0	0	0	17	448	0	0	465	6	0	21	0	27	921
Total Volume	0	1623	105	0	1728	0	0	0	0	0	68	1648	0	0	1716	28	0	93	0	121	3565
% App. Total	0	93.9	6.1	0	10000	0	0	0	0	- 51	4	96	0	0		23.1	0	76.9	0		
PHF	.000	.865	.905	.000	.878	.000	.000	,000	.000	.000	.895	.920	.000	.000	.923	.778	.000	.750	.000	.756	.968

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1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:3890/5675 Counted By:SH and NH Weather:Clear File Name: KulpiiAM Site Code: 00000007 Start Date: 1/26/2011 Page No: 1

Groups Printed- Unshifted Kulanihakoi Street Pillani Highway Southbound Piilani Highway Northbound Kulanihakoi Street Westbound Eastbound App. Total 103 127 215 Start Time Left Thru Right Peds Left Thru Right Peds Left Thru Right Peds Left Thru Right Peds Total 229 329 478 546 1582 06:00 AM 06:15 AM 06:30 AM 06:45 AM Total 104 176 236 100 125 214 11 8 14 000 268 713 37 57 61 47 202 07:00 AM 07:15 AM 07:30 AM 07:45 AM Total 6 12 7 610 837 852 0 0 0 264 443 437 270 449 449 0.00 303 331 342 10 19 21 000 27 000 000 296 323 334 38 40 390 1534 351 795 3094 330 08:00 AM 08:15 AM 08:30 AM 08:45 AM Total 355 344 285 376 357 296 284 250 297 296 266 308 714 670 654 16 9 22 26 38 28 42 47 50 299 1283 50 629 2667 112 3 1.5 000 3572 97 48.6 000 105 3.3 1.4 204 40.6 2.8 Grand Total 3684 000 0 3051 3156 000 0 0 0 503 7343 59.4 Apprch % Total % 96.7 41.5 43 6.9 50.2

			lani High Southbou					nihakoi : Vestbou					lani High Iorthbou			-,4		nihakoi : Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int Tota
Peak Hour Analys	is From	06:00 A	VI to 08:4	5 AM - P	eak 1 of 1																
Peak Hour for Ent	ire Inters	section E	Begins at	07:15 AN	4																
07:15 AM	0	443	6	0	449	0	0	0	0	0	8	323	0	0	331	19	0	38	0	57	83
07:30 AM	0	437	12	0	449	0	0	0	0	0	8	334	0	0	342	21	0	40	0	61	85
07:45 AM	0	390	7	0	397	0	0	0	0	0	21	330	.0	0	351	16	0	31	0	47	79
08:00 AM	0	355	21	0	376	0	0	0	0	0	12	284	0	0	296	16	0	26	0	42	71
Total Volume	0	1625	46	0	1671	0	0	0	0	0	49	1271	0	0	1320	72	0	135	0	207	319
% App. Total	0	97.2	2.8	0	12.100124	0	0	0	0		3.7	96.3	0	0	0.000	34.8	0	65.2	0	200	
PHF	.000	.917	.548	.000	.930	.000	.000	.000	.000	.000	.583	.951	.000	.000	.940	.857	.000	.844	.000	.848	.93

Counter:5671/5672 Counted By:JS, TT Weather:Clear File Name : WaipiiPM Site Code : 00000009 Start Date : 1/27/2011 Page No : 1

			lani High outhbou					'aipuilani Vestbour	Road	s Printed-		Pii	lani High Iorthbou					alpullani Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Tota
02:00 PM	0	314	21	0	335	0	0	0	0	0	0	381	0	0	381	0	0	6	0	6	722
02:15 PM	0	355	21	0	376	0	0	0	0	0	0	547	0	0	547	0	0	6	0	6	929
02:30 PM	0	363	27	0	390	0	0	0	0	0	0	697	0	0	697	0	0	14	0	14	1101
02:45 PM	0	369	34	0	403	0	0	0	0	0	0	931	0	0	931	0	0	7	0	7	1341
Total	0	1401	103	0	1504	0	0	0	0	0	0	2556	0	0	2556	0	0	33	0	33	4093
03:00 PM	0	422	29	0	451	0	0	0	0	0	0	655	0	0	655	0	0	6	0	6	1112
03:15 PM	0	385	23	0	408	0	0	0	0	0	0	515	0	0	515	0	0	9	0	9	932
03:30 PM	0	403	27	0	430	0	0	0	0	0	0	616	0	0	616	0	0	11	0	11	1057
03:45 PM	0	395	24	0	419	0	0	0	0	0	0	426	0	0	426	0	0	7	0	7	852
Total	0	1605	103	0	1708	0	0	0	0	0	0	2212	0	0	2212	0	0	33	0	33	3953
04:00 PM	0	360	28	0	388	0	0	0	0	0	0	587	0	0	587	0	0	15	0	15	990
04:15 PM	0	357	18	0	375	0	0	0	0	0	0	481	0	0	481	0	0	7	0	7	863
04:30 PM	0	368	30	0	398	0	0	0	0	0	0	481	0	0	481	0	0	3	0	3	882
04:45 PM	0	374	37	0	411	0	0	0	0	0	0	397	0	0	397	0	0	6	0	6	814
Total	0	1459	113	0	1572	0	0	0	0	0	0	1946	0	0	1946	0	0	31	0	31	3549
05:00 PM	0	396	36	0	432	0	0	0	0	0	0	383	0	0	383	0	0	5	0	5	820
05:15 PM	0	395	28	0	423	0	0	0	0	0	0	348	0	0	348	0	0	10	0	10	781
05:30 PM	0	321	33	0	354	0	0	0	0	0	0	397	0	0	397	0	0	3	0	3	754
05:45 PM	0	363	32	0	395	0	0	0	0	0	0	0	0	0	0	0	0	15	0	15	410
Total	0	1475	129	0	1604	0	0	0	0	0	0	1128	0	0	1128	0	0	33	0	33	2765
arand Total	0	5940	448	0	6388	0	0	0	0	0	0	7842	0	0	7842	0	0	130	0	130	14360
Approh %	0	93	7	0	0.000	0	0	0	0	(3)	0	100	0	0		0	0	100	0	10000	
Total %	0	41.4	3.1	0	44.5	0	0	0	0	0	0	54.6	. 0	0	54.6	0	0	0.9	0	0.9	

			lani High					aipuilani Vestbou					lani High Iorthbou					aipuilani Eastbour			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int Tota
Peak Hour Analys									- 5%		1.5				X-11						
eak Hour for Ent	ire Inters	ection E	Begins at	02:30 PM																	
02:30 PM	0	363	27	0	390	0	0	0	0	0	0	697	0	0	697	0	0	14	0	14	110
02:45 PM	0	369	34	0	403	0	0	0	0	0	0	931	0	0	931	0	0	7	0	7	134
03:00 PM	0	422	29	0	451	0	0	0	0	0	0	655	0	0	655	0	0	6	0	6	111
03:15 PM	0	385	23	0	408	0	0	0	0	0	0	515	0	0	515	0	0	9	0	9	93
Total Volume	0	1539	113	0	1652	0	0	0	0	0	0	2798	0	0	2798	0	0	36	0	36	4486
% App. Total	0	93.2	6.8	0	and the said	0	0	0	0		0	100	0	0		0	0	100	0		
PHF	.000	.912	.831	.000	.916	.000	.000	.000	.000	.000	.000	.751	.000	.000	.751	.000	.000	.643	.000	.643	.836

Wilson Okamoto Corporation 1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:D4-5671, D4-5672 Counted By:JS, TT Weather:Clear

File Name : WaipiiAM Site Code : 00000009 Start Date : 1/27/2011 Page No :1

						Inshifted	roups Printed- L	G					
			aipuilani Roa astbound	E		Northbound	Westbound			lani Highway Southbound	5		
Int. Total	App. Total	Peds	Right	Thru	Left	App. Total	App. Total	App. Total	Peds	Right	Thru	Left	Start Time
121	4	0	4	0	0	0	0	117	0	2	115	0	06:00 AM
175	3	0	3	0	0	0	0	172	0	2	170	0	06:15 AM
184	6	0	6	0	0	0	0	178	0	5	173	0	06:30 AM
233	5	0	5	0	0	0	0	228	0	5	223	0	06:45 AM
713	18	0	18	0	0	0	0	695	0	14	681	0	Total
310	4	0	4	0	0	01	0	306	0	11	295	0	07:00 AM
455	7	0	7	0	0	0	0	448	0	21	427	0	07:15 AM
446	5	0	5	0	0	0	0	441	0	23	418	0	07:30 AM
412	1	0	1	0	0	0	0	411	0	21	390	0	07:45 AM
1623	17	0	17	0	0	0	0	1606	0	76	1530	0	Total
373	10	0	10	0	0	0	0	363	0	16	347	0	08:00 AM
343	5	0	5	0	0	0	0	338	0	18	320	0	08:15 AM
283	15	0	15	0	0	0	0	268	0	9	259	0	08:30 AM
324	9	0	9	0	0	0	0	315	0	18	297	0	08:45 AM
1323	39	0	39	0	0	0	0	1284	0	61	1223	0	Total
3659	74	0	74	0	0	0	0	3585	0	151	3434	0	Grand Total
		0	100	0	0	-	-	200000000000000000000000000000000000000	0	4.2	95.8	0	Appreh %
	2	0	2	0	0	0	0	98	0	4.1	93.9	0	Total %

		Piilani Hiç Southbo			Westbound	Northbound		E. Waipuila Eastbo			
Start Time	Left	Thru	Right	App. Total	App. Total	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 06:00	AM to 08:45 AM	1 - Peak 1 of 1									
Peak Hour for Entire Intersection	Begins at 07:1	5 AM									
07:15 AM	0	427	21	448	0	0	0	0	7	7	455
07:30 AM	0	418	23	441	0	0	0	0	5	5	446
07:45 AM	0	390	21	411	0	0	0	0	1	1	412
08:00 AM	0	347	16	363	0	0	0	0	10	10	373
Total Volume	0	1582	81	1663	0	0	0	0	23	23	1686
% App. Total	0	95.1	4.9		5.545	2000	0	0	100	-	100
PHF	.000	.926	.880	.928	.000	.000	.000	.000	.575	.575	.926

Counter:D4-3889, D4-5674 Counted By:DF, BB Weather:Clear

File Name: PiipiiPM Site Code: 00000010 Start Date: 1/27/2011 Page No: 1

							Groups	Printed- I	Jnshifted			(A)(C) (A)(A)		1022	DOM: N		
			ilani Highw Southbound			Westboun			ilani Highw Northbound					ikea Avenu Eastbound			
Start Time	Left	Thru	Right	Peds	App. Total	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
02:00 PM	0	218	80	0	298	0	65	317	0	0	382	83	0	68	0	151	831
02:15 PM	0	279	103	0	382	0	60	342	0	0	402	82	0	69	0	151	935
02:30 PM	0	325	99	0	424	0	68	315	0	0	383	88	0	79	0	167	974
02:45 PM	0	282	106	0	388	0	85	327	0	0	412	87	0	69	0	156	956
Total	0	1104	388	0	1492	0	278	1301	0	0	1579	340	0	285	0	625	3696
03:00 PM]	0	330	83	0	413	0	51	332	0	0	383	55	0	70	0	125	921
03:15 PM	0	305	104	0	409	0	64	358	0	0	422	76	0	75	0	151	982
03:30 PM	0	321	90	0	411	0	59	355	0	0	414	69	0	91	0	160	985
03:45 PM	0	299	96	0	395	0	62	316	0	0	378	75	0	79	0	154	927
Total	0	1255	373	0	1628	0	236	1361	0	0	1597	275	0	315	0	590	3815
04:00 PM	0	268	97	0	365	0	55	354	0	0	409	60	0	76	0	136	910
04:15 PM	0	308	79	0	387	0	68	349	0	0	417	69	0	57	0	126	930
04:30 PM	0	279	88	0	367	0	48	336	0	0	384	65	0	65	0	130	881
04:45 PM	0	284	94	0	378	0	55	232	0	0	287	58	0	67	0	125	790
Total	0	1139	358	0	1497	. 0	226	1271	0	0	1497	252	0	265	0	517	3511
05:00 PM	0	240	75	0	315	0 0	46	326	0	0	372	55	0	78	0	133	820
05:15 PM	0	276	124	0	400	0	57	256	0	0	313	70	0	74	0	144	857
05:30 PM	0	244	91	0	335	0	42	232	0	0	274	66	0	63	0	129	738
05:45 PM	0	196	104	0	300	0	0	0	0	0	0	60	0	66	0	126	426
Total	0	956	394	0	1350	0	145	814	0	0	959	251	0	281	0	532	2841
Grand Total	0	4454	1513	0	5967	0	885	4747	0	0	5632	1118	0	1146	0	2264	13863
Apprch %	0	74.6	25.4	0			15.7	84.3	0	0		49.4	0	50.6	0		
Total %	0	32.1	10.9	0	43	0	6.4	34.2	0	0	40.6	8.1	0	8.3	0	16.3	

		Pillani H Southb			Westbound		Pillani H Northb				Piikea A Eastb			
Start Time	Left	Thru	Right	App. Total	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From (02:00 PM to	05:45 PM -	Peak 1 of 1	l										
Peak Hour for Entire Inters	ection Begin	s at 02:45 f	PM											
02:45 PM	0	282	106	388	0	85	327	0	412	87	0	69	156	956
03:00 PM	0	330	83	413	0	51	332	0	383	55	0	70	125	921
03:15 PM	0	305	104	409	0	64	358	0	422	76	0	75	151	982
03:30 PM	0	321	90	411	0	59	355	0	414	69	0	91	160	985
Total Volume	0	1238	383	1621	0	259	1372	0	1631	287	0	305	592	3844
% App. Total	0	76.4	23.6			15.9	84.1	0		48.5	0	51.5		
PHF	.000	.938	.903	.981	.000	.762	.958	.000	.966	.825	.000	.838	.925	.976

Wilson Okamoto Corporation

1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:3889/5674 Counted By:DF and BB Weather:Clear

File Name: PiipiiAM Site Code: 00000010 Start Date: 1/27/2011 Page No: 1

							Groups	Printed- U	Jnshifted								
			lani Highwa Southbound			Westboun d		Pi	ilani Highw Northbound	ay i				ikea Avenu Eastbound	e		
Start Time	Left	Thru	Right	Peds	App. Total	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
06:00 AM	0	80	24	0	104	0	4	81	0	0	85	20	0	13	0	33	222
06:15 AM	0	164	27	0	191	0	14	105	0	0	119	26	0	5	0	31	341
06:30 AM	0	194	29	0	223	0	10	167	0	0	177	28	0	20	0	48	448
06:45 AM	0	210	45	0	255	0	19	213	0	0	232	48	0	15	0	63	550
Total	0	648	125	0	773	0	47	566	0	0	613	122	0	53	0	175	1561
07:00 AM	0	240	61	0	301	0	19	251	0	0	270	43	0	19	0	62	633
07:15 AM	0	359	72	0	431	0	20	314	0	0	334	67	0	22	0	89	854
07:30 AM	0	365	98	0	463	0	33	270	0	0	303	66	0	40	0	106	872
07:45 AM	0	328	79	0	407	0	33	249	0	0	282	89	0	30	0	119	808
Total	0	1292	310	0	1602	0	105	1084	0	0	1189	265	0	111	0	376	3167
08:00 AM	٥,	310	68	0	378	0	37	220	0	0	257	72	0	41	0	113	748
08:15 AM	0	285	70	0	355	0	34	196	0	0	230	68	0	40	0	108	693
08:30 AM	0	226	65	0	291	0	50	218	0	0	268	62	0	33	0	95	654
08:45 AM	0	238	76	0	314	0	43	208	0	0	251	62	0	40	0	102	667
Total	0	1059	279	0	1338	0	164	842	0	0	1006	264	0	154	0	418	2762
Grand Total	0	2999	714	0	3713	0	316	2492	0	0	2808	651	0	318	0	969	7490
Apprch %	0	80.8	19.2	0			11.3	88.7	0	0	1000000	67.2	0	32.8	0	1000	
Total %	0	40	9.5	0	49.6	0	4.2	33.3	0	0	37.5	8.7	0	4.2	0	12.9	

			ani Highwa outhbound			Westboun	Pillani Highway Northbound						ikea Avenu Eastbound				
Start Time	Left	Thru	Right	Peds	App. Total	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Tota
Peak Hour Analysis F	rom 06:00	AM to 08:4	5 AM - Pe	ak 1 of 1		L					L-market management of						
Peak Hour for Entire I	Intersection	Begins at	07:15 AM														
07:15 AM	0	359	72	0	431	0	20	314	0	0	334	67	0	22	0	89	854
07:30 AM	0	365	98	0	463	0	33	270	0	0	303	66	0	40	0	106	872
07:45 AM	0	328	79	0	407	0	33	249	0	0	282	89	0	30	0	119	808
08:00 AM	0	310	68	0	378	0	37	220	0	0	257	72	0	41	0	113	748
Total Volume	0	1362	317	0	1679	0	123	1053	0	. 0	1176	294	0	133	0	427	3282
% App. Total	0	81.1	18.9	0			10.5	89.5	0	0		68.9	0	31.1	0		
PHF	.000	.933	.809	.000	.907	.000	.831	.838	.000	.000	.880	.826	.000	.811	.000	.897	.941

Counmter:5676/5672 Counted By:BB and LM Weather:Clear

File Name: KulkiePM Site Code : 00000000 Start Date : 1/26/2011 Page No : 1

				17.7		0.0000000	Grou	ps Printed	- Unshift	ed	1000						
			. Kihei Roa Southbound					nihakoi St Vestbound					Kihei Roa Iorthbound			Eastboun	
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	App. Total	Int. Total
02:00 PM	3	49	0	0	52	11	0	14	4	29	0	88	28	3	119	0	200
02:15 PM	8	89	0	0	97	12	0	10	3	25	0	78	21	0	99	0	221
02:30 PM	6	78	0	0	84	20	0	8	3	31	0	100	34	2	136	0	251
02:45 PM	20	108	0	0	128	15	0	12	1	28	0	75	35	3	113	0	269
Total	37	324	0	0	361	58	0	44	11	113	0	341	118	8	467	0	941
03:00 PM	5	105	0	0	110	10	0	10	7	27	0	130	17	3	150	0	287
03:15 PM	6	99	0	0	105	7	0	15	4	26	0	135	10	0	145	0	276
03:30 PM	11	108	0	0	119	11	0	13	2	26	0	158	21	3	182	0	327
03:45 PM	5	136	0	0	141	20	0	7	2	29	0	147	- 11	0	158	0	328
Total	27	448	0	0	475	48	0	45	15	108	0	570	59	6	635	0	1218
04:00 PM	7	114	0	0	121	7	0	4	1	12	0	126	12	0	138	0	271
04:15 PM	5	89	0	0	94	14	0	7	2	23	0	142	14	1	157	0	274
04:30 PM	6	108	0	0	114	15	0	9	2	26	0	143	16	1	160	0	300
04:45 PM	7	132	0	0	139	13	0	14	4	31	0	133	17	2	152	0	322
Total	25	443	0	0	468	49	0	34	9	92	0	544	59	4	607	0	1167
05:00 PM	8	125	0	0	133	5	0	15	2	22	0	145	15	1	161	0	316
05:15 PM	6	106	0	0	112	11	. 0	6	9	26	0	134	22	8	164	0	302
05:30 PM	10	109	0	0	119	13	0	8	5	26	0	117	20	0	137	0	282
05:45 PM	4	94	0	0	.98	17	0	12	2	31	0	128	14	2	144	0	273
Total	28	434	0	0	462	46	0	41	18	105	0	524	71	11	606	0	1173
Grand Total	117	1649	0	0	1766	201	0	164	53	418	0	1979	307	29	2315	0	4499
Apprch %	6.6	93.4	0	0		48.1	0	39.2	12.7		0	85.5	13.3	1.3			
Total %	2.6	36.7	0	0	39.3	4.5	0	3.6	1.2	9.3	0	44	6.8	0.6	51.5	0	

			Kihel Roa			Kulanihakoi Street Westbound							. Kihei Roa Vorthbound			Eastboun	
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	App. Total	Int. Total
eak Hour Analysis Fr	om 02:00	PM to 05:4	5 PM - Pe	ak 1 of 1													
eak Hour for Entire Ir	ntersection	Begins at	04:30 PM														
04:30 PM	6	108	0	0	114	15	0	9	2	26	0	143	16	1	160	0	300
04:45 PM	7	132	0	0	139	13	0	14	4	31	0	133	17	2	152	0	322
05:00 PM	8	125	0	0	133	5	0	15	2	22	0	145	15	1	161	0	316
05:15 PM	6	106	0	0	112	11	0	6	9	26	0	134	22	8	164	0	302
Total Volume	27	471	0	0	498	44	0	44	17	105	0	555	70	12	637	0	1240
% App. Total	5.4	94.6	0	0		41.9	0	41.9	16.2		0	87.1	11	1.9			
PHF	.844	.892	.000	.000	.896	.733	.000	.733	.472	.847	.000	.957	.795	.375	.971	.000	.963

Wilson Okamoto Corporation 1907 S. Beretania Street Suite 400 Honolulu, Hi 96826

Counter:5676/5672 Counted By:BB and LM Weather:Clear

File Name: KulkieAM Site Code : 00000000 Start Date : 1/26/2011 Page No : 1

							Grou	ps Printed	- Unshift	ed							
			. Kihei Roa Southbound					inihakol St Westbound					. Kihei Roa Northbound			Eastboun	
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	App. Total	Int. Total
06:00 AM	0	17	0	1	18	0	0	5	0	5	0	38	0	0	38	0	61
06:15 AM	1	33	0	1	35	3	0	8	0	11	0	47	0	0	47	0	93
06:30 AM	1	47	0	4	52	3	0	5	3	11	0	69	5	0	74	0	137
06:45 AM	2	66	0	0	68	8	0	13	1	22	0	89	6	0	95	0	185
Total	4	163	0	6	173	14	0	,31	4	49	0	243	-11	0	254	0	476
07:00 AM	1	46	0	1	48	10	0	16	3	29	0	98	6	0	104	0	181
07:15 AM	4	84	0	0	88	10	. 0	12	4	26	0	93	9	0	102	0	216
07:30 AM	4	81	0	0	85	11	0	18	4	33	0	104	10	0	114	0	232
07:45 AM	2	83	0	0	85	8	0	13	8	29	0	111	14	0	125	0	239
Total	11	294	0	1	306	39	0	59	19	117	0	406	39	0	445	0	868
08:00 AM	11	76	0	1	88	14	0	17	4	35	0	112	14	0	126	0	249
08:15 AM	5	80	0	0	85	13	0	15	6	34	0	85	7	2	94	0	213
08:30 AM	13	64	0	0	77	10	0	15	4	29	0	83	13	2	98	0	204
08:45 AM	12	60	0	2	74	13	0	5	8	26	0	70	34	1	105	.0	205
Total	41	280	0	3	324	50	. 0	52	22	124	0	350	68	5	423	0	871
Grand Total	56	737	0	10	803	103	0	142	45	290	0	999	118	5	1122	0	2215
Approh %	7	91.8	0	1.2		35.5	0	49	15.5	(1000)	0	89	10.5	0.4			
Total %	2.5	33.3	0	0.5	36.3	4.7	0	6.4	2	13.1	0	45.1	5.3	0.2	50.7	0	

			Kihei Road outhbound			Kulanihakoi Street Westbound						. Kihei Roa Vorthbound			Eastboun		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	App. Total	Int. Total
eak Hour Analysis Fr	rom 06:00	AM to 08:4	5 AM - Pe	ak 1 of 1													
eak Hour for Entire In	ntersection	Begins at	07:15 AM														
07:15 AM	4	84	0	0	88	10	0	12	4	26	0	93	9	0	102	0	216
07:30 AM	4	81	0	0	85	11	0	18	4	33	0	104	10	0	114	0	232
07:45 AM	2	83	0	0	85	8	0	13	8	29	0	111	14	0	125	0	239
08:00 AM	11	76	0	1	88	14	0	17	4	35	0	112	14	0	126	0	249
Total Volume	21	324	0	1	346	43	0	60	20	123	0	420	47	0	467	0	936
% App. Total	6.1	93.6	0	0.3		35	0	48.8	16.3		0	89.9	10.1	0	0.111000		
PHF	.477	.964	.000	.250	.983	.768	.000	.833	.625	.879	.000	.938	.839	.000	.927	.000	.940

APPENDIX B

LEVEL OF SERVICE DEFINITIONS

LEVEL OF SERVICE DEFINITIONS

LEVEL-OF-SERVICE CRITERIA FOR SIGNALIZED INTERSECTIONS

Level of Service (LOS) for signalized intersections is defined in terms of delay, which is a measure of driver discomfort, frustration, fuel consumption, and increased travel time. Specifically, level-of-service (LOS) criteria are stated in terms of the average control delay per vehicle, typically a 15-min analysis period. The criteria are given in the following table.

Table 1: Level-of-Service Criteria for Signalized Intersections

Level of Service	Control Delay per Vehicle (sec/veh)
A	≤10.0
В	>10.0 and ≤ 20.0
C	>20.0 and ≤ 35.0
D	>35.0 and ≤ 55.0
E	>55.0 and ≤ 80.0
F	>80.0

Delay is a complex measure and depends on a number of variables, including the quality of progression, the cycle length, the green ratio, and the v/c ratio for the lane group.

Level of Service A describes operations with low control delay, up to 10 sec per vehicle. This level of service occurs when progression is extremely favorable and most vehicles arrive during the green phase. Many vehicles do not stop at all. Short cycle lengths may tend to contribute to low delay values.

Level of Service B describes operations with control delay greater than 10 and up to 20 sec per vehicle. This level generally occurs with good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of delay.

Level of Service C describes operations with control delay greater than 20 and up to 35 sec per vehicle. These higher delays may result from only fair progression, longer cycle lengths, or both. Individual cycle failures may begin to appear at this level. Cycle failure occurs when a given green phase does not serve queued vehicles and overflows occur. The number of vehicles stopping is significant at this level, though many still pass through the intersection without stopping.

Level of Service D describes operations with control delay greater than 35 and up to 55 see per vehicle. At level of service D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high v/c ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.

[&]quot;Highway Capacity Manual," Transportation Research Board, 2000.

Level of Service E describes operation with control delay greater than 55 and up to 80 sec per vehicle. These high delay values generally indicate poor progression, long cycle lengths, and high v/c ratios. Individual cycle failures are frequent.

Level of Service F describes operations with control delay in excess of 80 sec per vehicle. This level, considered to be unacceptable to most drivers, often occurs with oversaturation, that is, when arrival flow rates exceed the capacity lane groups. It may also occur at high v/c ratios with many individual cycle failures. Poor progression and long cycle lengths may also contribute significantly to high delay levels.

LEVEL OF SERVICE DEFINITIONS

LEVEL-OF-SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS

Level of Service (LOS) criteria are given in Table 1. As used here, control delay is defined as the total clapsed time from the time a vehicle stops at the end of the queue to the time required for the vehicle to travel from the last-in-queue position to the first-in-queue position, including deceleration of vehicles from free-flow speed to the speed of vehicles in the queue.

The average total delay for any particular minor movement is a function of the service rate or capacity of the approach and the degree of saturation. If the degree of saturation is greater than about 0.9, average control delay is significantly affected by the length of the analysis period.

Table 1: Level-of-Service Criteria for Unsignalized Intersections

Level of Service	Average Control Delay (Sec/Veh)
A	≤10.0
В	>10.0 and ≤ 15.0
C	>15.0 and ≤ 25.0
D	>25.0 and ≤ 35.0
E	>35.0 and ≤ 50.0
F	>50.0

APPENDIX C

CAPACITY ANALYSIS CALCULATIONS EXISTING PEAK HOUR TRAFFIC ANALYSIS

HCM Unsignalized Intersection Capacity Analysis 3: Kaonoulu & Piilani

5/3/2011

	*	*	4	†	1	1					
Movement	EBU	EBR	NBL	NBTO	SBT	SBR					- A. S. S. S.
Lane Configurations	ሻ	7	*	11	^	7					
Volume (veh/h)	28	209	61	1306	1458	16					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.84	0.84	0.95	0.95	0.91	0.91					
Hourly flow rate (vph)	33	249	64	1375	1602	18					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				TWLTL	TWLTL						
Median storage veh)				2	2						
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2418	801	1602								
vC1, stage 1 conf vol	1602										
vC2, stage 2 conf vol	816										
vCu, unblocked vol	2418	801	1602								
tC, single (s)	*5.8	*5.9	4.1								
tC, 2 stage (s)	4.8										
tF(s)	*2.5	*2.3	2.2								
p0 queue free %	86	54	84								
cM capacity (veh/h)	245	538	404								
Dravilan, Sane #	£818	EB/2	ENB	NB12	(ANB)31	SBI	SB 2	SB3			
/olume Total	33	249	64	687	687	801	801	18			
Volume Left	33	0	64	0	0	0	0	0			
Volume Right	0	249	0	0	0	0	0	18			
SH	245	538	404	1700	1700	1700	1700	1700			
Volume to Capacity	0.14	0.46	0.16	0.40	0.40	0.47	0.47	0.01			
Queue Length 95th (ft)	12	60	14	0	0	0	0	0			
Control Delay (s)	22.0	17.3	15.6	0.0	0.0	0.0	0.0	0.0			
Lane LOS	C	C	C								
Approach Delay (s)	17.9		0.7			0.0					
Approach LOS	C										
intersection Summary :	i santini	HO M				121213				a property	
Average Delay			1.8								
Intersection Capacity Utiliza	ation		57.9%	IC	U Level	of Service			В		
Analysis Period (min)			15								

User Entered Value

Afovement ane Configurations /olume (veh/h) Sign Control Grade Peak Hour Factor	22 Stop 0% 0.79 28	107 0.79	NBL 기 118	1475 Free	SBT 1547	SBR				4 18 A
ane Configurations /olume (veh/h) Sign Control Grade Peak Hour Factor	22 Stop 0% 0.79	0.79	118	^	^					
Sign Control Grade Peak Hour Factor	Stop 0% 0.79	0.79	EEE!		1547					
Grade Peak Hour Factor	0% 0.79			Free		107				
Peak Hour Factor	0.79				Free					
				0%	0%					
Levely flow sets (costs)	28		0.87	0.87	0.87	0.87				
Hourly flow rate (vph)		135	136	1695	1778	123				
Pedestrians										
ane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type				TWLTL	TWLTL					
Median storage veh)				2	2					
Jpstream signal (ft)										
X, platoon unblocked										
C, conflicting volume	2897	889	1778							
C1, stage 1 conf vol	1778									
C2, stage 2 conf vol	1119								2017.00	
Cu, unblocked vol	2897	889	1778							
C, single (s)	*5.8	*5.9	4.1							
C, 2 stage (s)	4.8									
F (s)	*2.5	*2.3	2.2							
00 queue free %	84	72	61							
cM capacity (veh/h)	172	478	345							
Direction (Lane)#	EB1	EB2	NB 1	NB2	NB 8	SBI	SB12	SB/8		
Volume Total	28	135	136	848	848	889	889	123		
/olume Left	28	0	136	0	0	0	0	0		
/olume Right	0	135	0	0	0	0	0	123		
SH	172	478	345	1700	1700	1700	1700	1700		
/olume to Capacity	0.16	0.28	0.39	0.50	0.50	0.52	0.52	0.07		
Queue Length 95th (ft)	14	29	45	0	0	0	0	0		
Control Delay (s)	29.9	15.5	22.0	0.0	0.0	0.0	0.0	0.0		
ane LOS	D	С	С							
Approach Delay (s)	17.9		1.6			0.0				
Approach LOS	C									
ntersection Summary		908/618	P. S. Color							
Average Delay			1.5							
ntersection Capacity Utiliza	ation		60.2%		CU Level	of Service			В	
Analysis Period (min)			15							

User Entered Value

	•	•	4	†	1	1				
Movement	EBL	EBR	NBL	NBT	SBT	SBR				
Lane Configurations	ሻ	7	*	**	^	7				
Volume (veh/h)	72	135	49	1295	1621	46				
Sign Control	Stop			Free	Free					
Grade	0%			0%	0%					
Peak Hour Factor	0.85	0.85	0.94	0.94	0.93	0.93				
Hourly flow rate (vph)	85	159	52	1378	1743	49				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type				None	TWLTL					
Median storage veh)					2					
Upstream signal (ft)										
pX, platoon unblocked										
vC, conflicting volume	2536	872	1743							
vC1, stage 1 conf vol	1743									
vC2, stage 2 conf vol	793							1,004		
vCu, unblocked vol	2536	872	1743							
tC, single (s)	*5.8	*5.9	4.1							
tC, 2 stage (s)	4.8									
tF (s)	*2.5	*2.3	2.2							
p0 queue free %	61	68	85							
cM capacity (veh/h)	218	489	357							
Direction: Laper4	MEB4	EBIR	NB4	NB ₁ 2	NB 6	SBA	\$8.2	SB 3	A CHARLES	
Volume Total	85	159	52	689	689	872	872	49		
Volume Left	85	0	52	0	0	0	0	0		
Volume Right	. 0	159	0	0	0	0	0	49		
cSH	218	489	357	1700	1700	1700	1700	1700		
Volume to Capacity	0.39	0.32	0.15	0.41	0.41	0.51	0.51	0.03		
Queue Length 95th (ft)	43	35	13	0	0	0	0	0		
Control Delay (s)	31.6	15.9	16.8	0.0	0.0	0.0	0.0	0.0		
Lane LOS	D	С	С							
Approach Delay (s)	21.3		0.6			0.0				
Approach LOS	С									
Intersection Summary										
Average Delay			1.8		20 Desirabel (**)					
Intersection Capacity Utilization	n		57.6%		CU Level	of Service			В	
Analysis Period (min)			15							

User Entered Value

	20	

	•	•	4	†	Į.	1					
Mavement	EBL	EBR	NBL	NBT	SBT	SBR		NE SX			
Lane Configurations	ሻ	#	*	^	11	7					
Volume (veh/h)	28	93	68	1565	1549	105					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.76	0.76	0.92	0.92	0.88	0.88					
Hourly flow rate (vph)	37	122	74	1701	1760	119					
Pedestrians Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				None	TWLTL						
Median storage veh)					2						
Upstream signal (ft)											
X, platoon unblocked											
C, conflicting volume	2759	880	1760								
C1, stage 1 conf vol	1760										
C2, stage 2 conf vol	998										
Cu, unblocked vol	2759	880	1760								
C, single (s)	*5.8	*5.9	4.1								
C, 2 stage (s)	4.8										
F (s)	*2.5	*2.3	2.2								
00 queue free %	82	75	79								
cM capacity (veh/h)	201	484	351								
Direction Barlett	EB 1	EB2	NB/4	MB:2	MB ₁₃	SBH	\$6.2	SB3		Zustaniki Karananiki	
/olume Total	37	122	74	851	851	880	880	119			
Volume Left	37	0	74	0	0	0	0	0			
Volume Right	0	122	0	0	0	0	0	119			
SH	201	484	351	1700	1700	1700	1700	1700			
Volume to Capacity	0.18	0.25	0.21	0.50	0.50	0.52	0.52	0.07			
Queue Length 95th (ft)	16	25	20	0	0	0	0	0			
Control Delay (s)	26.9	14.9	18.0	0.0	0.0	0.0	0.0	0.0			
Lane LOS	D	В	С								
Approach Delay (s)	17.7		0.7			0.0					
Approach LOS	С										
htersection Summary					A STATE					Nonless Manual Park	
Average Delay			1.1								
Intersection Capacity Utiliza	ation		57.6%	IC	U Level	of Service			В		
Analysis Period (min)			15								

^{*} User Entered Value

	1	•	4	†	↓	1	
Movement	EBL	EBR_	NBL	NBT	SBT	SBR	
Lane Configurations		7"		† †	44	7*	
Volume (veh/h)	0	23	0	1344	1674	81	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.58	0.58	0.88	0.88	0.91	0.91	
Hourly flow rate (vph)	0	40	0	1527	1840	89	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2603	920	1840				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2603	920	1840				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)							
tF(s)	3.5	*2.3	2.2				
p0 queue free %	100	91	100				
cM capacity (veh/h)	20	458	327				
Directions Laners	E CEBA	NB 1	NB2	SBI	SB2	SB3	
Volume Total	40	764	764	920	920	89	
Volume Left	0	0	0	0	0	0	
Volume Right	40	0	0	0	0	89	
cSH	458	1700	1700	1700	1700	1700	
Volume to Capacity	0.09	0.45	0.45	0.54	0.54	0.05	
Queue Length 95th (ft)	7	0	0	0	0	0	
Control Delay (s)	13.6	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В						
Approach Delay (s)	13.6	0.0		0.0			
Approach LOS	В						
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utiliza	ation		54.0%	IC	U Level	of Service	A
Analysis Period (min)			15				

15

HCM Unsignalized Intersection Capacity Analysis 7: E. Waipuilani & Piilani

Intersection Capacity Utilization Analysis Period (min)

User Entered Value

0.2 50.5%

15

	*	7	4	†	1	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBB TO THE SECOND SECOND	
Lane Configurations		7		^	44	7	
Volume (veh/h)	0	32	0	1633	1542	100	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.57	0.57	0.95	0.95	0.96	0.96	
Hourly flow rate (vph) Pedestrians	0	56	0	1719	1606	104	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2466	803	1606				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2466	803	1606				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)							
tF (s)	3.5	*2.3	2.2				
p0 queue free %	100	90	100				
cM capacity (veh/h)	25	537	403				
Direction; Lanet#3, 30, 10, 1	EB 15	NB 1	NB2	S8 1	SB126	SBS:	
Volume Total	56	859	859	803	803	104	
Volume Left	0	0	0	0	0	0	
Volume Right	56	0	0	0	0	104	
cSH	537	1700	1700	1700	1700	1700	
Volume to Capacity	0.10	0.51	0.51	0.47	0.47	0.06	
Queue Length 95th (ft)	.9	0	0	0	0	0	
Control Delay (s)	12.5	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В						
Approach Delay (s)	12.5	0.0		0.0			
Approach LOS	В						

ICU Level of Service

. 1	Jser	En	tered	Value
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Average Delay Intersection Capacity Utilization Analysis Period (min)

	•	*	1	1	1	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR,	
Lane Configurations	M	7	*1	44	**	7	
Volume (vph)	294	133	123	1050	1376	321	
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.90	0.90	0.88	0.88	0.91	0.91	25-57-170-1
Adi. Flow (vph)	327	148	140	1193	1512	353	
RTOR Reduction (vph)	0	113	0	0	0	0	
Lane Group Flow (vph)	327	35	140	1193	1512	353	
Turn Type	021	Perm	Prot	1100	1012	Free	HWIPPENINE.
Protected Phases	4	E26252555	5	2	6		
Permitted Phases	10/20/20/20/20/20/20/20/20/20/20/20/20/20	4			0	Free	
Actuated Green, G (s)	28.3	28.3	14.5	80.1	60.6	118.4	
Effective Green, g (s)	28.3	28.3	14.5	80.1	60.6	118.4	
Actuated g/C Ratio	0.24	0.24	0.12	0.68	0.51	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	1.00	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	423		228			1007	
		378		2520	1907	1667	Personal Contraction
v/s Ratio Prot	c0.18	0.00	c0.08	0.32	c0.41	PERSONAL PROPERTY.	
v/s Ratio Perm	0.77	0.02	0.04	0.45	0.70	0.21	
v/c Ratio	0.77	0.09	0.61	0.47	0.79	0.21	
Uniform Delay, d1	42.1	35.1	49.3	9.1	23.7	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	8.5	0.1	4.8	0.1	2.3	0.3	
Delay (s)	50.6	35.2	54.1	9.3	26.1	0.3	
Level of Service	D	D	D	Α	C	Α	
Approach Delay (s)	45.8			14.0	21.2		
Approach LOS	D			В	C		
Intersection Summary		10000				A CONTRACTOR OF	
HCM Average Control Dela	v		21.8	Н	CM Leve	of Service	
HCM Volume to Capacity ra			0.76				
Actuated Cycle Length (s)	227200000000000000000000000000000000000		118.4	S	um of los	t time (s)	
Intersection Capacity Utiliza	tion		71.4%			of Service	
Analysis Period (min)	West Committee		15	N. Carlotte	O LEVEL	OI OCIVICE	
C Critical Lane Group			September 19				

-	10	20	24	

	•	*	4	1	Ţ	4	
Movement	EBL	EI8A	MBL	NBT	SBT	SBR	
Lane Configurations	7	7	*	44	^	7	
Volume (vph)	270	277	233	1363	1200	374	
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Fit Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.89	0.89	0.95	0.95	0.96	0.96	
Adj. Flow (vph)	303	311	245	1435	1250	390	
RTOR Reduction (vph)	0	239	0	0	0	0	
Lane Group Flow (vph)	303	72	245	1435	1250	390	
Turn Type		Perm	Prot		7	Free	
Protected Phases	4		5	2	6	asinowete	
Permitted Phases	Mary delicated to the party	4	NO-THE MAIL	200000	Jan Politica	Free	
Actuated Green, G (s)	25.7	25.7	20.9	75.1	49.2	110.8	
Effective Green, g (s)	25.7	25.7	20.9	75.1	49.2	110.8	
Actuated g/C Ratio	0.23	0.23	0.19	0.68	0.44	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	THE STATE OF THE S	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	411	367	351	2525	1654	1667	
v/s Ratio Prot	c0.17	TENNES	c0.13	0.39	c0.34	NOT THE RESERVE	
v/s Ratio Perm	00.11	0.05	00.10	0.00	00.04	0.23	
v/c Ratio	0.74	0.20	0.70	0.57	0.76	0.23	
Uniform Delay, d1	39.4	34.2	42.0	9.4	25.8	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	6.8	0.3	6.0	0.3	2.0	0.3	
Delay (s)	46.2	34.5	48.0	9.7	27.8	0.3	
Level of Service	D	C	D	A	C	A	
Approach Delay (s)	40.3			15.2	21.3		
Approach LOS	D			В	C		
Intersection Summary		O PARTIE DA				22 C C C C C C C C C C C C C C C C C C	
HCM Average Control Dela	V	R-72-52	21.7	Н	CM Leve	of Service	C
HCM Volume to Capacity ra			0.74	SAREW.	2010	B. 18 (8)	
Actuated Cycle Length (s)	- gransmatids		110.8	S	um of los	t time (s)	15.0
Intersection Capacity Utiliza	ation		71.2%			of Service	Construction Construction
Analysis Period (min)	MY MARKET		15	The state of the s	JO LOVOI	OI OOI VICO	
c Critical Lane Group							

	1	*	1	1	1	ţ	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	4		ሻ	↑	
Volume (veh/h)	43	60	420	47	21	324	
Sign Control	Stop		Free			Free	
Grade	. 0%		0%			0%	
Peak Hour Factor	0.83	0.83	0.93	0.93	0.98	0.98	
Hourly flow rate (vph) Pedestrians Lane Width (ft)	52	72	452	51	21	331	
Walking Speed (ft/s) Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)					POGRADINA.		
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	850	477			502		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	850	477			502		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF(s)	3.5	3.3			2.2		
p0 queue free %	84	88			98		
cM capacity (veh/h)	324	588			1062		
Direction, Janear 18	WB#	WB 2	NBS.	SBI	SB 2		
Volume Total	52	72	502	21	331		
Volume Left	52	0	0	21	0		
Volume Right	0	72	51	0	0		
cSH	324	588	1700	1062	1700	·	
Volume to Capacity	0.16	0.12	0.30	0.02	0.19		
Queue Length 95th (ft)	14	10	0	2	0		
Control Delay (s)	18.2	12.0	0.0	8.5	0.0		
Lane LOS	C	В		A			
Approach Delay (s) Approach LOS	14.6 B		0.0	0.5			
ntersection Summary:		Arm (Ca)				Chipping Charles Const.	
Average Delay			2.0				
Intersection Capacity Utiliza	ation		35.3%	IC	U Level	f Service A	
Analysis Period (min)			15				

	1	*	1	-	1	↓		
Movement	WBL	WBR	E ANBA	NBR:	SBL	SBI		
Lane Configurations	1	#	1>		7	↑		
Volume (veh/h)	56	27	558	53	23	447		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.78	0.78	0.96	0.96	0.84	0.84		
Hourly flow rate (vph) Pedestrians	72	35	581	55	27	532		
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	1196	609			636			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	1196	609			636			
tC, single (s)	*5.4	*5.2			4.1			
tC, 2 stage (s)								
tF(s)	*2.5	*2.3			2.2			
p0 queue free %	79	96			97			
cM capacity (veh/h)	342	784			947			
Direction Banetic 18	WB 1	WB?	NB41	SB 1	SBI2			
Volume Total	72	35	636	27	532			
Volume Left	72	0	0	27	0			
Volume Right	0	35	55	0	0			
cSH	342	784	1700	947	1700			
Volume to Capacity	0.21	0.04	0.37	0.03	0.31			
Queue Length 95th (ft)	19	3	0	2	0			
Control Delay (s)	18.3	9.8	0.0	8.9	0.0			
Lane LOS	C	Α		Α				
Approach Delay (s)	15.5		0.0	0.4				
Approach LOS	C							
Intersection Summary		1978 W	(18 A)				left, tellfligt, sa	
Average Delay			1.5					
Intersection Capacity Utiliza	ation		42.6%	IC	U Level	of Service	A	
Analysis Period (min)			15					

User Entered Value

Existing PM Peak 4/20/2011 Baseline

Synchro 7 - Report Page 5

APPENDIX D

CAPACITY ANALYSIS CALCULATIONS PROJECTED YEAR 2015 PEAK HOUR TRAFFIC ANALYSIS WITHOUT PROJECT

	1	*	4	1	Ţ	4				
Movements	EBL	EBR	NBL	NBT	SBT	SBA	CT TEMPTE	100		
Lane Configurations	ሻ	7	*	44	44	7		Para Personal Anthony		ACCEPTABLE OF THE ST
Volume (veh/h)	28	209	61	1358	1516	16				
Sign Control	Stop			Free	Free					
Grade	0%			0%	0%					
Peak Hour Factor	0.84	0.84	0.95	0.95	0.91	0.91				
Hourly flow rate (vph)	33	249	64	1429	1666	18				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type				TWLTL	TWLTL					
Median storage veh)				2	2					
Upstream signal (ft)										
pX, platoon unblocked										
vC, conflicting volume	2509	833	1666							
vC1, stage 1 conf vol	1666									
vC2, stage 2 conf vol	843									
vCu, unblocked vol	2509	833	1666							
tC, single (s)	*5.8	*5.9	4.1							
tC, 2 stage (s)	4.8									
tF (s)	*2.5	*2.3	2.2							
p0 queue free %	85	52	83							
cM capacity (veh/h)	230	515	382							
Direction (Pane # 5 %)	SE FB#	MEB/2	NBdi	NB.2	NB3	SBJ	SB:2	SB 3		
Volume Total	33	249	64	715	715	833	833	18		
Volume Left	33	0	64	0	0	0	0	0		
Volume Right	0	249	0	0	0	0	0	18		
cSH	230	515	382	1700	1700	1700	1700	1700		
Volume to Capacity	0.15	0.48	0.17	0.42	0.42	0.49	0.49	0.01		
Queue Length 95th (ft)	12	65	15	0	0	0	0	0		
Control Delay (s)	23.3	18.3	16.3	0.0	0.0	0.0	0.0	0.0		
Lane LOS	С	C	С							
Approach Delay (s)	18.9		0.7			0.0				
Approach LOS	С									
intersection(Summary							70%为	History		
Average Delay			1.8							
Intersection Capacity Utiliza	ation		59.4%	- 1	CU Level	of Service			В	
Analysis Period (min)			15							

User Entered Value

	•	*	4	1	↓	1				
Movement	EBL	EBR	NBL	NBT	SBT	SBR				
Lane Configurations		7	ሻ	^	^	7	vere			
Volume (veh/h)	22	107	118	1534	1609	107				
Sign Control	Stop			Free	Free					
Grade	0%			0%	0%					
Peak Hour Factor	0.79	0.79	0.87	0.87	0.87	0.87				
Hourly flow rate (vph) Pedestrians Lane Width (ft) Walking Speed (ft/s)	28	135	136	1763	1849	123				
Percent Blockage Right turn flare (veh)										
Median type				TWLTL						
Median storage veh)				2	2					
Upstream signal (ft)										
pX, platoon unblocked										
vC, conflicting volume	3002	925	1849							
vC1, stage 1 conf vol	1849									
vC2, stage 2 conf vol	1153									
vCu, unblocked vol	3002	925	1849							
C, single (s)	*5.8	*5.9	4.1							
C, 2 stage (s)	4.8									
tF (s)	*2.5	*2.3	2.2							
p0 queue free %	82	70	58							
cM capacity (veh/h)	159	455	324							
Direction Lane #	EB,1	EB/2	NB IN	NB,25	NB-9	SBI	SBIZE	SB 3	\$ \$75.68 kg	
Volume Total	28	135	136	882	882	925	925	123		
Volume Left	28	0	136	0	0	0	0	0		
Volume Right	0	135	0	0	0	0	0	123		
cSH	159	455	324	1700	1700	1700	1700	1700		
Volume to Capacity	0.18	0.30	0.42	0.52	0.52	0.54	0.54	0.07		
Queue Length 95th (ft)	15	31	50	0	0	0	0	0		
Control Delay (s)	32.4	16.2	23.9	0.0	0.0	0.0	0.0	0.0		
Lane LOS	D	С	C							
Approach Delay (s)	19.0		1.7			0.0				
Approach LOS	С									
Intersection(Summary										
Average Delay			1.6							
Intersection Capacity Utiliza	ation		61.8%	10	CU Level	of Service	•		В	
Analysis Period (min)			15							

User Entered Value

	•	•	4	†	Į.	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	Alleria - Cara Cara Cara Cara Cara Cara Cara C
Lane Configurations	*	7	ሻ	^	^	7	
Volume (veh/h)	72	135	49	1347	1679	46	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.85	0.85	0.94	0.94	0.93	0.93	
Hourly flow rate (vph)	85	159	52	1433	1805	49	
Pedestrians			SACRESS SERVICES				
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	TWLTL		
Median storage veh)					2		
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2626	903	1805				
vC1, stage 1 conf vol	1805	STATE OF THE STATE OF THE					
vC2, stage 2 conf vol	821						
vCu, unblocked vol	2626	903	1805				
tC, single (s)	*5.8	*5.9	4.1				
tC, 2 stage (s)	4.8						
tF (s)	*2.5	*2.3	2.2				
0	Annual Additional Control	The state of the last	The Period of the last				

205	469	337							
EBAN	EB ₁₂	ANB dire	NB 2	MBI3:	SBI	SB.2	SB-8		
85	159	52	716	716	903	903	49		
85	0	52	0	0	0	0	0		
0	159	0	0	0	0	0	49		
205	469	337	1700	1700	1700	1700	1700		
0.41	0.34	0.15	0.42	0.42	0.53	0.53	0.03		
47	37	14	0	0	0	0	0		
34.5	16.6	17.6	0.0	0.0	0.0	0.0	0.0		
D	C	C							
22.8		0.6			0.0				
C									
	85 85 0 205 0.41 47 34.5 D	85 159 85 0 0 159 205 469 0.41 0.34 47 37 34.5 16.6 D C 22.8	85 159 52 85 0 52 0 159 0 205 469 337 0.41 0.34 0.15 47 37 14 34.5 16.6 17.6 D C C 22.8 0.6	85 159 52 716 85 0 52 0 0 159 0 0 205 469 337 1700 0.41 0.34 0.15 0.42 47 37 14 0 34.5 16.6 17.6 0.0 D C C 22.8 0.6	85 159 52 716 716 85 159 52 716 716 85 0 52 0 0 0 159 0 0 0 205 469 337 1700 1700 0.41 0.34 0.15 0.42 0.42 47 37 14 0 0 34.5 16.6 17.6 0.0 0.0 D C C C 22.8 0.6 0	85 159 52 716 716 903 85 0 52 716 716 903 85 0 52 0 0 0 0 0 159 0 0 0 0 0 0 205 469 337 1700 1700 1700 1700 0.41 0.34 0.15 0.42 0.42 0.53 47 47 37 14 0 0 0 0 34.5 16.6 17.6 0.0 0.0 0.0 0 D C C C 22.8 0.6 0.0 0	B11 FB,2 NB1 NB2 NB3 SB1 SB2 85 159 52 716 776 903 903 85 0 52 0 0 0 0 0 0 159 0 0 0 0 0 0 0 205 469 337 1700 1700 1700 1700 1700 1700 170 150 0.42 0.42 0.53 0.53 0.53 47 37 14 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BB1 BB2 NB4 NB2 NB 8 SB1 SB2 SB3 85 159 52 716 716 903 903 49 85 0 52 0 0 0 0 0 0 159 0 0 0 0 0 49 205 469 337 1700 1700 1700 1700 1700 0.41 0.34 0.15 0.42 0.42 0.53 0.53 0.03 47 37 14 0 0 0 0 0 34.5 16.6 17.6 0.0 0.0 0.0 0.0 0.0 D C C C 22.8 0.6 0.0 0.0	BB11 EB2 NB4 NB2 NB 8 SB1 SB2 SE3 85 159 52 716 716 903 903 49 85 0 52 0 0 0 0 0 0 159 0 0 0 0 0 49 205 469 337 1700 1700 1700 1700 1700 0.41 0.34 0.15 0.42 0.42 0.53 0.53 0.03 47 37 14 0 0 0 0 0 34.5 16.6 17.6 0.0 0.0 0.0 0.0 0.0 D C C C 22.8 0.6 0.0 0.0

Average Delay	1.8		
Intersection Capacity Utilization	59.1%	ICU Level of Service	В
Analysis Period (min)	15		

User Entered Value

p0 queue free %

	۶	•	4	1	1	1					
Movement	EBL	EBR	MBU	NBT	4 SBT	SBR				Zeidel	
Lane Configurations	ሻ	7	7	^	^	7			Control of the second		
Volume (veh/h)	28	93	68	1624	1611	105					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.76	0.76	0.92	0.92	0.88	0.88					
Hourly flow rate (vph)	37	122	74	1765	1831	119					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				None	TWLTL						
Median storage veh)					2						
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2861	915	1831								
vC1, stage 1 conf vol	1831										
vC2, stage 2 conf vol	1030										
vCu, unblocked vol	2861	915	1831								
tC, single (s)	*5.8	*5.9	4.1								
tC, 2 stage (s)	4.8										
tF(s)	*2.5	*2.3	2.2								
p0 queue free %	80	73	78								
cM capacity (veh/h)	187	461	330								
Direction: (ane #	EBH	4FB:2	NB 1	NB/2	NB3	SBIT	SB/2	SB3			
Volume Total	37	122	74	883	883	915	915	119			
Volume Left	37	0	74	0	0	0	0	0			
Volume Right	0	122	0	0	0	0	0	119			
cSH	187	461	330	1700	1700	1700	1700	1700			
Volume to Capacity	0.20	0.27	0.22	0.52	0.52	0.54	0.54	0.07			
Queue Length 95th (ft)	18	26	21	0	0	0	0	0			
Control Delay (s)	29.0	15.6	19.1	0.0	0.0	0.0	0.0	0.0			
Lane LOS	D	C	C								
Approach Delay (s)	18.7		0.8			0.0					
Approach LOS	С										
Intersection Summary			100								
Average Delay			1.1								
Intersection Capacity Utilization	П		59.2%		CU Level	of Service	1000		В		
Analysis Period (min)			15								

User Entered Value

HCM Unsignalized Intersection Capacity Analysis

5: Kulanihakoi & Piilani

	•	•	1	†	1	4	
Movement -	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	SZCentines ico	7		11	† †	7	
Volume (veh/h)	0	23	0	1396	1732	81	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.58	0.58	0.88	0.88	0.91	0.91	
Hourly flow rate (vph)	0	40	0	1586	1903	89	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2696	952	1903				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2696	952	1903				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)							
tF(s)	3.5	*2.3	2.2				
p0 queue free %	100	91	100				
cM capacity (veh/h)	17	439	309				
Direction Haners	EBA	INB.	ENB 2	SB 1	SB 2	SB3 7 POLETICAL SANCES	
Volume Total	40	793	793	952	952	89	
Volume Left	0	0	0	0	0	0	
Volume Right	40	0	0	0	0	89	
cSH	439	1700	1700	1700	1700	1700	
Volume to Canacity	0.00	0.47	0.47	0.56	0.56	0.05	

Lane LOS	В			
Approach Delay (s)	14.0	0.0	0.0	
Approach LOS	В			
Intersection Summary				
Average Delay		0.2		
Intersection Capacity Utilization	1	55.5%	ICU Level of Service	В
Analysis Period (min)		15		

0.09 0.47 0.47 0.56 0.56 0.05 0

14.0 0.0 0.0 0.0 0.0 0.0

Volume to Capacity Queue Length 95th (ft)

Control Delay (s)

	•	*	4	†	1	1	
Movement	EBL	KeEBRE	NBL-	NBT.	SBT	SBR	
Lane Configurations	17657	7		**	^	7	
Volume (veh/h)	0	32	0	1692	1604	100	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.57	0.57	0.95	0.95	0.96	0.96	
Hourly flow rate (vph)	0	56	0	1781	1671	104	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2561	835	1671				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2561	835	1671				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)							
tF(s)	3.5	*2.3	2.2				
p0 queue free %	100	89	100		de la gradie de descripto		
cM capacity (veh/h)	21	514	380				
Direction, Lane # 327	EBdy	NB 1	NBI2	«SB 1»	SB.2	SB3	
Volume Total	56	891	891	835	835	104	
Volume Left	0	0	0	0	0	0	
Volume Right	56	0	0	0	0	104	
cSH	514	1700	1700	1700	1700	1700	
Volume to Capacity	0.11	0.52	0.52	0.49	0.49	0.06	
Queue Length 95th (ft)	9	0	0	0	0	0	
Control Delay (s)	12.9	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В						
Approach Delay (s)	12.9	0.0		0.0			
Approach LOS	В						
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utiliza	ition		52.1%	IC	U Level	of Service	A
Analysis Period (min)			15				

User Entered Value

HCM Unsignalized Intersection Capacity Analysis

7: E. Waipuilani & Piilani

User Entered Value

Page 4

	•	*	1	1	†	1		
Movement	EBE	EBR	MBL	NBT	SBT	SBR		
Lane Configurations	*	7	ሻ	11	† †	7		
Volume (vph)	294	133	123	1102	1434	321		
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000		
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00		
Frt	1.00	0.85	1.00	1.00	1.00	0.85		
Fit Protected	0.95	1.00	0.95	1.00	1.00	1.00		
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667		
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00		
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667		
Peak-hour factor, PHF	0.90	0.90	0.88	0.88	0.91	0.91		
Adj. Flow (vph)	327	148	140	1252	1576	353		
RTOR Reduction (vph)	0	113	0	0	0	0		
Lane Group Flow (vph)	327	35	140	1252	1576	353		
Turn Type		Perm	Prot			Free		
Protected Phases	4		5	2	6	BASAGGG		
Permitted Phases	E STATE OF STATE	4	Sac escape Co.	or against the	SCALAR SO	Free		
Actuated Green, G (s)	28.6	28.6	15.0	82.8	62.8	121.4		
Effective Green, q (s)	28.6	28.6	15.0	82.8	62.8	121.4		
Actuated g/C Ratio	0.24	0.24	0.12	0.68	0.52	1.00		
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	-55000000000000000000000000000000000000		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0			
Lane Grp Cap (vph)	417	373	230	2541	1927	1667		SA HELENONE DON
v/s Ratio Prot	c0.18		c0.08	0.34	c0.42			
v/s Ratio Perm	00,10	0.02	00.00	0.01	00.12	0.21		
v/c Ratio	0.78	0.09	0.61	0.49	0.82	0.21		
Uniform Delay, d1	43.5	36.3	50.4	9.2	24.5	0.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	9.3	0.1	4.5	0.2	2.8	0.3		
Delay (s)	52.8	36.4	54.9	9.4	27.3	0.3		
Level of Service	D	D	D	A	C	Α.		
Approach Delay (s)	47.7	10000000		14.0	22.4	430000000000		
Approach LOS	D			В	C		WARRANT THE VICTOR STATES AND A SECOND	
Intersection Summary						0100 GGC 100 F600		
HCM Average Control Dela	ay .		22.5	H	CM Leve	of Service	C	
HCM Volume to Capacity r			0.78					
Actuated Cycle Length (s)			121.4	S	um of los	t time (s)	15.0	
			72.9%			of Service	C	
Analysis Period (min)			15				Washington Control of the Control of	
c Critical Lane Group								

	•	*	4	†	ţ	4	
Movement	EBL	EBR	NBL	NBT	SBJ	SBR	
Lane Configurations	ሻ	7	*	^	44	7	
Volume (vph)	270	277	233	1422	1262	374	
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.89	0.89	0.95	0.95	0.96	0.96	The second secon
Adj. Flow (vph)	303	311	245	1497	1315	390	
RTOR Reduction (vph)	0	240	0	0	0	0	AND PARTY TO THE CORRESPONDED TO SECURE DESCRIPTION AND PARTY OF THE P
Lane Group Flow (vph)	303	71	245	1497	1315	390	
Turn Type		Perm	Prot		1010	Free	AND DESCRIPTION OF THE PROPERTY OF THE PROPERT
Protected Phases	4		5	2	6		
Permitted Phases		4				Free	
Actuated Green, G (s)	26.3	26.3	21.4	78.3	51.9	114.6	
Effective Green, g (s)	26.3	26.3	21.4	78.3	51.9	114.6	
Actuated g/C Ratio	0.23	0.23	0.19	0.68	0.45	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0		A A CONTRACTOR OF A CONTRACTOR OF THE PROPERTY OF STREET OF STREET OF STREET OF STREET
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	406	363	348	2545	1687	1667	
v/s Ratio Prot	c0.17		c0.13	0.40	c0.35		
v/s Ratio Perm		0.05				0.23	
v/c Ratio	0.75	0.20	0.70	0.59	0.78	0.23	
Uniform Delay, d1	41.0	35.6	43.6	9.6	26.5	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	7.3	0.3	6.3	0.4	2.4	0.3	
Delay (s)	48.4	35.9	50.0	10.0	28.9	0.3	
Level of Service	D	D	D	A	C	Α	
Approach Delay (s)	42.0			15.6	22.3		
Approach LOS	D			В	С		THE CONTRACT OF THE STATE OF TH
Intersection Summary			Section 21				
HCM Average Control Delay	1		22.4	Н	CM Leve	l of Service	C
HCM Volume to Capacity rat	tio		0.75				
Actuated Cycle Length (s)			114.6	S	um of los	t time (s)	15.0
Intersection Capacity Utilizat	tion		72.9%			of Service	C
Analysis Period (min)			15				TO STATE OF THE ST
c Critical Lane Group							

14		

	1	*	1	1	1	ţ		
Movement	WEL	WER	MBT	NBR	SBL	SBT		
Lane Configurations	7	*	1>		7	†		_
Volume (veh/h)	43	60	437	47	21	337		
Sign Control	Stop		Free		i year and the same	Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.83	0.83	0.93	0.93	0.98	0.98		
Hourly flow rate (vph)	52	72	470	51	21	344		
Pedestrians				100				
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	882	495			520			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	882	495			520			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF(s)	3.5	3.3			2.2			
p0 queue free %	83	87			98			
cM capacity (veh/h)	310	574			1046			
Direction (Lane)#[WBIL	WB 2	NB1	ASB.	SB12			527
Volume Total	52	72	520	21	344			
Volume Left	52	0	0	21	0			
Volume Right	0	72	51	0	0			
cSH	310	574	1700	1046	1700	to anti-construction of the con-		
Volume to Capacity	0.17	0.13	0.31	0.02	0.20			
Queue Length 95th (ft)	15	11	0	2	0			
Control Delay (s)	18.9	12.2	0.0	8.5	0.0			
Lane LOS	С	В	committee in	Α				
Approach Delay (s)	15.0		0.0	0.5				
Approach LOS	В							
Intersection Summary								
Average Delay			2.0					
Intersection Capacity Utiliza	ation		36.2%	IC	CU Level	of Service	A	
Analysis Period (min)			15					

	1	*	†	1	-	1	
Movement	WBL	WER	WINBTO	NBR	SBL	SBT	
Lane Configurations	ሻ	7	4		ሻ	†	
Volume (veh/h)	56	27	580	53	23	465	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.78	0.78	0.96	0.96	0.84	0.84	
Hourly flow rate (vph)	72	35	604	55	27	554	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	1240	632			659		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	1240	632			659		
tC, single (s)	*5.4	*5.2			4.1		
tC, 2 stage (s)							
tF(s)	*2.5	*2.3			2.2		
p0 queue free %	78	95			97		
cM capacity (veh/h)	324	764			929		
Direction, Hane #	- WB(1)	WB2	WB4.	SBI	SB)2		
Volume Total	72	35	659	27	554		
Volume Left	72	0	- 0	27	0		
Volume Right	0	35	55	0	0		
cSH	324	764	1700	929	1700		
Volume to Capacity	0.22	0.05	0.39	0.03	0.33		
Queue Length 95th (ft)	21	4	0	2	0		
Control Delay (s)	19.2	9.9	0,0	9.0	0.0		
Lane LOS	C	Α		A			
Approach Delay (s)	16.2		0.0	0.4			
Approach LOS	C						
ntersection Summary	New Est		4/4/25				
Average Delay			1.5				
Intersection Capacity Utiliza	ition		43.7%	IC	U Level	Service	A
Analysis Period (min)			15				

User Entered Value

HCM Unsignalized Intersection Capacity Analysis 13: Kulanihakoi &

APPENDIX E

TRAFFIC SIGNAL WARRANT STUDY FOR THE INTERSECTION OF PIILANI HIGHWAY AND KULANIHAKOI STREET

Traffic Signal Warrant Study

Piilani Highway and Kulanihakoi Street



Prepared for: Group 70 International, Inc.

Prepared by: Wilson Okamoto Corporation

May 2010

TRAFFIC SIGNAL WARRANT STUDY FOR THE INTERSECTION OF PIILANI HIGHWAY AND KULANIHAKOI STREET

Prepared for: Group 70 International, Inc. 925 Bethel Street, 5th Floor Honolulu, HI 96813

Prepared by:

Wilson Okamoto Corporation 1907 S. Beretania Street, Suite 400 Honolulu, Hawaii 96826 WOC Ref. #7854-02

May 2011

Traffic Signal Warrant Study for the Intersection of Piilani Highway and Kulanihakoi Street

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I.	Intro	duction	Page 1
II.	Exis	ting Traffic Conditions	1
	A.	Area Roadway System	1
	В.	Traffic Volumes and Conditions	3
III.	Traff	ic Signal Warrants	3
	A.	General	3
	B.	Warrant 1	
	C.	Warrant 2	4
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IV.	Conc	clusion	5

Traffic Signal Warrant Study for the Intersection of Piilani Highway and Kulanihakoi Street

I. INTRODUCTION

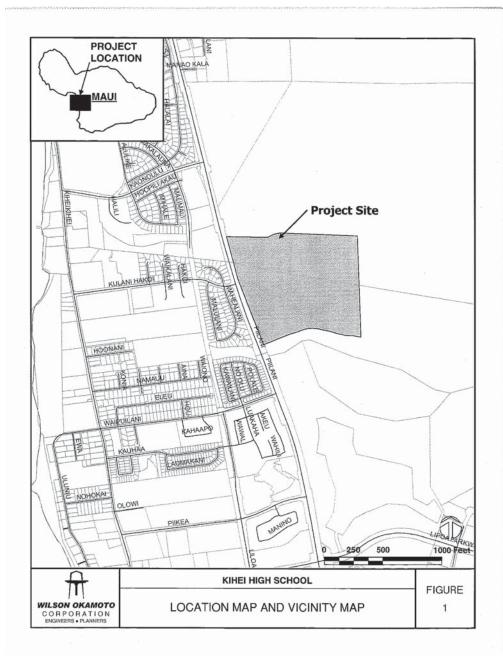
The purpose of this study is to determine if a traffic signal system is warranted at the intersection of Piilani Highway and Kulanihakoi Street in Kihei on the island of Maui (see Figure 1). The proposed Kihei High School will be located adjacent to the highway in the vicinity with access provided via a new roadway at this intersection. The traffic volumes at this intersection may warrant the installation of a traffic signal system and the provision of a traffic signal system at this intersection could provide additional safety for turning vehicles and pedestrians at the intersection.

II. EXISTING TRAFFIC CONDITIONS

A. Area Roadway System

In the vicinity of the intersection with Kulanihakoi Street, Piilani Highway is a predominantly four-lane, two-way roadway generally oriented in the north-south direction that provides access through Kihei. At the intersection with Kulanihakoi Street, the northbound approach of the highway has an exclusive left-turn lane and two through lanes while the southbound approach has two through lanes and an exclusive right-turn lane. Kulanihakoi Street is generally oriented in the east-west direction and serves as a connector roadway between South Kihei Road and Piilani Highway. At the intersection with Piilani Highway, the Kulanihakoi Street approach has two stop-controlled lanes that serve left-turn and right-turn traffic movements.

The access roadway for the proposed Kihei High School will connect to the east side of the intersection creating a four-way intersection. After the connection is completed, the westbound approach of the access road is expected to have two westbound lanes that serve left-turn, through, and right-turn traffic movements. In addition, northbound deceleration and acceleration lanes are expected to be constructed along Piilani Highway to facilitate entering and exiting traffic at the school's access.



Traffic Signal Warrant Study for the Intersection of Piilani Highway and Kulanihakoi Street

B. Traffic Volumes and Conditions

Field investigations were conducted on January 24-27, 2011 at the intersection of Piilani Highway and Kulanihakoi Street. The investigations consisted of manual turning movement count surveys conducted during the morning peak hours of 6:00 AM to 9:00 AM and the afternoon peak hours of 3:00 PM and 6:00 PM. In addition, 24-hour mechanical count surveys were conducted along Piilani Highway and Kulanihakoi Street for all approaches of the intersection. Appendix A includes the existing traffic count data.

III. TRAFFIC SIGNAL WARRANTS

A. General

The installation of a traffic signal at an intersection may be justified by one or more of the nine warrants outlined in the "Manual on Uniform Traffic Control Devices for Streets and Highways," 2009 Edition (MUTCD). These warrants take into account factors such as eight-hour vehicular volumes (Warrant 1), four-hour vehicular volumes (Warrant 2), peak hour volumes (Warrant 3), pedestrian volumes (Warrant 4), the presence of a school crossing or coordinated signal system (Warrants 5 and 6), crash experience (Warrant 7), other characteristics of the roadway network (Warrant 8), and the presence of railroad crossings (Warrant 9). The applicable Warrants 1, 2, and 3 are assessed in this study to determine if a traffic signal system is warranted at the intersection of Piilani Highway and Kulanihakoi Street.

B. Warrant 1

Warrant 1, the "Eight-Hour Volume Warrant," consists of two conditions that may justify the installation of a traffic signal at an intersection where vehicles experience high traffic delay due to large volumes of intersecting traffic during any eight hours of an average day. The first condition is the "Minimum Vehicular Volume Condition" and the second is the "Interruption of Continuous Traffic Condition." Warrant 1 can be satisfied either by meeting the thresholds shown in the 100% columns of either condition of Table 4C-1 of the MUTCD or by meeting the thresholds shown in the 80% columns for both conditions of Table 4C-1 of the

Traffic Signal Warrant Study for the Intersection of Piilani Highway and Kulanihakoi Street

MUTCD. Under existing conditions, the traffic volumes entering the intersection of Piilani Highway and Kulanihakoi Street meet the thresholds during any eight hours of the day and, as such, satisfy Warrant 1 for minor street approaches with two lanes for high traffic volumes on the major street (see Appendix B). It should also be noted that after the proposed Kihei High School is constructed traffic volumes at this intersection are expected to increase thereby more than adequately satisfying the conditions for Warrant 1.

C. Warrant 2

Warrant 2, the "Four-Hour Volume Warrant," consists of several conditions that may justify the installation of a traffic signal at an intersection where vehicles experience high traffic delay due to large volumes of intersecting traffic during any four hours of an average day. One of the conditions is based upon the relationship between the traffic volumes along the major and minor street. If the traffic volumes along the minor street exceed the thresholds shown in Figure 4C-1 of the MUTCD, a traffic signal system may be warranted. Under existing conditions, the traffic volumes entering the intersection of Piilani Highway and Kulanihakoi Street meet the thresholds during any four hours of the day and, as such, satisfy Warrant 2 for minor street approaches with two lanes for high traffic volumes on the major street (see Appendix C). It should also be noted that after the proposed Kihei High School is constructed traffic volumes at this intersection are expected to increase thereby more than adequately satisfying the conditions for Warrant 2.

D. Warrant 3

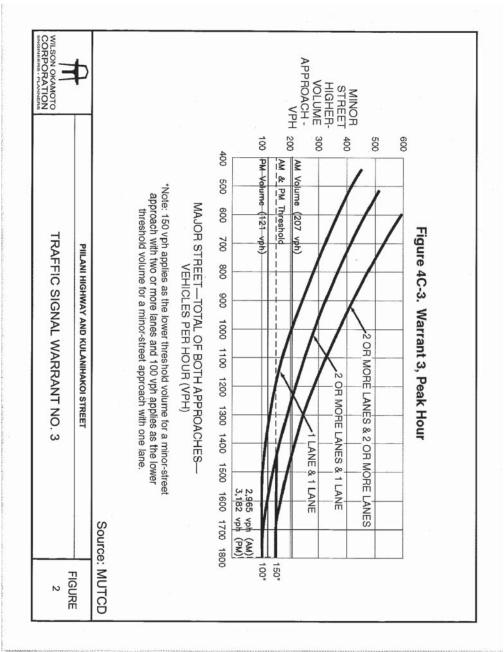
Warrant 3, the "Peak Hour Warrant," consists of several conditions that may justify the installation of a traffic signal at an intersection where vehicles experience high traffic delay due to large volumes of intersecting traffic during the peak hour periods. One of the conditions is based upon the relationship between the traffic volumes along the major and minor streets. If the traffic volumes along the minor street exceed the thresholds shown in Figure 4C-3 of the MUTCD, a traffic signal system may be warranted. Under existing conditions, the traffic volumes entering the

Traffic Signal Warrant Study for the Intersection of Piilani Highway and Kulanihakoi Street

intersection of Piilani Highway and Kulanihakoi Street meet the thresholds during the AM peak hour of traffic and, as such, satisfy Warrant 3 for minor street approaches with two lanes for high traffic volumes on the major street (see Figure 2). It should also be noted that after the proposed Kihei High School is constructed traffic volumes at this intersection are expected to increase thereby more than adequately satisfying the conditions for Warrant 3.

IV. CONCLUSION

The proposed Kihei High School will be located adjacent to Pillani Highway with access provided at the intersection with Kulanihakoi Street. As such, existing traffic conditions at the intersection of Pillani Highway and Kulanihakoi Street were assessed to determine if a traffic signal system is warranted at that intersection as outlined in the "Manual on Uniform Traffic Control Devices for Streets and Highways," 2009 Edition (MUTCD). The existing traffic volumes at the that intersection are currently high enough to satisfy the Eight-Hour Volume Warrant (Warrant 1), Four-Hour Volume Warrant (Warrant 2), and the Peak Hour Warrant (Warrant 3). In addition, the construction of the proposed Kihei High School, as well as, other projects in the vicinity is expected to increase traffic volumes at this intersection thereby more than adequately satisfying the warrant conditions. As such, a traffic signal system is recommended at the intersection of Pillani Highway and Kulanihakoi Street.



APPENDIX A EXISTING TRAFFIC COUNT DATA

Wilson Okamoto Corporation 1907 S. Beretania St., Suite 400 Honolulu, HI 96826

Page 1

Pillani Hwy. South of Kulanihakoi Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

S	tart	24-Jan-11		NB		lour T				SB		Hour	Totals		bined Totals
	ime	Mon	Morning	Afternoon	Momi	ng	Afternoon	1 1	forning	Afternoo	n Mo	rning	Afternoor	n Mornin	g Afternoor
	12:00					-				-					
	12:15						P575 1 1 1						2 5 F		
	12:30														
	12:45			-0 - 1	. ()	0					0	0	0	
	01:00												- 1		
	01:15						- 1			24 22.0			21 0.05		
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	03:00			263						54					
	03:15			262						61			- 2 2 1		
	03:30			256			- 1			53					
	03:45			292			1073			58		0	226	0	129
				275			1073			58		U	220		123
	04:00						e .						4 4 6 6		
	04:15			296			1			57					
	04:30			291					· * ·	56			300 - 35 SWIN		
	04:45		Will a Star	252)	1114			56		0	227	0	134
	05:00			242					•	53					
	05:15			243			100			54			218 44 44		
	05:30			270						49					
	05:45		1001	248	e - 0 0	V	1003			37		0	193	0	119
	06:00			223			1003			42			100	9	2211-112-119
			1000				20 10 10						ALC: UK		
	06:15			212			C 5			29			The same		
	06:30			220						31					
	06:45			195)	850			32		0	134	0	98
	07:00			187			T			21					
	07:15			196						32			10 B		
	07:30			169			- 1			18					
	07:45			168	0		720		18	9		0	80	0	80
	08:00			171	-		120			20					
				156			10 2 4 2			16			T - 5000 . 1		
	08:15						100		1				417		
	08:30			154	THE RES					7					
	08:45		STATES.	157	C		638		100	19		0	62	0	70
	09:00			176						12					
	09:15		1-1-1-1	173			e stra			31			14 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
	09:30			157						11			2010/2010		
	09:45		5017 * 1	147	0)	653			6		0	60	0	71
	10:00			131	COLUMN TO STATE OF	8 5				9				10.34	200
	10:15			126			24			17			107 Feb 1		
	10:30			102						6			9.0		
						. 10									
	10:45		ant light	96		,	455			2		0	34	0	48
	11:00			84						4					
	11:15		Late of the	84			1		100	3			X 5. 101		
	11:30			68						3			office of		
	11:45		1 - 5 *	56	0	1	292					0	- 11	0	30
	Total		0	6798				-	0	1027				0	782
	Percent		0.0%	100.0%					0.0%	100.0%				0.0%	100.09

Page 2

Pillani Hwy. South of Kulanihakol Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

Start	25-Jan-11	1	VB.	Hour	Totals	5	SB	Hour	Totals	Combin	ed Totals
Time	Tue	Moming	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00	100	52	209	monning	711101110011	3	55				
12:15		52	221		100000000000000000000000000000000000000	1 1	53		SA DE DE		
12:30		24	225		20 00	2	44				
12:45		23	218	151	873	0	63	- 6	215	157	1088
01:00		20	236	101	0,0	0	47		2.0		
01:00		17	210		ET ROTE WITH	o o	61		START OF		
01:13		20	229			2	40				
		16	230	73	905	ő	46	2	194	75	1099
01:45		25	239	10	905	0	49		104	15	100
02:00					1 1 41 40	1	61		1 - 12 - 21		
02:15		9	263		3.17.2				B (20 1) 1 20		
02:30		17	247			0	57		004		100
02:45		16	227	67	976	0	57		224	68	120
03:00		14	238			0	51		132		
03:15		20	234		31 5 E. E.		67				
03:30		29	255			0	66			1 2 200	50 1000
03:45		26	274	89	1001	1	58	2	242	91	124
04:00		51	246			3	66		transport of the		
04:15		39	263			2	67		144 F		
04:30		54	226			3	74				
04:45		52	224	196	959	2	60	10	267	206	122
05:00		74	240			8	59				
05:15		74	211		***** × (2)	5	57		地名美国克里斯		
05:30		108	204			8	65				
05:45		113	206	369	861	4.0	65	25	246	394	110
06:00		129	208		a. a. 110-e.e.v. per 1	13	39	100000000000000000000000000000000000000	110000000000000000000000000000000000000		
06:15		193	199		Sec. 25.	20	59		SE POLICE		
06:30		215	207			23	45		2000		
06:45		246	223	783	837	39	43	95	186	878	102
07:00		253	176	700	007	39	30	00		0.0	
07:00		245	179		a system and	54	28		5568 (10)		
		238	178		E 1307 E	59	27		5,154, 40		
07:30		268	172	1004	705	41	21	102	106	1197	81
07:45		242	176	1004	705	44	23	193	100	1107	01
08:00					100 2000 200	45	18		a work and less		
08:15		252	184		100000000000000000000000000000000000000	45			The second		
08:30		245	148			The state of the state of	16		2000 C 1000		70
08:45		202	146	941	654	34	16	170	73	1111	72
09:00		206	158		* IV *++5	47	19				
09:15		214	150		ON A STATE OF	35	19		25 45 00		
09:30		212	159			51	27				
09:45		211	151	843	618	59	20	192	85	1035	70
10:00		213	162			58	17				
10:15		223	157		255 V F4 13	53	18		C. C. S.		
10:30		228	120			42	8		202		
10:45		227	102	891	541	46	6	199	49	1090	59
11:00		223	98			50	9				
11:15		239	99		Sec. 13.	46	13		BASE VISIT		
11:30		215	66			51	5		H		
11:45		223	65	900	328	59	4	206	31	1106	35
Total		6307	9258	300	520	1101	1918	200		7408	11176
Percent		40.5%	59.5%			36.5%	63.5%			39.9%	60.19

Wilson Okamoto Corporation 1907 S. Beretania St., Suite 400 Honolulu, HI 96826

Piilani Hwy. South of Kulanihakoi Street Site Code: Station ID;

Latitude: 0' 0.000 Undefined

Start Time 12:00 12:15 12:30	Wed	Morning	Afternoon		Totals						
12:00 12:15			Atternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
		46	203			1	66				
12:30		60	220		E 11731 12	1.00	94		3 Tag 2 1 1 1 1 1		
		31	240			1	65				
12:45		35	225	172	888	1	82	4	307	176	1195
01:00		34	264			0	69				
01:15		29	226		1104-1107	3	78		50 5 1 10		
01:30		17	249			0	65				
01:45		15	238	95	977	0	65	3	277	98	1254
02:00		24	227			0	69		-		120
02:15		13	241		India 3	0	64		adiscensis for		
02:30		22	244			0	74		1000		
02:45		15	248	74	960	1	71		278	75	1238
03:00		19	238	14-	500		60		210	13	1200
03:15		22	236		100 300		82		State 1		
03:30		22	218		a reserving	0	80		Control of the		
03:30		27	208	90	900	0	97	1100	319	94	1010
		42		90	900		88	4	218	94	1219
04:00			218		a street to	2		0.000	A CONTRACTOR OF		
04:15		39	223		X 24311 NO	4	97				
04:30		62	212			7	108		4 222		
04:45		60	216	203	869	0	79	13	372	216	1241
05:00		68	219		and the sales	5	80				
05:15		86	205			5	97				
05:30		106	195			22	98		a series con el cara		
05:45		105	185	365	804	20	96	52	371	417	1175
06:00		138	159			11	77				
06:15		157	190		200 T 12 T 195	27	92		TO BE STORY		
06:30		195	181			36	56				
06:45		223	175	713	705	44	74	118	299	831	1004
07:00		190	176		100-00-00-00-00-00-00-00-00-00-00-00-00-	74	58		1.17838.		
07:15		189	158		25 6 26	56	56		2000		
07:30		212	157		2.5	47	38		4.5		
07:45		221	167	812	658	75	47	252	199	1064	857
08:00		219	149			62	42		- 171100		
08:15		236	143		337 FEE 63	63	32		felie attack.		
08:30		218	137			57	53				
08:45		245	134	918	563	52	42	234	169	1152	732
09:00		204	150	0.0	000	66	35				
09:15		181	151		GENERAL PROPERTY.	75	68		and the last		
09:30		191	147		10.00	61	18		4.50		
09:45		222	137	798	585	55	24	257	145	1055	730
10:00		211	129	130	303	63	24	201	140	1000	700
		219	124		A 12-57-10	65	30		and the Ro		
10:15 10:30		219							100		
			111	000	454	60	18	000	00	1000	537
10:45		211	90	868	454		11	230	83	1098	537
11:00		218	74		J	57	5				
11:15		217	110		51	63	9				
11:30		222	65	1		66	8				
11:45	CE STATE OF SEA	226	69	883	318	62	8	248	30	1131	348
Total Percent		5991 40.8%	8681 59.2%			1416 33.2%	2849 66.8%			7407 39.1%	11530 60.9%

Page 3

Pillani Hwy. South of Kulanihakoi Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

Page 4

	tart	27-Jan-11		1B		Totals		SB		Totals		ed Totals
Ti	me	Thu	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
7007	12:00		48				5		- I PARTICIPATE OF THE	-		
	12:15		36			11 To 12	0	100		- 11		
	12:30		19			1 1	0			100		
	12:45		21		124	0	2	17.00	7	0	131	
	01:00		16				1					
	01:15		20				2					
	01:30		14				4			14		
			23		70	0					04	
	01:45				73	0	1-1	The second second	- 8	0	81	in fillbet
	02:00		25				2					
	02:15		24				6			ALCOHOLD TO		
	02:30		18				3			to the seal of the		
	02:45	\$5.10 SERVICE AND ADDRESS.	11		78	0	3	10 (3)	14	0	92	200
	03:00		18				7					
	03:15		17			- 1 - 12	9	11 20 1 to		_P(0) - N/A		
	03:30		20				9					
	03:45		25		- 80	0	8		33	0	113	te- Time
	04:00		36				13					
	04:15		35			16	14		9.5	. +		
	04:30		64			- 111	17			1 1000		
	04:45		50		185	0			59	0	244	
					100	0			59	U	244	
	05:00		63			4.54	13	a - 12 L		1 to 1 to 1		
	05:15		78			100		Fig. 1. 120		Selection 19 H		
	05:30		108			CO 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	27					
	05:45		124		373	0	15	Au 15-11 2	82	0	455	
	06:00		136				15					
	06:15		165	337 .		3 40 16	23	25 42 5		3277171 125		
	06:30		174				60					
	06:45		171		646	0	66		164	0	810	
	07:00		207		75.55	1000	70		1000			
	07:15		196			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	80	15 4 5 10		1957 a 194		
	07:30		168				83			921 6		
	07:45		205		776	0	70	2 30 10	303	0	1079	
			191		//0		99		303	0	1079	
	08:00					91.5						
	08:15		212			1.0	56	22 6 6 2				
	08:30		196				52					
	08:45		238	S (4	837	0	48	White State	255	0	1092	
	09:00		209				84					
	09:15		211	20		Carper in	55	-5 / 7 * * ·		20 THE R. P. LEW.		
	09:30		221			Section Control	77			100		
	09:45		220	4 25 10 11	861	0	62	r didi.	278	0	1139	an Carlotte
	10:00		196				56					
	10:15		151-75 10	Buch St.	1 - V	\$385 · 15		5 5 5 8 8 Pm		10 mm at 12	* * ad	
	10:30											
	10:45		dan di				F. 32.0	10.00			115.50	
	11:00											
	11:15			A 100 100 100 100 100 100 100 100 100 10		44° 50°	100	A 100 PM	137 T		- 1	
	11:30							•				
	11:45		111.	* * * * * * * * * * * * * * * * * * * *			95705.7	2 0 0 0	1817		X 3 454	300
	Total		4229	0		1000	1259	0		10000000	5236	9.00
F	ercent		100.0%	0.0%			100.0%	0.0%			100.0%	0.09
	rand Total	al .	165		27			776 57	0.4		200	

AADT 18,760

ADT 18,760

Wilson Okamoto Corporation

1907 S. Beretania St., Suite 400 Honolulu, HI 96826

Piilani Hwy. North of Kulanihakoi Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

Mon	Morning	Afternoon	Morning 0	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
		:	0	0	1750 Le	eme (g	a ar egyv	Jan. 100		
		:	0	0				1.		
	ng am tip Lipin (1995) Lipin (1995)		. 0	0						
	ergan er Grynd erg Grynd erg		. 0	0						
						CONTRACTOR AND	0	0	0	0
				100		*				
	Lagir Lagir			1.0		hear a pri		5 St. 12		
		5								
			0	0	COUNTY.	55 828 91	0	0	0	0
								NAME OF		
		309		Sec. 1	2.	120				
		310	0	619		152	0	272	0	891
		337				123		22 5 5		
	unierity:	285				159				
		310	4.00			140			F 2 F	
		320	0	1252	HAT INTE	128	0	550	0	1802
		367		200		107		497		
		296		September 1	100	150		THE REPORT		
		390 375		4400	eris . 7	132		504		1000
			0	1428	#2_70_ja	98	0	501	0	1929
	a recorder	364 353		-0.00		127		action of the		
				And the				STATE OF THE		
	100 Feb. 19	310 292		1010	S	101		452	0	1771
	A Property of the	289	0	1319		85	0	402	0	THE LIVE
		288		SERVICE BY	TO MAKE	86		a second la		
	re was angers	260		Coeman Ex		85		\$1.50 mm 2 100		
	444 - 444	221	0	1058		79	0	335	0	1393
	SERVICE SERVICE	212		1000		80	U	333	U	1000
		212		F. 47000	2 00	74		Concess to		
		175		3/10		63		VC 45		
	C 1 + 1	173	0	772		57	0	274	0	1046
	Harry III James	141	0	116		57	0	214		1040
	147 5 * S.N	146		arter Garage		70		6 (297) Gir		
	20 50 50 5	146		100		53				
	5010000000	164	0	597	A	55	0	235	0	832
	British British	151				76		200	1-110	CONTRACTOR
	material service	122		HERE IN SHEE	ar Gastan	74		J-23 1 3		
		141				58		an early by		
	5-11 Y - 14 A . A	92	0	506		69	0	277	0	783
		104	10000			58		-511		
				to be the	5.14 ·			F : 100		
		87				41				
	40		0	345	12 m	35	0	196	0	541
		56	11.	S 5000		37	1.0			
		48		A 224 Ex		39		5 1 3 B		
		41						1 7 15		
	14 15 16 16	40	0	185	15 to \$25	25	0	131	0	316
	0		-		0	3223			0	11304
	336 Vene 1536		* 86 * 87 68 * 56 * 48 * 41 * 40 0 8081	* 86 * 87 * 68 * 56 * 48 * 41 * 40 0 8061	* 86 * 87 * 68 * 56 * 48 * 41 * 40 0 185	* 86 * 87 * 68 * 56 * 48 * 41 * 40 * 0 8081	* 86	* 86	* 86	* 86

Page 1

Page 2

Pillani Hwy. North of Kulanihakoi Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

	Start	25-Jan-11		SB	Hour	Totals		NB	Hour	Totals	Combin	ed Totals
	Time	Tue	Morning	Afternoon	Morning	Afternoon	Moming	Afternoon	Morning	Afternoon	Morning	Afternoor
	12:00		37	199	-		17	230				
	12:15		25	244		April 1	26	155		5 F 711 F 20		
	12:30		27	276			7	163				
	12:45		18	263	107	982	. 5	175	55	723	162	170
	01:00		10	267			8	196		1 3 5 5 5 5		
	01:15			311		5 H . 1 . 1 C	5	183		2004		
	01:30		12	293			7	202				
	01:45		10	306	41	1177	7	189	27	770	68	194
	02:00		10	300	104		14	168				
	02:15		11	316		STATE OF LAND	4	184		de amount from		
	02:10		10	299		Ast Assessed Des	6	188				
	02:45		14	318	45	1233	5	163	29	703	74	193
			10	305	40	1233		197	20	703	14-1	100
	03:00		17	362		10000	5 8	164				
	03:15					0 4-10 100						
	03:30		29	351			6	202				010
	03:45		24	323	80	1341	14	201	33	764	113	210
	04:00		51	337			9	158	100	40.00		
	04:15		43	341		HOSPIN IN	15	209		WHAT BE TO		
	04:30		63	284		1000000	18	169				
	04:45		49		206	1268	16	173	58	709	264	197
	05:00		61	317			31	180				
	05:15		79	313		1977	28	181				
	05:30		96	286			46	164				
	05:45		99	310	335	1226	51	131	156	656	491	188
	06:00		114	276			62	142				
	06:15		154	264		2.45	102	142		CALL RESIDEN		
	06:30		216	238			124	185				
	06:45		217	210	701	988	153	155	441	624	1142	161
	07:00		288	216			150	130			0.000	
	07:15		352	208		190 48	187	115		SBWMHU		
	07:30		366	175			158	117		Latin His		
	07:45		324	207	1330	806	146	95	641	457	1971	126
	08:00		259	168	1330	000	152	103	041	407	1011	120
	08:00		281	160		e monte de	133	112		Security of		
				155		ALC: Let 1	166	89		Water China		
	08:30		232		4000	000		85	570	200	1000	102
	08:45		261	150	1033	633	119		570	389	1603	102
	09:00		258	161			115	77				
	09:15		248	166		1000	162	111		CALLES AND AND AND ADDRESS.		
	09:30		223	177			147	97				
	09:45		229	172	958	676	185	93	609	378	1567	105
	10:00		181	148			184	88				
	10:15		217	116		er const	188	98		1922/2014		
	10:30		233	92			178	82		JAN 2003		
	10:45		238	63	869	419	166	64	716	332	1585	75
	11:00		208	68			204	72		A. (1995)		
	11:15		234	54		- 75 -	183	66		14 at 1 at 25		
	11:30		268	48			190	40				
	11:45		273	39	983	209	161	40	738	218	1721	42
-	Total		6688	10958	500	200	4073	6723	.00	2101	10761	1768
	Percent		37.9%	62.1%			37.7%	62.3%			37.8%	62.2

Wilson Okamoto Corporation 1907 S. Beretania St., Suite 400 Honolulu, HI 96826

Page 3

Piilani Hwy. North of Kulanihakoi Street Site Code: Station ID:

Latitude: 0' 0.000 Undefined

Start	26-Jan-11	- 3	SB	Hour	Totals	1	NB .	Hour	Totals	Combin	ed Totals
Time	Wed	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		29	315			27	181			3	
12:15		21	339		BENEFIT SE	37	237				
12:30		17	325		100	22	221				
12:45		9	295	76	1274	31	251	117	890	193	2164
01:00		14	313		12.7	23	289		000	100	2.01
01:15		19	329		JC 945	19	218		e esta les		
01:30		13	317			9	248		3 1		
		15	312	61	1271	12	228	63	983	124	2254
01:45				01	12/1			63	903	124	2254
02:00		5	282		Francisco Brita	21	224		Acres de la		
02:15		8	347		51 4 4 4 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8	219		A STATE OF THE PARTY OF THE PAR		
02:30		10	322			- 11	229				
02:45		18	361	41	1312	4	249	44	921	85	2233
03:00		15	342			4	196				
03:15		17	343			9	217				
03:30		17	341			11	223				
03:45		28	345	- 77	1371	14	190	38	826	115	2197
04:00		43	368			19	186				
04:15		44	311		2365 T 1	12	253		Pitter		
04:30		71	325			29	176				
04:45		42	347	200	1351	30	195	90	810	290	2161
05:00		54	320			37	198				
05:15		84	350		S - 146	39	192		A123.02 (6)		
05:30		100	299		100	56	183		WINDSHIP LA		
05:45		105	303	343	1272	65	141	197	714	540	1986
06:00		108	290	343	12/2	91	141	197	F 194	340	1000
06:15		174	286		Arter State Link	100	163		S SHOW ROS		
					Marie Company				Superior St.		
06:30		189	232			169	141				
06:45		219	271	690	1079	167	128	527	573	1217	1652
07:00		253	218			171	140		1112000		
07:15		320	195		4.50	153	126		0.00		
07:30		347	197			146	117				
07:45		273	198	1193	808	191	. 121	661	504	1854	1312
08:00		346	167			152	105				
08:15		295	164			159	110		8 44.1		
08:30		259	153			167	117				
08:45		272	170	1172	654	175	102	653	434	1825	1088
09:00		238	162			181	99				
09:15		233	173		224	177	128		1,000		
09:30		221	148		FU 10 10 10 10 10 10 10 10 10 10 10 10 10	192	96		127/22 (100)		
09:45		228	141	920	624	202	97	752	420	1672	1044
10:00		235	112	020	024	198	85	102		1012	19.17
10:15		251	102		ENVISOR SERVICES	200	90		35 ST 15 ST		
		224	108			198	68				
10:30		277		007	200			784	304	4774	696
10:45			70	987	392	188	61	784	304	1771	696
11:00		255	38			221	56				
11:15		251	37		200	240	78		100		
11:30		301	41			215	45				
11:45		335	41	1142	157	106	48	782	227	1924	384
Total	Cath multiple out	6902	11565	HER STREET, STREET, SCI.		4708	7606			11610	19171
Percent		37.4%	62.6%			38.2%	61.8%			37.7%	62.3%

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Piilani Hwy. North of Kulanihakoi Street Site Code; Station ID;

Latitude: 0' 0.000 Undefined

Start	27-Jan-11		B		Totals		VB.		Totals		ed Totals
Time	Thu	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoor
12:00		23				31			0)3/4/4/0.00	200-000	
12:15		14	Self (**)		100	25			100 TO 100		
12:30		16			376	11					
12:45		15		68	0	12	Deserte of	79	0	147	
01:00		5				11					
01:15		9			1-12-5	12	e wich early		197 11 ba		
01:30		11				9					
01:45		12		37	0	18	an uders a lib	50	0	87	
02:00		15				17					
02:15		14			San San San		11 - 11		ALMER - D.		
02:15		13			a same	14	W 4 2 12 14		werd and		
				F0.				40			
02:45		11		53	0			46	0	99	
03:00		18			2 2	8			5 G 10 E		
03:15		24			the state of	6			0.000		
03:30		28	- 1	- 122		. 7			100		
03:45		28		98	0	14		35	0	133	
04:00		42				15					
04:15		46			200	13	- 94		1 / ZP -		
04:30		71			eer - 220 1	25					
04:45		54	A	213	0	28		81	0	294	
05:00		53				34					
05:15		91	S		17 - 5	39	212 mile (1)		established in		
05:30		89				63					
05:45		105		338	0	72		208	0	546	
06:00		107		and the second second		79			0.4050000000000000000000000000000000000		
06:15		151	A		PARCEN IN	119	52 BELL # 1		-mag/85 85		
06:30		209			405	137			HERE		
06:45		213		680	0	172		507	0	1187	
		256		000		179		307		1107	
07:00					one o				seesan lee		
07:15		288			en 5	188			4.		
07:30		346		and the second second	N 7002 0	173					
07:45		298		1188	0	196	100	736	0	1924	
08:00		298				169			Section 1 and 1		
08:15		293	45.00		Section 1	125			3-1-5		
08:30		268				133					
08:45		261	5. T. S. S.	1120	0	165	经保持制度	592	0	1712	
09:00		250				180					
09:15		254	9.37		SAME OF ST	166			3000		
09:30		232			2010	216			1000		
09:45		129	4.00	865	0	79	HE 128	641	0	1506	
10:00											
10:15		1	7.70	25000	1.00 m	5 Ta A * 65		10.00	read(selection)	145 (C. *) 15	
10:30											
10:45		65 president							5. f	55-1 B+5	
11:00							1				
11:15				100		Transfer and the second			2000	President	
11:30							2000				
11:45	Carrie El	10000000			10000	1.55 (6.55)	100	CO-CONTRACTOR	37.6 mg	7005	2500 [-0
Total		4660	0			2975	0			7635	
Percent		100.0%	0.0%			100.0%	0.0%			100.0%	0.0
Grand Total	al	182	50 30	604		117	756 175	552		300 38.4	

AADT 29,612

ADT 29,612

																																														Description 1: Description 2: Description 3:	
Factor	Volume	Peak Hour	Count				11:45 AM	11:30 AM	11:15 AM	11:00 AM	10:45 AM	10:30 AM	10-15 AM	9:45 AM	9:30 AM	9:15 AM	9:00 AM	8:30 AM	8:15 AM	8:00 AM	7:45 AM	7:30 AM	7:00 AM	6:45 AM	6:30 AM	6:15 AM	5:45 AM	5:30 AM	5:00 AM	4:45 AM	4:30 AM	4:00 AM	3:45 AM	3:30 AM	3:00 AM	2:45 AM	2:30 AM	2:00 AM	1:45 AM	1:30 AM	1:15 AM	12:45 AM	12:30 AM	12:15 AM	Begin		
	,		c	EB		24 Hour Volume			*		,				,	,								ı																				. ,	EB	Kihel High School Kulanihakol Street	
			c	МВ	12:00 A	ne 830 (45.4%)	3																															,		í		1,			WB		
		*8	c	Combined		1000 (54.6%)													e																		, T								Combined		Honolu
						1830		11	11	11	10	10	10	10	9 9	9	9	20 0			7:	7:	7.	6.	6:	6. 6	5	57.5	un u	4:	4	4.4		ωι	u w	2:	2:	2 2				12:	12:	12:		Daily Volume	Honolulu, HI 96826
							:45 PM	30 PM	15 PM	:00 PM	45 PM	30 PM	15 PM	DO PM	30 PM	15 PM	00 PM	45 PM	15 PM	00 PM	45 PM	30 PM	I S DM	45 PM	30 PM	6:15 PM	45 PM	30 PM	15 PM	45 PM	30 PM	15 PM	3:45 PM	30 PM	00 PM	45 PM	30 PM	OU PM	45 PM	30 PM	1:15 PM	45 PM	12:30 PM	15 PM	Begin		
000	174	3:30 PM	45000				C.	o UI	2	U	4	7	16	19		19	24	140		14	20	18	20	46	50	26	27	42	46	51	37	47	45	46	: .	,				•					EB	3	
					12:0					15			i	46			71			55			04	04		C+T			ChT			1/1			102												
000	191	5:30 PM	74 70 00	al MB	12:00 PM - 12:00 AM		0	0	6	15	15	14	13	10	28	16	17	20	100	26	36	22	40	28	47	46	50	50	45	33	49	44	34	42	13 .								4		WB		
					O AM					36				52			82			94			CCT	3		700	200		104			100	355		88											Date:	
0.92	336	4:00 PM		Daulollion	Cartina		,	14	co	20	19	21	29	29	40	35	41	34	37	40	56	40	69	74	97	72 8	77	92	91	84	86	91	79	88	23.			, ,		,			,		Comomed	Parking and the same of the sa	
										51				98			153			149			***	519		202	300		120	534		200	356		190						90					1/24/2011 Monday	

escription 1: Kihei escription 2: Kula escription 3:	i High School nihakoi Street											Date:		1/26/201 Wednesda
Begin	EB		WB		Combine		y Volume	Begin	EB		WB	++	Combined	
Begin 12:00 AM	3	25	6	26	9	51		12:00 PM	68	370	49	142	117	512
12:15 AM	4		11	20	15	31		12:15 PM	98	370	35	146	133	342
12:30 AM	3		6		9			12:30 PM	111		35		146	
12:45 AM	15		3		18			12:45 PM	93		23		116	
1:00 AM	2			21	10	25		1:00 PM	106	289	24	155	130	444
	0			21		25				289	39	133	110	444
1:15 AM 1:30 AM	. 0		4		4 3			1:15 PM 1:30 PM	71 54		41		95	
			3										109	
1:45 AM	2		9		11		and the second	1:45 PM	58	000	51 25	142	85	430
2:00 AM	4			5	6	17		2:00 PM	60	288	25	142		430
2:15 AM	3		1		4			2:15 PM	71		47		118	
2:30 AM	5		2		. 2			2:30 PM	63		42		105	
2:45 AM	0		0		0		100	2:45 PM	94	-	28	2.00	122	
3:00 AM	0		0	8	0	21		3:00 PM	75	204	36	160	111	364
3:15 AM	2		4		6			3:15 PM	35		45		80	
3:30 AM	1		1		2			3:30 PM	37		38		75	
3:45 AM	10		3		13			3:45 PM	57		41		98	
4:00 AM	. 2	23	0	10	2	33		4:00 PM	41	173	41	190	82	363
4:15 AM	3		1		4			4:15 PM	32		54		86	
4:30 AM	9		4		13			4:30 PM	38		41		79	
4:45 AM	9		5		14			4:45 PM	62		54		116	
5:00 AM	- 6		5	28	11	83		5:00 PM	32	168	41	203	73	371
5:15 AM	6		4		10			5:15 PM	44		49		93	
5:30 AM	21		8		29			5:30 PM	54		53		107	
5:45 AM	22		11		33			5:45 PM	38		60		98	
6:00 AM	30	134	10	57	40	191		6:00 PM	39	172	51	186	90	358
6:15 AM	36		10		46			6:15 PM	62		59		121	
6:30 AM	31		9		40			6:30 PM	34		41		75	
6:45 AM	37		28		65			6:45 PM	37		35		72	
7:00 AM	56		20	115	76	406		7:00 PM	27	101	33	125	60	226
7:15 AM	73		29		102	100		7:15 PM	25	777	26	1000	51	5577
7:30 AM	92		29		121			7:30 PM	27		36		63	
7:45 AM	70		37		107			7:45 PM	22		30		52	
8:00 AM	50			174	87	430	_	8:00 PM	19	95	23	119	42	214
8:15 AM	56		45	174	101	430		8:15 PM	23	33	32	2.10	55	
8:30 AM	74		38		112			8:30 PM	24		32		56	
8:45 AM	76		54		130			8:45 PM	29		32		61	
9:00 AM				100				9:00 PM	12	73	33	117	45	190
	97			133	115	419				13	25	11/	44	190
9:15 AM	47		44		91			9:15 PM	19		42		65	
9:30 AM	83		30		113			9:30 PM	23 19		17		36	
9:45 AM	59		41	454	100	555	_	9:45 PM	17	47	21	73	38	120
10:00 AM	67	222		131	100	353		10:00 PM		4/		/3	29	120
10:15 AM	56		30		86			10:15 PM	14		15			
10:30 AM	38		29		67			10:30 PM	10		18		28	
10:45 AM	61		39		100			10:45 PM	6	27	19	35	25 17	62
11:00 AM	44			170	88	350		11:00 PM	9	27	8	35		02
11:15 AM	38		46		84			11:15 PM	3		9		12	
11:30 AM	49		37		86			11:30 PM	9		13		22	
11:45 AM	49		43		92		11-12-12-12-12	11:45 PM	6	J. 17	5		11	
	24 Hour	Volume	EB 3508 12:00 AM - 12	(58.1%) 2:00 PM	<u>WB</u> 252	5 (41.9%)	Combined 6033			1	2:00 PM - 12	:00 AM		
	EB		WB		Combine	h			EB	-	WB		Combiner	1
Count			878		2379				2007		1647		3654	
Count	63.1		36.9 %		23/9				54.9 %		45.1 %			
Peak Hour	8:15		8:00 AI		8:15 A7	a.			12:15 PM		5:30 PM		12:15 PM	1
Volume			174	4	458	-1			408		223		525	
Volume									0.92		0.93		0.90	
	0.7	25	0.81		0.88				0.92					

Wilson Okamoto Corporation 1907 S. Beretania St., Suite 400 Honolulu, HI 96826

ition 2: Kular ition 3:	ihakoi Street											Date:		1/25 Tu
		34-11-20					Daily Volume	1000						
Begin	EB		WB		Combined			Begin	EB		WB	100	Combined	
12:00 AM	3		5	23	8	38		12:00 PM	32	142	33	120	65	262
12:15 AM	4		8		12			12:15 PM	33		27		60	
12:30 AM	6		7		13			12:30 PM	45		34		79	
12:45 AM	2		3		5			12:45 PM	32		26		58	
1:00 AM	6	9	4	13	10	22		1:00 PM	23	147	20	107	43	254
1:15 AM	1		3		4			1:15 PM	27		30		57	
1:30 AM	1		1		2			1:30 PM	37		22		59	
1:45 AM	1		5		6			1:45 PM	60		35		95	
2:00 AM	2		4	11	6	23		2:00 PM	48	193	32	141	80	334
2:15 AM	3		6	**	9	20		2:15 PM	21	400	34		55	
2:30 AM	6		0		. 6			2:30 PM	65		33		98	
													101	
2:45 AM	1	-	1		2			2:45 PM	59		42	1.00		328
3:00 AM	2		2	7	4	29		3:00 PM	40	159	42	169	82	328
3:15 AM	9		3		12			3:15 PM	34		52		86	
3:30 AM	1		1		2			3:30 PM	47		34		81	
3:45 AM	10		1		11			3:45 PM	38		41		79	
4:00 AM	5	27	4	13	9	40		4:00 PM	34	145	35	146	69	291
4:15 AM	4		4		8			4:15 PM	30		42		72	
4:30 AM	11		2		13			4:30 PM	44		33		77	
4:45 AM	7		3		10			4:45 PM	37		36		73	
5:00 AM	10		4	11	14	82		5:00 PM	37	156	41	152	78	308
5:15 AM	23		1		24			5:15 PM	36		27		63	
5:30 AM	15		2		17			5:30 PM	48		43		91	
5:45 AM	23		4		27			5:45 PM	35		41		76	
6:00 AM	29		3	52	32	186		6:00 PM	39	137	48	171	87	308
6:15 AM	23		9	34	32	100		6:15 PM	32		48		80	
6:30 AM	41		23		64			6:30 PM	40		44		84	
6:45 AM	41		17		58			6:45 PM	26		31		57	
7:00 AM	52		24	87	76	353		7:00 PM	29	87	37	115	66	202
7:15 AM	73		23	07	96	333		7:15 PM	22		32		54	
7:30 AM	76		14		90			7:30 PM	19		21		40	
7:45 AM	65		26		91			7:45 PM	17		25		42	
				440		244			23	74	30	102	53	176
8:00 AM	54		29	119	83	311		8:00 PM		14	24	102	33	1/0
8:15 AM	52		37		89			8:15 PM	9					
8:30 AM	56		28		84			8:30 PM	19		30		49	
8:45 AM	30		25		55			8:45 PM	23		18		41	
9:00 AM	39		22	80	61	267		9:00 PM	19	65	20	87	39	152
9:15 AM	40		15		55			9:15 PM	16		26		42	
9:30 AM	46		16		62			9:30 PM	18		24		42	
9:45 AM	62		27		89			9:45 PM	12		17		29	
10:00 AM	60	186	21	115	81	301		10:00 PM	11	50	22	72	33	122
10:15 AM	46		37		83			10:15 PM	22		24		46	
10:30 AM	44		24		68			10:30 PM	7		10		17	
10:45 AM	36		33		69			10:45 PM	10		16		26	
11:00 AM	47		30	122	77	306		11:00 PM	12	20	8	28	20	48
11:15 AM	43		20		63			11:15 PM	1		9		10	
11:30 AM	59		45		104			11:30 PM	5		9		14	
11:45 AM	35		27		62			11:45 PM	2		2		4	
11.42 MH			EB		WB		Combined	221-13 711		-				
	24 Hou	r Volume		(56.5%)		(43.5%								
	24 Hou	Volume	12:00 AM - 1		2003	(43.370	, 4,45			12	2:00 PM - 12	:00 AM		
	EB		WB	ALGO PPI	Combined	d			EB	-	WB		Combine	d
Count	130		653		1958	d.			1375		1410		2785	
Count			33.4 9	e:	1958				49.4 %		50.6 %		2703	
Peak Hour	66.6 7:15		33.4 9 10:45 A		7:15 AM				2:30 PM		5:45 PM		2:30 PM	
				un					198		181		367	
Volume	26		128		360									
Factor	0.8	8	0.71		0.94				0.76		0.94		0.91	

2			**										Description 1: Description 2: Description 3:	
Count Peak Hour Volume Factor	9:30 AM 9:45 AM 10:00 AM	9:00 AM 9:15 AM	8:00 AM 8:15 AM 8:30 AM	7:15 AM 7:30 AM 7:45 AM	6:15 AM 6:30 AM 6:45 AM	5:15 AM 5:30 AM	4:00 AM 4:15 AM 4:30 AM 4:45 AM	3:00 AM 3:15 AM 3:30 AM 3:45 AM	2:00 AM 2:15 AM 2:30 AM 2:45 AM	1:15 AM 1:30 AM 1:45 AM	12:15 AM 12:30 AM 12:45 AM	Begin	Kihel High School Kulanihakoi Street	
24 Hour Volume EB 1003 52.3 % 7:00 AM 309 0.73	35 27	59 52	ននន	106 77	36360	22 8	់ ហ ហ យ យ	4004	աաատ	4004	0124	EB	ol Street	
lume 12:	27	182	220	504	200	28 2	20	vı	19	v	. 18			
EB (52.3%) Volume 1003 (52.3%) 12:00 AM - 12:00 PM WB 068 5 37.7% 5 37.7% 183 0.83	33 38	33 27 40	55 4 55 50 5 55	23 33	13	101154	шойд	-202	ω 4 0 σ	2 w 0 4	បលម្	WB		
00 PM	38	127	183	ţ		2 2	و	UI	22	v	17			
WB 608 (37.7%) Combined 1611 7:15 AM 449 0.87						33 33 37 37 37		υ n o ω	13 6			Combined		1907 S. H
.7%)	65	309	403	15		103	29	10	41	14	35	ш	i.	. Bereta lonolulu
Combined 1611												Daily volume		1907 S. Beretania St., Suite 400 Honolulu, HI 96826
												Begin		e 400
oE												EB		
12:00 PM - 12:00 AM WB												WB	Date:	
Combined												Combined		
													1/27/2011 Thursday	

APPENDIX B

WARRANT 1 EIGHT HOUR VOLUME WARRANT

	Piilan	i Hwy	Kulanih	akoi St	10	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
8:00	488	1922	54	192	0	1	1	1
8:15	519	1885	52	177				
8:30	465	1816	56	165				
8:45	450	1775	30	155				
9:00	451	1754	39	187	0	1	1	1
9:15	450	1688	40	208				
9:30	424	1667	46	214				
9:45	429	1692	62	212				The state of the s
10:00	385	1716	60	186	0	1	1	1
10:15	429	1752	46	173				
10:30	449	1784	44	170	7	CC YOUR THORNESS COM		
10:45	453	1805	36	185				
11:00	421	1837	47	184	0	1	1	1
11:15	461	1814	43	169	2 10112-0-21			
11:30	470	1806	59	159		*	11000	
11:45	485	1823	35	145	A 1897/AV			55.0
12:00	398	1806	32	142	0	1	0	1
12:15	453	1898	33	133				
12:30	487	1950	45	127	A CONTRACTOR OF THE PARTY OF TH	i cana		ET I III I
12:45	468	1970	32	119	ii .			
13:00	490	2023	23	147	0	1	0	- 1
13:15	505	2057	27	172	The second			TANK TANK
13:30	507	2115	37	166	n zajekinin			
13:45	521	2139	60	194			Same 1	
14:00	524	2147	48	193	0	1	1	1
14:15	563	2151	21	185	\$-100 P	S DOWNSTON	7410	
14:30	531	2166	65	198				
14:45	529	2223	59	180				- 7776
15:00	528	2275	40	159	0	1	0	- 1
15:15	578	2313	34	153	gradient W		1	S
15:30	588	2322	47	149	barane i	,		31.75m/3259m
15:45	581	2230	38	146				
16:00	566	2164	34	145	0	1	0	1
16:15	587	2139	30	148		Zarana wa		11.3.10
16:30	496	2060	44	154	-			

Page 2 of 3

Warrant 1 8-Hour Volumes (:00)

TERMS:	Piilan	i Hwy	Kulanih	akoi St	10	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition B
0:00	87	253	3	15	0	0	0	0
0:15	76	196	4	18				
0:30	50	146	6	15				
0:45	40	127	-2	10				
1:00	30	113	6	9	0	0	0	0
1:15	26	118	1	5			-	
1:30	31	111	1	7				
1:45	26	107	1	12				
2:00	35	110	2	12	0	0	0	0
2:15	19	99	3	12	1.15			3
2:30	27	116	6	18				-
2:45	29	146	1	13				
3:00	24	166	2	22	0	0	0	0
3:15	36	241	9	25		\$200 TO		
3:30	57	285	1	20		5000000		
3:45	49	342	10	30				
4:00	99	392	5	27	0	0	0	0
4:15	80	425	4	32		4		
4:30	114	494	11	51		0.72 0.211 0.311 0.21		
4:45	99	579	7	55			Constant	į.
5:00	132	687	10	71	0	0	0	0
5:15	149	792	23	90				
5:30	199	982	15	90				
5:45	207	1203	23	116		10000		Ę.
6:00	237	1448	29	134	0	1	0	1
6:15	339	1738	23	157	- 7			Res:
6:30	420	1978	41	207	Control 1			
6:45	452	2144	41	242			t same	
7:00	527	2268	52	266	1	1	1	1
7:15	579	2229	73	268				l Various constitu
7:30	586	2169	76	247	7			Secretary Security of
7:45	576	2048	65	227				

Warrant 1 8-Hour Volumes (:15)

	Piilan	i Hwy	Kulanih	akoi St	100	0%	80	1%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
0:00	87	253	3	15			i i	
0:15	76	196	4	18	0	0	0	0
0:30	50	146	6	15				
0:45	40	127	2	10				
1:00	30	113	6	9				
1:15	26	118	1	5	0	0	0	0
1:30	31	111	1	7				
1:45	26	107	1	12				
2:00	35	110	2	12				
2:15	19	99	3	12	0	0	0	0
2:30	27	116	6	18				-32
2:45	29	146	1	13				Utore Anna Savana
3:00	24	166	2	22			- 1	
3:15	36	241	9	25	0	0	0	0
3:30	57	285	1	20	Same and the second			
3:45	49	342	10	30				
4:00	99	392	5	27				
4:15	80	425	4	32	0	0	0	0
4:30	114	494	11	. 51				
4:45	99	579	7	55	(a			
5:00	132	687	10	71				
5:15	149	792	23	90	0	0	0	1
5:30	199	982	15	90	Grando-M			
5:45	207	1203	23	116	English Committee		- Andalan Liver review	2000
6:00	237	1448	29	134				
6:15	339	1738	23	157	0	1	0	1
6:30	420	1978	41	207	- 11			
6:45	452	2144	41	242				ur-sun STYAnse PA
7:00	527	2268	52	266	en en			
7:15	579	2229	73	268	. 1	1	1	1
7:30	586	2169	76	247				
7:45	576	2048	65	227				

Page 1 of 3

	Piilan	i Hwy	Kulanih	akoi St	10	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
16:45	515	2040	37	158				
17:00	541	2026	37	156	0	1	0	1 '
17:15	508	1955	36	158				
17:30	476	1897	48	154				
17:45	501	1854	35	146				
18:00	470	1776	39	137	0	- 1	0	1
18:15	450	1687	32	127				Ĩ
18:30	433	1614	40	117				
18:45	423	1525	26	96				
19:00	381	1471	29	87	0	0	0	1
19:15	377	1426	22	81				i
19:30	344	1385	19	68				
19:45	369	1336	17	68				
20:00	336	1256	23	74	0	0	0	0
20:15	336	1231	9	70				
20:30	295	1203	19	77			8	
20:45	289	1235	23	76	ĵ3			
21:00	311	1260	19	65	0	0	0	0
21:15	308	1252	16	57	S			00
21:30	327	1211	18	63			*	
21:45	314	1091	12	52	See 1			
22:00	303	939	11	50	0	0	0	0
22:15	267	799	22	51			7.7.7.7.111.11.7.7.2	
22:30	207	682	7	30			- 2	
22:45	162	587	10	28				No. of the last of
23:00	163	527	12	20	0	0	0	0
23:15	150	364	1	8				U.
23:30	112	214	5	7				reference describe
23:45	102	102	2	2	-			

Janes I	Piilan	i Hwy	Kulanih	akoi St	10	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
16:45	515	2040	37	158				
17:00	541	2026	37	156				
17:15	508	1955	36	158	0	1	0	1
17:30	476	1897	48	154				
17:45	501	1854	35	146			W	
18:00	470	1776	39	137				
18:15	450	1687	32	127	0	1	0	1
18:30	433	1614	40	117				-1.17.71.171.1-
18:45	423	1525	26	96				7.7.2
19:00	381	1471	29	87		· ·		
19:15	377	1426	22	81	0	0	0	1
19:30	344	1385	19	68				
19:45	369	1336	17	68				
20:00	336	1256	23	74				
20:15	336	1231	9	70	0	0	0	0
20:30	295	1203	19 .	77				
20:45	289	1235	23	76	\$ = \$25 ALCONNAC	- Carlinania	consul	19772.00k
21:00	311	1260	19	65		1		
21:15	308	1252	16	57	0	0	0	0
21:30	327	1211	18	63				72 = 70.7
21:45	314	1091	12	52				
22:00	303	939	11	50		Events:		
22:15	267	799	22	51	0	0	0	0
22:30	207	682	7	30				
22:45	162	587	10	28	Company of the			
23:00	163	527	12	20	Santa and Santa			- 1
23:15	150	364	- 1	8		San Sange		
23:30	112	214	5	7				
23:45	102	102	2	2		10000		
	189		of Periods Wa	rrant Satisfied	2	13	7	15

Page 3 of 3

	Piilan	i Hwy	Kulanih	akoi St	10	0%	80	0%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
8:00	488	1922	54	192				
8:15	519	1885	52	177	0	1	1	1
8:30	465	1816	56	165				
8:45	450	1775	30	155				
9:00	451	1754	39	187				
9:15	450	1688	40	208	1	1	1	1
9:30	424	1667	46	214				
9:45	429	1692	62	212				
10:00	385	1716	60	186				
10:15	429	1752	46	173	0	1	1	1
10:30	449	1784	44	170				
10:45	453	1805	36	185				
11:00	421	1837	47	184	-			
11:15	461	1814	43	169	0	1	1	1
11:30	470	1806	59	159	a a			
11:45	485	1823	35	145				
12:00	398	1806	32	142		9200		
12:15	453	1898	33	133	0	1.	0	1
12:30	487	1950	45	127				700007
12:45	468	1970	32	119				
13:00	490	2023	23	147			The state of the s	111
13:15	505	2057	27	172	0	1	1	1
13:30	507	2115	37	166				
13:45	521	2139	60	194				+6-00-000
14:00	524	2147	48	193	19.77	0 2871417.7		F32450.22-
14:15	563	2151	21	185	0	1 -	1	1
14:30	531	2166	65	198	-48			- MANUAL CONTRACTOR
14:45	529	2223	59	180		*		2000
15:00	528	2275	40	159				
15:15	578	2313	34	153	0	1	0	1
15:30	588	2322	47	149				
15:45	581	2230	38	146				
16:00	566	2164	34	145	g - 000011-1-			
16:15	587	2139	30	148	0	1	0	1
16:30	496	2060	44	154		J. 10. 10. 10. 10. 1		

	Piilan			akoi St		0%		1%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
8:00	488	1922	54	192				
8:15	519	1885	52	177				1
8:30	465	1816	56	165	0	1	1	1
8:45	450	1775	30	155				
9:00	451	1754	39	187				
9:15	450	1688	40	208				
9:30	424	1667	46	214	1	1	1	1
9:45	429	1692	62	212				
10:00	385	1716	60	186				
10:15	429	1752	46	173	-			
10:30	449	1784	44	170	0	1	1	1
10:45	453	1805	36	185			- 4	
11:00	421	1837	47	184	7/60 3			
11:15	461	1814	43	169				
11:30	470	1806	59	159	0	1	0	1
11:45	485	1823	35 .	145				
12:00	398	1806	32	142	7			-20/00/00
12:15	453	1898	33	133			1 7	
12:30	487	1950	45	127	0	1	0	1
12:45	468	1970	32	119	9			
13:00	490	2023	23	147				
13:15	505	2057	27	172				
13:30	507	2115	37	166	0	- 1	1	1
13:45	521	2139	60	194	Commission of the second			
14:00	524	2147	48	193				
14:15	563	2151	21	185	8		The state of the s	UUS C
14:30	531	2166	65	198	0	1	1	1
14:45	529	2223	59	180				
15:00	528	2275	40	159				
15:15	578	2313	34	153	8			
15:30	588	2322	47	149	0	1	0	1
15:45	581	2230	38	146	Anna agricultura oppur			
16:00	566	2164	34	145				
16:15	587	2139	30	148				
16:30	496	2060	44	154	0	1	0	1

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Warrant 1 8-Hour Volumes (:30)

-11	Piilan	i Hwy	Kulanih	akoi St	10	0%	80	1%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
0:00	87	253	3	15				
0:15	76	196	4	18	100			
0:30	50	146	6	15	0	0	0	0
0:45	40	127	2	10				
1:00	30	113	6	9		2		
1:15	26	118	1	5				10/17
1:30	31	111	1	7	0	0	0	0
1:45	26	107	1	12				33
2:00	35	110	2	12				
2:15	19	99	3	12	F			
2:30	27	116	6	18	0	0	0	0
2:45	29	146	1	13		VIII - 18 - 21	11.45.40	
3:00	24	166	2	22		- 1 1 T T T T T T T T T T T T T T T T T		
3:15	36	241	9	25			- 3	
3:30	57	285	1	20	0	0	0	0
3:45	49	342	10	30			COMPANIE, A	2
4:00	99	392	5	27				
4:15	80	425	4	32			1000	
4:30	114	494	11	51	0	0	0	0
4:45	99	579	7	55	1000	(120 A) (A)	- 2	Grandar VIII
5:00	132	687	10	71	Salt Control		3	S 1,000
5:15	149	792	23	90			2 CARANY 2	3
5:30	199	982	15	90	0	0	0	1
5:45	207	1203	23	116			10000	Ž.
6:00	237	1448	29	134			=100 W2	Same
6:15	339	1738	23	157				y == -w==
6:30	420	1978	41	207	1	1	1	1
6:45	452	2144	41	242	Part and the second		7800mm - V	
7:00	527	2268	52	266				9
7:15	579	2229	73	268			- 9	Kengara - V
7:30	586	2169	76	247	1	1	1	1
7:45	576	2048	65	227				Į.

Warrant 1 8-Hour Volumes (:45)

	Piilan	i Hwy	Kulanih	akoi St	100	0%	80	1%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
0:00	87	253	3	15			1	
0:15	76	196	4	18				2
0:30	50	146	6	15				
0:45	40	127	2	. 10	0	0	0	0
1:00	30	113	6	9				<u> </u>
1:15	26	118	1	5				
1:30	31	111	1	7				
1:45	26	107	1	12	0	0	0	0
2:00	35	110	2	12	100 - 0000.000			But -
2:15	19	99	3	12				Establish Name
2:30	27	116	6	18			sesurents)	West .
2:45	29	146	1	13	0	0	0	0
3:00	24	166	2	22				
3:15	36	241	9	25				
3:30	57	285	1	20	- NAMES OF STREET			0
3:45	49	342	10	30	0	0	0	0
4:00	99	392	5	27			nervice minoral	
4:15	80	425	4	32			9	i i
4:30	114	494	11	51			- 6	8
4:45	99	579	-7	55	0	0	0	0
5:00	132	687	10	71			311107117.	i e
5:15	149	792	23	90				i casson.
5:30	199	982	15	90			Successive Sales	
5:45	207	1203	23	116	0	1	0	1
6:00	237	1448	29	134				l.
6:15	339	1738	23	157				
6:30	420	1978	41	207	and the state of the state of		- 1	
6:45	452	2144	41	242	1	1	1	1
7:00	527	2268	52	266		o essaro umaserol		
7:15	579	2229	73	268				1.
7:30	586	2169	76	247				
7:45	576	2048	65	227	1	1	1	1

Page 1 of 3

	Piilan	i Hwy	Kulanih	nakoi St	10	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
16:45	515	2040	37	158				
17:00	541	2026	37	156				3 2
17:15	508	1955	36	158				
17:30	476	1897	48	154	0	1	0	1
17:45	501	1854	35	146			i i	
18:00	470	1776	39	137				
18:15	450	1687	32	127				
18:30	433	1614	40	117	0	1	0	1
18:45	423	1525	26	96				
19:00	381	1471	29	87				
19:15	377	1426	22	81				
19:30	344	1385	19	68	0	0	0	0
19:45	369	1336	17	68				
20:00	336	1256	23	74			i i	
20:15	336	1231	9	70				
20:30	295	1203	19	77	0	0	0	0
20:45	289	1235	23	76			i i	
21:00	311	1260	19	65			1	
21:15	308	1252	16	57				
21:30	327	1211	18	63	0	0	0	0
21:45	314	1091	12	52				
22:00	303	939	11	50			1	
22:15	267	799	22	51			7	
22:30	207	682	7	30	0	0	0	0
22:45	162	587	10	28				
23:00	163	527	12	20				
23:15	150	364	1	8				
23:30	112	214	5	7				
23:45	102	102	2	2				- 1000

	Piilan	i Hwy	Kulanih	akoi St	100	0%	80)%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
16:45	515	2040	37	158	0	1	0	1
17:00	541	2026	37	156				
17:15	508	1955	36	158				
17:30	476	1897	48	154				
17:45	501	1854	35	146	0	1	0	1
18:00	470	1776	39	137				
18:15	450	1687	32	127				
18:30	433	1614	40	117	- 112		1 10/2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
18:45	423	1525	26	96	0	0	0	1
19:00	381	1471	29	87				
19:15	377	1426	22	81				-
19:30	344	1385	19	68				
19:45	369	1336	17	68	0	0	0	0
20:00	336	1256	23	74				
20:15	336	1231	9	70				
20:30	295	1203	19	77	CHIANCE TO SERVE			
20:45	289	1235	23	76	0	0	0	0
21:00	311	1260	19	65			7.00	
21:15	308	1252	16	57			14 THE STATE	
21:30	327	1211	18	63				
21:45	314	1091	12	52	0	0	0	0
22:00	303	939	11	50	100000		- 3	
22:15	267	799	22	51	1 20			
22:30	207	682	7	30			The Comment Page	(A)
22:45	162	587	10	28	0	0	0	0
23:00	163	527	12	20				110
23:15	150	364	1	8			11	
23:30	112	214	5	7		larding to him	5	
23:45	102	102	2	2				
			of Periods Wa		3	13	6	14

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	Piilan	i Hwy	Kulanih	akoi St	10	0%	80	1%
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Condition A	Condition B	Condition A	Condition E
8:00	488	1922	54	192		V		
8:15	519	1885	52	177				
8:30	465	1816	56	165				
8:45	450	1775	30	155	0	1	0	1
9:00	451	1754	39	187				
9:15	450	1688	40	208				
9:30	424	1667	46	214				
9:45	429	1692	62	212	1	1	1	1
10:00	385	1716	60	186				
10:15	429	1752	46	173				7,010
10:30	449	1784	44	170				H
10:45	453	1805	36	185	0	1	1	1
11:00	421	1837	47	184				
11:15	461	1814	43	169	î .			- 1000
11:30	470	1806	59	159		MINI - 1		
11:45	485	1823	35	145	0	1	0	1
12:00	398	1806	32	142				37.00
12:15	453	1898	33	133	100000			
12:30	487	1950	45	127	2			
12:45	468	1970	32	119	0	1	0	1
13:00	490	2023	23	147	o company			10 East 1 W. W.
13:15	505	2057	27	172				200-1-040-
13:30	507	2115	37	166			232231	
13:45	521	2139	60	194	0	1	1	. 1
14:00	524	2147	48	193				
14:15	563	2151	21	185	9 -9/2	7	1944	22 115.7
14:30	531	2166	65	198	E -2000			FIFE USZ-TA
14:45	529	2223	59	180	0 -	1	1	1
15:00	528	2275	40	159				
15:15	578	2313	34	153				
15:30	588	2322	47	149				
15:45	581	2230	38	146	0	1	0	1
16:00	566	2164	34	145				
16:15	587	2139	30	148	V transport		CHICKS THE	CS how
16:30	496	2060	44	154			4	- Common - 1

APPENDIX C

WARRANT 2 FOUR HOUR VOLUME WARRANT

Warrant 2 4-Hour Volumes (:00)

	Piilan	i Hwy	10	Kulanihakoi St	2-11-1200-00-20	Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
0:00	87	253	3	15	0	
0:15	76	196	4	18	0	
0:30	50	146	6	15	0	
0:45	40	127	2	10	0	
1:00	30	113	6	9	0	
1:15	26	118	1	5	0	
1:30	31	111	1	7	0	
1:45	26	107	1	12	0	
2:00	35	110	2	12	0	
2:15	19	99	3	12	0	
2:30	27	116	6	18	0	
2:45	29	146	1	13	0	
3:00	24	166	2	22	0	
3:15	36	241	9	25	0	
3:30	57	285	1	20	0	100
3:45	49	342	10	30	0	
4:00	99	392	5	27	0	
4:15	80	425	4	32	0	
4:30	114	494	11	51	0	
4:45	99	579	7	55	0	
5:00	132	687	10	71	0	
5:15	149	792	23	90	0	
5:30	199	982	15	90	0	
5:45	207	1203	23	116	1	
6:00	237	1448	29	134	1	1_
6:15	339	1738	23	157	1	
6:30	420	1978	41	207	1	1
6:45	452	2144	41	242	1	
7:00	527	2268	52	266	1	1
7:15	579	2229	73	268	1	
7:30	586	2169	76	247	1	180
7:45	576	2048	65	227	1	
8:00	488	1922	54	192	1	1
8:15	519	1885	52	177	1	
8:30	465	1816	56	165	1	
8:45	450	1775	30	155	1	
9:00	451	1754	39	187	1	1
9:15	450	1688	40	208	1	
9:30	424	1667	46	214	1	
9:45	429	1692	62	212	1	
10:00	385	1716	60	186	1	1
10:15	429	1752	46	173	1	
10:30	449	1784	44	170	1	
10:45	453	1805	36	185	1	
11:00	421	1837	47	184	1	1
11:15	461	1814	43	169	1	621
11:30	470	1806	59	159	1	

		i Hwy		Kulanihakoi St	3	Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
11:45	485	1823	35	145	_ 1	966
12:00	398	1806	32	142	1	1
12:15	453	1898	33	133	1	
12:30	487	1950	45	127	1	
12:45	468	1970	32	119	1	1.00
13:00	490	2023	23	147	1	1
13:15	505	2057	27	172	1	
13:30	507	2115	37	166	1	
13:45	521	2139	60	194	1	405204
14:00	524	2147	48	193	1	1
14:15	563	2151	21	185	1	
14:30	531	2166	65	198	1	
14:45	529	2223	59	180	1	144-67
15:00	528	2275	40	159	1	1
15:15	578	2313	34	153	1	
15:30	588	2322	47	149	1	
15:45	581	2230	38	146	1	
16:00	566	2164	34	145	1	1
16:15	587	2139	30	148	1	
16:30	496	2060	44	154	1	
16:45	515	2040	37	158	1	
17:00	541	2026	37	156	1	1
17:15	508	1955	36	158	1	
17:30	476	1897	48	154	1	
17:45	501	1854	35	146	1	
18:00	470	1776	39	137	1	1
18:15	450	1687	32	127	1	
18:30	433	1614	40	117	1	
18:45	423	1525	26	96	0	
19:00	381	1471	29	87	0	
19:15	377 -	1426	22	81	0	
19:30	344	1385	19	68	0	
19:45	369	1336	17	68	0	
20:00	336	1256	23	74	0	-
20:15	336	1231	9	70	0	
20:30	295	1203	19	77	0	
20:45	289	1235	23	76	0	
21:00	311	1260	19	65	0	
21:15	308	1252	16	57	0	100000000000000000000000000000000000000
21:30	327	1211	18	63	0	
21:45	314	1091	12	52	0	
22:00	303	939	11	50	0	12 15
22:15	267	799	22	51	0	- SA
22:30	207	682	7	30	0	
22:45	162	587	10	28	0	
23:00	163	527	12	20	0	
23:15	150	364	1	8	0	
23:30	112	214	5	7	0	
23:45	102	102	2	2	0	100

Warrant 2 4-Hour Volumes (:15)

	Piilan	i Hwy		Kulanihakoi St		Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
0:00	87	253	3	15	0	
0:15	76	196	4	18	0	
0:30	50	146	6	15	0	
0:45	40	127	2	10	0	
1:00	30	113	6	9	0	
1:15	26	118	1	5	0	
1:30	31	111	1	7	0	
1:45	26	107	1	12	0	
2:00	35	110	2	12	0	
2:15	19	99	3	12	0	
2:30	27	116	6	18	0	
2:45	29	146	1	13	0	
3:00	24	166	2	22	0	
3:15	36	241	9	25	0	
3:30	57	285	1	20	0	
3:45	49	342	10	30	0	
4:00	99	392	5	27	0	
4:15	80	425	4	32	0	
4:30	114	494	11	51	0	
4:45	99	579	7	55	0	
5:00	132	687	10	71	0	
5:15	149	792	23	90	0	
5:30	199	982	15	90	0	110.2
5:45	207	1203	23	116	1	
6:00	237	1448	29	134	1	
6:15	339	1738	23	157	1	1
6:30	420	1978	41	207	1	
6:45	452	2144	41	242	1	
7:00	527	2268	52	266	1	
7:15	579	2229	73	268	1	1
7:30	586	2169	76	247	1	
7:45	576	2048	65	227	1	
8:00	488	1922	54	192	1.	
8:15	519	1885	52	177	1	1
8:30	465	1816	56	165	1	Eulis
8:45	450	1775	30	155	1	- 10
9:00	451	1754	39	187	1	ām
9:15	450	1688	40	208	1	1
9:30	424	1667	46	214	1	ogs og
9:45	429	1692	62	212	1	
10:00	385	1716	60	186	1	
10:15	429	1752	46	173	1	1
10:30	449	1784	44	170	1	- I
10:45	453	1805	36	185	1	
11:00	421	1837	47	184	1	
11:15	461	1814	43	169	1	1
11:30	470	1806	59	159	1	

22000		i Hwy		Kulanihakoi St		Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
11:45	485	1823	35	145	1	
12:00	398	1806	32	142	1	
12:15	453	1898	33	133	1	1
12:30	487	1950	45	127	1	
12:45	468	1970	32	119	1	
13:00	490	2023	23	147	1	
13:15	505	2057	27	172	1	1
13:30	507	2115	37	166	1	
13:45	521	2139	60	194	1	
14:00	524	2147	48	193	1	
14:15	563	2151	21	185	1	1
14:30	531	2166	65	198	_ 1	
14:45	529	2223	59	180	1	
15:00	528	2275	40	159	1	
15:15	578	2313	34	153	1	1
15:30	588	2322	47	149	1	
15:45	581	2230	38	146	1	Thomas and the same of the sam
16:00	566	2164	34	145	1	and the second
16:15	587	2139	30	148	1	1
16:30	496	2060	44	154	1	
16:45	515	2040	37	158	1	
17:00	541	2026	37	156	1	
17:15	508	1955	36	158	1	1
17:30	476	1897	48	154	1	
17:45	501	1854	35	146	1	
18:00	470	1776	39	137	1	
18:15	450	1687	32	127	1	1
18:30	433	1614	40	117	1	
18:45	423	1525	26	96	0	
19:00	381	1471	29	87	0	
19:15	377 -	1426	22	81	0	
19:30	344	1385	19	68	0	
19:45	369	1336	17	68	0	
20:00	336	1256	23	74	0	-
20:15	336	1231	9	70	0	
20:30	295	1203	19	77	0	
20:45	289	1235	23	76	0	
21:00	311	1260	19	65	0	-
21:15	308	1252	16	57	0	
21:30	327	1211	18	63	0	
21:45	314	1091	12	52	0	-
22:00	303	939	11	50	0	
22:15	267	799	22	51	0	
22:30	207	682	7	30	0	
	162	587	10	28	0	
22:45		527	12			
23:00	163			20	0	775
23:15 23:30	150	364	1	8	0	
	112	214	5	7	0	

Warrant 2 4-Hour Volumes (:30)

	Piilan	i Hwy		Kulanihakoi St	2.7	Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
0:00	87	253	3	15	0	
0:15	76	196	4	18	0	
0:30	50	146	6	15	0	
0:45	40	127	2	10	0	
1:00	30	113	6	9	0	
1:15	26	118	1	5	0	
1:30	31	111	1	7	0	
1:45	26	107	1	12	0	
2:00	35	110	2	12	0	
2:15	19	99	3	12	0	
2:30	27	116	6	18	0	- Contract to
2:45	29	146	1	13	0	
3:00	24	166	2	22	0	
3:15	36	241	9	25	0	
3:30	57	285	1	20	0	
3:45	49	342	10	30	0	
4:00	99	392	5	27	0	
4:15	80	425	4	32	0	
4:30	114	494	11	51	0	
4:45	99	579	7	55	0	
5:00	132	687	10	71	0	
5:15	149	792	23	90	0	
5:30	199	982	15	90	0	
5:45	207	1203	23	116	1	
6:00	237	1448	29	134	1	
6:15	339	1738	23	157	1	
6:30	420	1978	41	207	1	1
6:45	452	2144	41	242	1	
7:00	527	2268	52	266	1	
7:15	579	2229	73	268	1	-
7:30	586	2169	76	247	1	1
7:45	576	2048	65	227	1	
8:00	488	1922	54	192	1	
8:15	519	1885	52	177	1	
8:30	465	1816	56	165	1	1
8:45	450	1775	30	155	1	
9:00	451	1754	39	187	1	
9:15	450	1688	40	208	1	11'
9:30	424	1667	46	214	1	1
9:45	429	1692	62	212	1	
10:00	385	1716	60	186	1	10-17-12-1
10:15	429	1752	46	173	1	
10:30	449	1784	44	170	1	1
10:45	453	1805	36	185	1	
11:00	421	1837	47	184	1	
11:15	461	1814	43	169	1	
11:30	470	1806	59	159	1	1

	Piilan			Kulanihakoi St		Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
11:45	485	1823	35	145	1	
12:00	398	1806	32	142	1	
12:15	453	1898	33	133	1	
12:30	487	1950	45	127	1	1
12:45	468	1970	32	119	1	
13:00	490	2023	23	147	_ 1	
13:15	505	2057	27	172	1	
13:30	507	2115	37	166	1	1
13:45	521	2139	60	194	1	
14:00	524	2147	48	193	1	
14:15	563	2151	21	185	1	
14:30	531	2166	65	198	1	1
14:45	529	2223	59	180	1	
15:00	528	2275	40	159	1	
15:15	578	2313	34	153	1	
15:30	588	2322	47	149	1	1
15:45	581	2230	38	146	1	
16:00	566	2164	34	145	1	
16:15	587	2139	30	148	1	
16:30	496	2060	44	154	1	1
16:45	515	2040	37	158	1	
17:00	541	2026	37	156	1	
17:15	508	1955	36	158	1	
17:30	476	1897	48	154	1	1
17:45	501	1854	35	146	1	10000
18:00	470	1776	39	137	1	
18:15	450	1687	32	127	1	
18:30	433	1614	40	117	1	1
18:45	423	1525	26	96	0	
19:00	381	1471	29	87	0	
19:15	377	1426	22	81	0	- 2
19:30	344	1385	19	68	0	
19:45	369	1336	17	68	0	
20:00	336	1256	23	74	0	
20:15	336	1231	9	70	0	-
20:30	295	1203	19	77	0	
20:45	289	1235	23	76	0	
21:00	311	1260	19	65	0	
21:15	308	1252	16	57	0	
21:30	327	1211	18	63	0	S
21:45	314	1091	12	52	0	
22:00	303	939	11	50	0	
22:15	267	799	22	51	0	
22:30	207	682	7	30	0	
22:45	162	587	10	28	0	
23:00	163	527	12	20	0	
23:15	150	364	1	8	0	
23:30	112	214	5	7	0	
23:45	102	102	2	2	0	

Warrant 2 4-Hour Volumes (:45)

	Piilan			Kulanihakoi St		Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
0:00	87	253	3	15	0	
0:15	76	196	4	18	0	
0:30	50	146	6	15	0	
0:45	40	127	2	10	0	
1:00	30	113	6	9	0	25 00
1:15	26	118	1	5	0	
1:30	31	111	1	7	0	1000
1:45	26	107	1	12	0	5 CH 4450 - 1-25
2:00	35	110	2	12	0	
2:15	19	99	3	12	0	_
2:30	27	116	6	18	0	
2:45	29	146	1	13	0	
3:00	24	166	2	22	0	
3:15	36	241	9	25	0	
3:30	57	285	1	20	0	
3:45	49	342	10	30	0	
4:00	99	392	5	27	0	
4:15	80	425	4	32	0	
4:30	114	494	11	51	0	
4:45	99	579	7	55	0	
5:00	132	687	10	71	0	
5:15	149	792	23	90	0	
5:30	199	982	15	90	0	
5:45	207	1203	23	116	1	
6:00	237	1448	29	134	1	
6:15	339	1738	23	157	1	
6:30	420	1978	41	207	1	
6:45	452	2144	41	242	1	1
7:00	527	2268	52	266	1	
7:15	579	2229	73	268	1	
7:30	586	2169	76	247	1	
7:45	576	2048	65	227	1	1
8:00	488	1922	54	192	1	
8:15	519	1885	52	177	1	
8:30	465	1816	56	165	1	
8:45	450	1775	30	155	1	1
9:00	451	1754	39	187	1	
9:15	450	1688	40	208	1	
9:30	424	1667	46	214	1	- 400
9:45	429	1692	62	212	1	1
10:00	385	1716	60	186	1	
10:15	429	1752	46	173	1	The con-
10:30	449	1784	44	170	1	
10:45	453	1805	36	185	1	1
11:00	421	1837	47	184	i	
11:15	461	1814	43	169	i	(A.0-6)0)
11:30	470	1806	59	159	1	1000 72

-		i Hwy		Kulanihakoi St		Warrant
Time	15 Min Count	Hourly Total	15 Min Count	Hourly Total	Above Min	Satisfied
11:45	485	1823	35	145	1	1
12:00	398	1806	32	142	1	
12:15	453	1898	33	133	1	100
12:30	487	1950	45	127	1	
12:45	468	1970	32	119	1	1
13:00	490	2023	23	147	11	
13:15	505	2057	27	172	1	
13:30	507	2115	37	166	1	
13:45	521	2139	60	194	1	1
14:00	524	2147	48	193	1	
14:15	563	2151	21	185	1	
14:30	531	2166	65	198	1	
14:45	529	2223	59	180	1	11
15:00	528	2275	40	159	1	
15:15	578	2313	34	153	1	
15:30	588	2322	47	149	1	
15:45	581	2230	38	146	1	1
16:00	566	2164	34	145	1	
16:15	587	2139	30	148	1	
16:30	496	2060	44	154	1	
16:45	515	2040	37	158	1	1
17:00	541	2026	37	156	1	
17:15	508	1955	36	158	1	
17:30	476	1897	48	154	1	
17:45	501	1854	35	146	1	1
18:00	470	1776	39	137	1	127
18:15	450	1687	32	127	1	17-77-2
18:30	433	1614	40	117	1	
18:45	423	1525	26	96	0	
19:00	381	1471	29	87	0	1877-1
19:15	377	1426	22	81	0	
19:30	344	1385	19	68	0	
19:45	369	1336	17	68	0	1005/177
20:00	336	1256	23	74	0	
20:15	336	1231	9	70	0	
20:30	295	1203	19	77	0	
20:45	289	1235	23	76	0	
21:00	311	1260	19	65	0	
21:15	308	1252	16	57	0	1000
21:30	327	1211	18	63	0	
21:45	314	1091	12	52	0	
22:00	303	939	11	50	0	
22:15	267	799	22	51	0	7.55
22:30	207	682	7	30	0	
22:45	162	587	10	28	0	
23:00	163	527	12	20	0	
23:15	150	364	1	8	0	_
23:30	112	214	5	7	0	
23:45	102	102	-2	2	0	

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APPENDIX F

CAPACITY ANALYSIS CALCULATIONS PROJECTED YEAR 2015 PEAK HOUR TRAFFIC ANALYSIS WITH PROJECT

	٠	*	1	1	†	4					
Movement	EBL	EBR	NBU	NBT	SBT	SBR			VOICE SERVICE	(40) A (5)	
Lane Configurations	ሻ	*	*	^	^	74					
Volume (veh/h)	18	220	66	1229	1490	11					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.84	0.84	0.95	0.95	0.91	0.91					
Hourly flow rate (vph)	21	262	69	1294	1637	12					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				TWLTL	TWLTL						
Median storage veh)				2	2						
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2423	819	1637								
vC1, stage 1 conf vol	1637										
vC2, stage 2 conf vol	786										
vCu, unblocked vol	2423	819	1637								
tC, single (s)	*5.8	*5.9	4.1								
tC, 2 stage (s)	4.8										
tF(s)	*2.5	*2.3	2.2								
p0 queue free %	91	50	82								
cM capacity (veh/h)	238	525	392								
Direction, Lane # 1999	EB1	LB/2	ANB 1	NB 2	NB.3	SB1	SB2	\$83	Mile Miles		PORT
Volume Total	21	262	69	647	647	819	819	12			
Volume Left	21	0	69	0	0	0	0	0			
Volume Right	0	262	0	0	0	0	0	12			
cSH	238	525	392	1700	1700	1700	1700	1700			
Volume to Capacity	0.09	0.50	0.18	0.38	0.38	0.48	0.48	0.01			
Queue Length 95th (ft)	7	69	16	0	0	0	0	0			
Control Delay (s)	21.6	18.5	16.2	0.0	0.0	0.0	0.0	0.0			
Lane LOS	C	C	С								
Approach Delay (s)	18.7		0.8			0.0					
Approach LOS	C										
Intersection Summary		estable is									
Average Delay	ocquemmarcher-ra c		1.9	CONTRACTOR STATE	Language and the second	*********			and the contract of the contra		
Intersection Capacity Utiliza	ation		59.4%	1	CU Level	of Service)		В		
Analysis Period (min)			15								

User Entered Value

HCM Unsignalized Intersection Capacity Analysis 3: Kaonoulu & Piilani

5/4/2011

	1	*	1	1	1	1					
Movement	EBL	E88	NBL	NBT4	SBT	SBB					
Lane Configurations	7	7	٦	^	^	7				Walnest T.	
Volume (veh/h)	20	109	120	1513	1582	105					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.79	0.79	0.87	0.87	0.87	0.87					
Hourly flow rate (vph) Pedestrians	25	138	138	1739	1818	121					
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				TWLTL	TWLTL						
Median storage veh) Upstream signal (ft)				2	2						
pX, platoon unblocked											
vC, conflicting volume	2964	909	1818								
vC1, stage 1 conf vol	1818										
vC2, stage 2 conf vol	1145										
vCu, unblocked vol	2964	909	1818								
tC, single (s)	*5.8	*5.9	4.1								
tC, 2 stage (s)	4.8										
tF (s)	*2.5	*2.3	2.2								
p0 queue free %	85	70	59								
cM capacity (veh/h)	163	465	333								
Direction Paner	E PAEBOL	EB/2	at/Bt/	NB/2	NB.3.	r SBtha	SB2	SB3			
Volume Total	25	138	138	870	870	909	909	121			
Volume Left	25	0	138	0	0	0	0	0			
Volume Right	0	138	0	0	0	0	. 0	121			
cSH	163	465	333	1700		1700	1700	1700			
Volume to Capacity	0.15	0.30	0.41	0.51	0.51	0.53	0.53	0.07			
Queue Length 95th (ft)	13	31	49	0	0	0	0	0			
Control Delay (s)	31.0	16.0	23.2	0.0	0.0	0.0	0.0	0.0			
Lane LOS	D	C	C								
Approach Delay (s)	18.3		1.7			0.0					
Approach LOS	С										
Intersection Summary				STORY.	A FILE		Server to be				
Average Delay			1.6								
Intersection Capacity Utiliza	ation		61.2%		ICU Level	of Service	1.64		В		
Analysis Period (min)			15								

User Entered Value

Synchro 7 - Report

Page 2

	۶	-	*	1	-	*	4	†	-	-	1	1
Movement	EBL	EBT	EBH	-WBE	WBT	WBB	NBL	NBT	NBR:	SBL	SBT	SBF
Lane Configurations	100000000000000000000000000000000000000	र्न	7		4	7	1	11	7	1	† †	7
Volume (vph)	22	7	93	39	3	13	68	1598	30	12	1577	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0	4.0	5.0	5.0	4.0
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		0.96	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1794	1583		1780	1583	1863	3725	1667	1863	3725	1667
Flt Permitted		0.75	1.00		0.71	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1392	1583		1327	1583	1863	3725	1667	1863	3725	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.92	0.92	0.92	0.88	0.88	0.88
Adj. Flow (vph)	29	9	122	42	3	14	74	1737	33	14	1792	116
RTOR Reduction (vph)	0	0	106	0	0	13	0	0	0	0	0	0
Lane Group Flow (vph)	0	38	16	0	45	1	74	1737	33	14	1792	116
Turn Type	Perm		Perm	Perm		Perm	Prot		Free	Prot		Free
Protected Phases		- 4			8		5	2		1	6	
Permitted Phases	4		4	8		8			Free			Free
Actuated Green, G (s)		9.3	9.3		9.3	9.3	8.0	74.7	101.3	2.3	69.0	101.3
Effective Green, g (s)		9.3	9.3		9.3	9.3	8.0	74.7	101.3	2.3	69.0	101.3
Actuated g/C Ratio		0.09	0.09		0.09	0.09	0.08	0.74	1.00	0.02	0.68	1.00
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0		5.0	5.0	
Vehicle Extension (s)		3.0	3.0		3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	Cartelino Seo	128	145		122	145	147	2747	1667	42	2537	1667
v/s Ratio Prot							c0.04	c0.47		0.01	c0.48	
v/s Ratio Perm		0.03	0.01		c0.03	0.00			0.02			0.07
v/c Ratio		0.30	0.11		0.37	0.01	0.50	0.63	0.02	0.33	0.71	0.07
Uniform Delay, d1		42.9	42.2		43.2	41.8	44.7	6.5	0.0	48.7	9.9	0.0
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		1.3	0.3		1.9	0.0	- 2.7	0.5	0.0	4.6	0.9	0.1
Delay (s)		44.2	42.5		45.1	41.8	47.4	7.0	0.0	53.4	10.8	0.1
Level of Service		D	D		D	D	D	Α	Α	D	В	A
Approach Delay (s)		42.9			44.3			8.5			10.5	
Approach LOS		D			D			Α			В	
Intersection Strainacy	i i de leur	Sales (Sales (Sa							5.4.4			
HCM Average Control Delay		9915.CATE-11	11.4	Н	CM Leve	of Service	e	om non mo	В			
HCM Volume to Capacity ratio			0.70									
Actuated Cycle Length (s)			101.3	S	um of los	t time (s)			20.0			
Intersection Capacity Utilization	1		66.8%	IC	U Level	of Service	15.55.2		C			
Analysis Period (min)			15									
c Critical Lane Group												

5		

	•	>	1	†	1	1	
Movement-	EBL	EBR	NBL	NBT.	SBT	SBR	
Lane Configurations		7		44	† †	7	
Volume (veh/h)	0	23	0	1412	1740	82	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.58	0.58	0.88	0.88	0.91	0.91	
Hourly flow rate (vph)	0	40	0	1605	1912	90	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)	NATION OF THE		war and the same				
Upstream signal (ft)							
pX, platoon unblocked		7502220					
vC, conflicting volume	2714	956	1912				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol	0714	050	1912				
vCu, unblocked vol tC, single (s)	2714 6.8	956 *5.9	4.1				
tC, single (s) tC, 2 stage (s)	0.0	5.9	4.1				
tF (s)	3.5	*2.3	2.2			CONTRACTOR	
p0 queue free %	100	91	100				
cM capacity (veh/h)	17	437	306			NEW PARTE DE	
			PARTY OF THE	And a		AN A	
Direction dane (+)	EB 1	NB 1	NB 2	SB 1	SB/2 956	90	
Volume Total Volume Left	0	002	002	950	950	0	
Volume Right	40	0	0	0	0	90	
volume riight	437	1700	1700	1700	1700	1700	
Volume to Capacity	0.09	0.47	0.47	0.56	0.56	0.05	
Queue Length 95th (ft)	7	0.47	0.47	0.50	0.50	0.00	
Control Delay (s)	14.1	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В	0.0	0.0	0.0	The state of the s	9,0	
Approach Delay (s)	14.1	0.0		0.0			
Approach LOS	В	COLUMN TO SERVICE		NAMES OF TAXABLE			
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utiliza	ation		55.7%	10	CU Level	of Service	В
Analysis Period (min)	#7170/AMARIS - NO.		15	Proposition of the Party of the			usultaministerintaksiatie-organitalijanis-stätust Susupassian
areas and a second and a second			Andrew Prince				

User Entered Value

Movement Lane Configurations Volume (veh/h)	COLEGIS		1	- 1	+	4	
	FBL -	FBR	NBL	NBT	SBT	SBR	
Volume (uph/h)		7		^	^	ř	
volume (verim)	0	32	0	1696	1608	101	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.57	0.57	0.95	0.95	0.96	0.96	
Hourly flow rate (vph)	0	56	0	1785	1675	105	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2568	838	1675				
vC1, stage 1 conf vol		HELIOTHER PROPERTY.					
vC2, stage 2 conf vol							
vCu, unblocked vol	2568	838	1675				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)	CHANGE STORY OF						
tF (s)	3.5	*2.3	2.2				
p0 queue free %	100	89	100				
cM capacity (veh/h)	21	512	379				
Directioni Bane 4	LLEB 1	NB 1	NB 2	SRIT	SB2	ISB(3	
Volume Total	56	893	893	838	838	105	
Volume Left	0	0	0	0	0	0	
Volume Right	56	0	0	0	0	105	
cSH	512	1700	1700	1700	1700	1700	
Volume to Capacity	0.11	0.53	0.53	0.49	0.49	0.06	
Queue Length 95th (ft)	9	0	0	0	0	0	
Control Delay (s)	12.9	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В	nan-in-	POTOTO TOTAL	April 19 April 19	AND PARTY OF THE P	DESCRIPTION OF THE PROPERTY OF	
Approach Delay (s)	12.9	0.0		0.0			
Approach LOS	В			OSSPANNIA.			
Intersection/Summary							
Average Delay	. 17	-1.	0.2				
Intersection Capacity Utilizat	ion		52.2%	10	U Level	of Service	A

User Entered Value

	•	1	1	1	†	4	
Movement -	EBL.	E88	NBU	NBT	SBT	SBR	
Lane Configurations	7	7	*	44	11	7	
Volume (vph)	294	133	123	1118	1442	321	
deal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd, Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.90	0.90	0.88	0.88	0.91	0.91	
Adj. Flow (vph)	327	148	140	1270	1585	353	
RTOR Reduction (vph)	0	113	0	0	0	0	
Lane Group Flow (vph)	327	35	140	1270	1585	353	
Turn Type	VL.	Perm	Prot	36.0	1000	Free	The state of the s
Protected Phases	4	THE SHIP	5	2	6		
Permitted Phases	CASTING CAN	4	The state of the	egarra L a		Free	
Actuated Green, G (s)	28.6	28.6	15.2	83.0	62.8	121.6	SAME A PRODUCTION OF THE PERSON STORY
Effective Green, q (s)	28.6	28.6	15.2	83.0	62.8	121.6	
Actuated g/C Ratio	0.24	0.24	0.12	0.68	0.52	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	1.00	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	416	372	233	2543	1924	1667	CONDUCTOR ADMINISTRATION OF THE PROPERTY OF THE
v/s Ratio Prot	c0.18	312	c0.08	0.34	c0.43	1007	
//s Ratio Prot	CU.18	0.02	CU.U8	0.34	CU.43	0.21	
v/s Ratio Perm	0.79	0.02	0.60	0.50	0.82	0.21	
Uniform Delay, d1	43.6	36.4	50.3	9.3	24.7	0.0	
	1.00		1.00	1.00	1.00	1.00	
Progression Factor		1.00			THE PARTY OF THE PARTY.		
Incremental Delay, d2	9.4	0.1	4.3	0.2	3.0	0.3	
Delay (s)	53.1 D	36.5	54.6	9.5	27.7	0.3	
Level of Service	and the same of th	D	D	A	C	A	
Approach Delay (s)	47.9			13.9	22.7		
Approach LOS	D			В	С		
Intersection Summary	4.5 (4.7)	1214-2			MARKET .		
HCM Average Control Dela			22.6	H	CM Leve	l of Service	С
HCM Volume to Capacity re	atio		0.78				
Actuated Cycle Length (s)			121.6	S	um of los	t time (s)	15.0
Intersection Capacity Utiliza	ation		73.1%	10	CU Level	of Service	D
Analysis Period (min)			15				
c Critical Lane Group							

	•	*	1	†	↓	1	
Movement	E (8)1	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	*	7	44	^	7	
Volume (vph)	270	277	233	1426	1266	374	
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Fit Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.89	0.89	0.95	0.95	0.96	0.96	
Adj. Flow (vph)	303	311	245	1501	1319	390	
RTOR Reduction (vph)	0	240	0	0	0	0	
Lane Group Flow (vph)	303	71	245	1501	1319	390	
Turn Type		Perm	Prot			Free	
Protected Phases	4		5	2	6		
Permitted Phases		4		de transfer		Free	
Actuated Green, G (s)	26.3	26.3	21.4	78.7	52.3	115.0	
Effective Green, g (s)	26.3	26.3	21.4	78.7	52.3	115.0	
Actuated g/C Ratio	0.23	0.23	0.19	0.68	0.45	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	405	362	347	2549	1694	1667	
v/s Ratio Prot	c0.17		c0.13	0.40	c0.35		
v/s Ratio Perm		0.04				0.23	
v/c Ratio	0.75	0.20	0.71	0.59	0.78	0.23	
Uniform Delay, d1	41.3	35.8	43.9	9.6	26.5	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	7.4	0.3	6.4	0.4	2.3	0.3	
Delay (s)	48.7	36.1	50.3	9.9	28.8	0.3	
Level of Service	D	D	D	Α	С	Α	
Approach Delay (s)	42.3			15.6	22.3		
Approach LOS	D			В	С		
Intersection Summary			No.		NAME OF THE OWNER.	Water State of the	
HCM Average Control Dela	ay		22.4	H	ICM Leve	I of Service	C
HCM Volume to Capacity r			0.76				
Actuated Cycle Length (s)	erri (Campina (A.)		115.0	S	um of los	t time (s)	15.0
Intersection Capacity Utiliz	ation		73.0%			of Service	C
Analysis Period (min)	NOON THE PERSON NAMED IN		15		TO THE PARTY OF TH	- commontation	ATTACHMENT TO THE GRAND STATE OF THE PARTY OF THE STATE O
c Critical Lane Group							

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5	4/	2	U	1	1	

	1	*	1	-	1	1		
Movement	WEL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	ሻ	7	1		ሻ	†		
Volume (veh/h)	43	60	437	50	21	337		
Sign Control	Stop		Free		D Market Control	Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.83	0.83	0.93	0.93	0.98	0.98		
Hourly flow rate (vph)	52	72	470	54	21	344		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)								
X, platoon unblocked								
vC, conflicting volume	884	497			524			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
Cu, unblocked vol	884	497			524			
C, single (s)	6.4	6.2			4.1			
IC, 2 stage (s)							*	
tF(s)	3.5	3.3			2.2			
p0 queue free %	83	87			98			
cM capacity (veh/h)	310	573			1043			
Direction (Caner#	W84.	WE:2	NB.1	SBif	SB 2			
Volume Total	52	72	524	21	344			
Volume Left	52	0	0	21	0			
Volume Right	0	72	54	0	0			
cSH	310	573	1700	1043	1700			
Volume to Capacity	0.17	0.13	0.31	0.02	0.20			
Queue Length 95th (ft)	15	11	0	2	0			
Control Delay (s)	19.0	12.2	0.0	8.5	0.0			
Lane LOS	C	В		Α				
Approach Delay (s)	15.0		0.0	0.5				
Approach LOS	С							
Intersection Summary								
Average Delay	- Alakin		2.0					
Intersection Capacity Utiliza	ation		36.4%	IC	U Level	of Service	A	
Analysis Period (min)			15					

Lane Configurations 1 F L 1 Volume (veh/h) 56 27 603 54 23 Sign Control Stop Free	SBT
Volume (veh/h) 56 27 603 54 23 Sign Control Stop Free F Grade 0% 0% 0% Peak Hour Factor 0.78 0.78 0.96 0.96 0.84	
Sign Control Stop Free F Grade 0% 0% Peak Hour Factor 0.78 0.78 0.96 0.96 0.84	↑
Grade 0% 0% Peak Hour Factor 0.78 0.78 0.96 0.96 0.84 0	483
Peak Hour Factor 0.78 0.78 0.96 0.96 0.84 (Free
the control of the co	0%
	0.84
	575
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	Contraction of the Contraction o
	None
Median storage veh)	
Upstream signal (ft)	
pX, platoon unblocked vC, conflicting volume 1286 656 684	
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol 1286 656 684 tC. single (s) *5.4 *5.2 4.1	
tC, 2 stage (s)	upper services
tF(s)	
p0 queue free % 77 95 97 cM capacity (veh/h) 307 743 909	
TANGLE MEDICAN MEDICAN CONTROL OF A STREET STATE OF THE S	240000
Direction Lane #1. WB 1 WB12 NB1 SB1 SB2	
Volume Total 72 35 684 27 575	
Volume Left 72 0 0 27 0	
Volume Right 0 35 56 0 0	
cSH 307 743 1700 909 1700	
Volume to Capacity 0.23 0.05 0.40 0.03 0.34	
Queue Length 95th (ft) 22 4 0 2 0	
Control Delay (s) 20.3 10.1 0.0 9.1 0.0	
Lane LOS C B A	
Approach Delay (s) 17.0 0.0 0.4	
Approach LOS C	
Intersection Summary	
Average Delay 1.5	Y THE STREET
Intersection Capacity Utilization 45.0% ICU Level of S	Service

^{*} User Entered Value

APPENDIX G

CAPACITY ANALYSIS CALCULATIONS PROJECTED YEAR 2025 PEAK HOUR TRAFFIC ANALYSIS WITH PROJECT

HCM Unsignalized Intersection Capacity Analysis 3: Kaonoulu & Piilani

5/4/2011

	١	*	4	1	1	1					
Movement	EBL	EBR	INBU	NBT	SBT	SBR			and the second	11/1/19	
Lane Configurations	7	7	7	^	^	7	///				
Volume (veh/h)	18	232	72	1383	1685	11					
Sign Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.84	0.84	0.95	0.95	0.91	0.91					
Hourly flow rate (vph)	21	276	76	1456	1852	12					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				TWLTL	TWLTL						
Median storage veh)				2	2						
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2731	926	1852								
vC1, stage 1 conf vol	1852										
vC2, stage 2 conf vol	879										
vCu, unblocked vol	2731	926	1852								
tC, single (s)	*5.8	*5.9	4.1								
tC, 2 stage (s)	4.8										
tF(s)	*2.5	*2.3	2.2								
p0 queue free %	89	39	77								
cM capacity (veh/h)	190	455	323								
Direction (Lane.#.	E844	EB/24	Nadi	NB:2		SB4	SB/2	SB 8	0023.00		
Volume Total	21	276	76	728	728	926	926	12			
Volume Left	21	0	76	0	0	0	0	0			
Volume Right	0	276	0	0	0	0	0	12			
cSH	190	455	323	1700	1700	1700	1700	1700			
Volume to Capacity	0.11	0.61	0.23	0.43	0.43	0.54	0.54	0.01			
Queue Length 95th (ft)	9	99	22	0	0	0	0	0			
Control Delay (s)	26.3	24.4	19.5	0.0	0.0	0.0	0.0	0.0			
Lane LOS	D	С	С								
Approach Delay (s)	24.6		1.0			0.0					
Approach LOS	С										
Intersection Summary					4,000						
Average Delay	engranda a santa a sa		2.4		National Property and Co.	rondon su sec					
Intersection Capacity Utiliza	ition		65.3%		CU Level	of Service			C		
Analysis Period (min)			15								

User Entered Value

Page 2

	•	7	4	1	1	1					
Movement	EBL	EBB	NBL	NETE	SBT	SBR		et - You day		100	10000000
ane Configurations	ሻ	7	7	^	44	7					
Volume (veh/h)	20	112	123	1673	1747	105					
Sian Control	Stop			Free	Free						
Grade	0%			0%	0%						
Peak Hour Factor	0.79	0.79	0.87	0.87	0.87	0.87					
Hourly flow rate (vph)	25	142	141	1923	2008	121					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type				TWLTL	TWLTL						
Median storage veh)				2	2						
Upstream signal (ft)				HIERON.	2504000						
pX, platoon unblocked				Servanija Kwa							
vC, conflicting volume	3252	1004	2008								
vC1, stage 1 conf vol	2008	1001	2000								
vC2, stage 2 conf vol	1244										
Cu, unblocked vol	3252	1004	2008								
tC, single (s)	*5.8	*5.9	4.1								
IC, 2 stage (s)	4.8	0.0									
tF (s)	2.5	*2.3	2.2								
p0 queue free %	80	65	50	rani, malikatak		SEA BLOKES	St. Classical Co.				
cM capacity (veh/h)	129	409	281								
Direction: Lane #	EB 1	. IEB 2.	NB 1	NB/2	NB 3	SEC	SB 2	SB3			
Volume Total	25	142	141	961	961	1004	1004	121			
Volume Left	25	0	141	0	0	0	0	0			
Volume Right	0	142	0	0	ő	0	ő	121			
cSH	129	409	281	1700	1700	1700	1700	1700			
Volume to Capacity	0.20	0.35	0.50	0.57	0.57	0.59	0.59	0.07			
Queue Length 95th (ft)	17	38	66	0.57	0.57	0.59	0.59	0.07			
Control Delay (s)	39.8	18.4	30.1	0.0	0.0	0.0	0.0	0.0			
Lane LOS	39.8 E	18.4 C	30.1 D	0.0	0.0	U.U	0.0	0.0			
Approach Delay (s)	21.6	U				0.0					
			2.1			0.0					
Approach LOS	С										
Intersection Summary		To Car					2,000,24				
Average Delay	CONTRACTOR OF		1.8	tunida de la	Market Company						
Intersection Capacity Utiliza	ation		65.7%	IC	CU Level	of Service			С		
Analysis Period (min)			15								

 User Entered Valu 		User	Ent	ered	Valu	ue
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Movement	EBL	EBT	EBB	WBL	WBT	WBR	NBL	NBT.	NBR	SBL	SBT	SBE
Lane Configurations		4	7		4	7	7	^	7	*5	^	7
Volume (vph)	42	71	135	154	12	56	49	1357	283	117	1759	41
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0	4.0	5.0	5.0	4.0
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		0.98	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1829	1583		1780	1583	1863	3725	1667	1863	3725	1667
Flt Permitted		0.74	1.00		0.58	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1386	1583		1084	1583	1863	3725	1667	1863	3725	1667
Peak-hour factor, PHF	0.85	0.85	0.85	0.92	0.92	0.92	0.94	0.94	0.94	0.93	0.93	0.93
Adj, Flow (vph)	49	84	159	167	13	61	52	1444	301	126	1891	44
RTOR Reduction (vph)	0	0	75	0	0	48	0	0	0	0	0	0
Lane Group Flow (vph)	0	133	84	0	180	13	52	1444	301	126	1891	44
Turn Type	Perm	- 75	Perm	Perm	11.65	Perm	Prot		Free	Prot		Free
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8			Free			Free
Actuated Green, G (s)		26.4	26.4		26.4	26.4	5.1	67.6	123.0	14.0	76.5	123.0
Effective Green, q (s)		26.4	26.4		26.4	26.4	5.1	67.6	123.0	14.0	76.5	123.0
Actuated g/C Ratio		0.21	0.21		0.21	0.21	0.04	0.55	1.00	0.11	0.62	1.00
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0		5.0	5.0	
Vehicle Extension (s)		3.0	3.0		3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		297	340		233	340	77	2047	1667	212	2317	1667
v/s Ratio Prot							0.03	0.39		c0.07	c0.51	
v/s Ratio Perm		0.10	0.05		c0.17	0.01			0.18			0.03
v/c Ratio		0.45	0.25		0.77	0.04	0.68	0.71	0.18	0.59	0.82	0.03
Uniform Delay, d1		42.0	40.0		45.5	38.2	58.1	20.4	0.0	51.8	17.9	0.0
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		1.1	0.4		14.6	0.0	- 20.9	1.1	0.2	4.4	2.3	0.0
Delay (s)		43.0	40.4		60.1	38.3	79.1	21.5	0.2	56.2	20.2	0.0
Level of Service	ALL PROPERTY.	D	D		E	D	E	С	Α	E	C	A
Approach Delay (s)		41.6			54.6			19.6			22.0	
Approach LOS		D			D			В			С	
Intersection Summary.	A											
HCM Average Control Delay			24.1	Н	CM Leve	of Service	e		С			
HCM Volume to Capacity ratio			0.81									
Actuated Cycle Length (s)			123.0	S	um of los	t time (s)			15.0			
Intersection Capacity Utilizatio	n		77.9%	IC	U Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

	•	*	1	Ť	¥	4	
Movement	EBL	EBR	NBE -	NBT	SBT	SBR	
ane Configurations		7		^	^	7	
/olume (veh/h)	0	23	0	1689	1954	93	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.58	0.58	0.88	0.88	0.91	0.91	
Hourly flow rate (vph)	0	40	0	1919	2147	102	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
X, platoon unblocked							
vC, conflicting volume	3107	1074	2147				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	3107	1074	2147				
tC, single (s)	6.8	*5.9	4.1				
tC, 2 stage (s)							
tF (s)	3.5	*2.3	2.2				
p0 queue free %	100	89	100				
cM capacity (veh/h)	9	372	248				
Direction, Pane #	SEBIL	PNB4	NBI2	SB 1	\$B.2	SB(3)	
Volume Total	40	960	960	1074	1074	102	
Volume Left	0	0	0	0	0	0	
Volume Right	40	0	0	0	0	102	
cSH	372	1700	1700	1700	1700	1700	
Volume to Capacity	0.11	0.56	0.56	0.63	0.63	0.06	
Queue Length 95th (ft)	9	0	0	0	0	0	
Control Delay (s)	15.8	0.0	0.0	0.0	0.0	0.0	
Lane LOS	С						
Approach Delay (s)	15.8	0.0		0.0			
Approach LOS	С						
intersection Summary		Kin Ole					
Average Delay			0.1				

User Entered Value

Synchro 7 - Report

Page 2

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Movement	EBL	FBR	NBL	NBT	SBT	SBR	
Lane Configurations		7		^	**	7	F17
Volume (veh/h)	0	32	0	1875	1798	107	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.57	0.57	0.95	0.95	0.96	0.96	
Hourly flow rate (vph) Pedestrians	0	56	0	1974	1873	111	
Lane Width (ft) Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)			relitation and a		11/2/2001/14		
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2860	936	1873				
vC1, stage 1 conf vol	and the state					and the second second and the second	
vC2, stage 2 conf vol							
vCu, unblocked vol	2860	936	1873				
tC, single (s)	6.8	*5.9	4.1				
IC, 2 stage (s)							
tF (s)	3.5	*2.3	2.2				
p0 queue free %	100	87	100				
cM capacity (veh/h)	13	448	317				
Direction/Lanettz	EB.	NBdz	NB.2	SB 11	SR2	(SB ₃).	
Volume Total	56	987	987	936	936	111	
Volume Left	0	0	0	0	0	0	
Volume Right	56	0	0	0	0	111	
cSH	448	1700	1700	1700	1700	1700	
Volume to Capacity	0.13	0.58	0.58	0.55	0.55	0.07	
Queue Length 95th (ft)	11	0	0	0	0	0	
Control Delay (s)	14.2	0.0	0.0	0.0	0.0	0.0	
Lane LOS	В						
Approach Delay (s)	14.2	0.0		0.0			
Approach LOS	В						

ICU Level of Service

0.2 57.2%

15

Average Delay Intersection Capacity Utilization Analysis Period (min)

	1	*	1	1	1	1	
Movement	EBL	EBR	NBL	SABI	SBT	SBA	
Lane Configurations	ኻ	7	7	44	44	7	
Volume (vph)	294	133	123	1395	1656	321	
Ideal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	1,00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.90	0.90	0.88	0.88	0.91	0.91	
Adj. Flow (vph)	327	148	140	1585	1820	353	
RTOR Reduction (vph)	0	116	0	0	0	0	
Lane Group Flow (vph)	327	32	140	1585	1820	353	
Turn Type	OL!	Perm	Prot	1000	TOLO	Free	CHARLES TO THE PROPERTY OF THE
Protected Phases	4	1 Gilli	5	2	6	60100000000000000000000000000000000000	
Permitted Phases	Marian Maria	4		1000 Merch 5	0	Free	
Actuated Green, G (s)	29.6	29.6	14.2	95.3	76.1	134.9	
Effective Green, g (s)	29.6	29.6	14.2	95.3	76.1	134.9	
Actuated g/C Ratio	0.22	0.22	0.11	0.71	0.56	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	1.00	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
	388	347	196	2632	2101	1667	erskand, poles especiallo de la contrata contrata de la contrata del contrata de la contrata de la contrata del contrata de la contrata del contrata de la contrata de la contrata del contrata de la contrata del contrata del contrata del contrata de la contrata del contrat
Lane Grp Cap (vph)		347	c0.08	0.43		1007	
v/s Ratio Prot v/s Ratio Perm	c0,18	0.02	CU.U0	0.43	c0.49	0.21	
	0.84		0.74	0.00	0.07	0.21	
v/c Ratio	50.4	0.09	0.71	0.60	0.87		
Uniform Delay, d1	1.00	42.0 1.00	58.4 1.00	10.1	25.1	1.00	
Progression Factor							
Incremental Delay, d2	15.2	0.1	11.7	0.4	4.0	0.3	
Delay (s)	65.6	42.1	70.0	10.5	29.1	0.3	
Level of Service	E	D	E	B	C	A	
Approach Delay (s)	58.3			15.3	24.4		
Approach LOS	E			В	С		
intersection/Summary			e kulikan				SHEDOS STAVE THE CONTROL OF THE CONTROL
HCM Average Control Dela			24.5	Н	CM Leve	of Service	C
HCM Volume to Capacity r	atio		0.84				
Actuated Cycle Length (s)			134.9	S	um of los	t time (s)	15.0
Intersection Capacity Utiliza	ation		78.7%	10	CU Level	of Service	D
Analysis Period (min)			15				
c Critical Lane Group							

HCM Signalized Intersection Capacity Analysis

User Entered Value

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	_		7

	•	*	1	1	¥	4	
Movement	EBU.	SEBR.	NBE	NBT	SBT	SBR	Sealer purish the sealer of the sealer
Lane Configurations	ሻ	7	ሻ	^	44	7	
Volume (vph)	270	277	233	1605	1456	374	
deal Flow (vphpl)	1900	1900	2000	2000	2000	2000	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	
Frt .	1.00	0.85	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1770	1583	1863	3725	3725	1667	
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	1770	1583	1863	3725	3725	1667	
Peak-hour factor, PHF	0.89	0.89	0.95	0.95	0.96	0.96	
Adj. Flow (vph)	303	311	245	1689	1517	390	
RTOR Reduction (vph)	0	244	0	0	0	0	
Lane Group Flow (vph)	303	67	245	1689	1517	390	
Turn Type		Perm	Prot			Free	- 100
Protected Phases	4		5	2	6		
Permitted Phases		4				Free	
Actuated Green, G (s)	27.2	27.2	22.0	88.5	61.5	125.7	
Effective Green, g (s)	27.2	27.2	22.0	88.5	61.5	125.7	
Actuated g/C Ratio	0.22	0.22	0.18	0.70	0.49	1.00	
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	383	343	326	2623	1822	1667	
v/s Ratio Prot	c0.17		c0.13	0.45	c0.41		
v/s Ratio Perm		0.04				0.23	
v/c Ratio	0.79	0.20	0.75	0.64	0.83	0.23	
Uniform Delay, d1	46.6	40.3	49.3	10.1	27.7	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	10.6	0.3	9.4	0.5	3.4	0.3	
Delay (s)	57.2	40.6	58.7	10.6	31.1	0.3	
Level of Service	Ε	D	E	В	С	Α	A STATE OF THE STA
Approach Delay (s)	48.8			16.7	24.8		
Approach LOS	D			В	С		
Intersection (Summary							GP Selling Colored Colored
HCM Average Control Delay			24.6	Н	ICM Leve	of Service	С
HCM Volume to Capacity ratio)		0.81				
Actuated Cycle Length (s)			125.7	S	um of los	st time (s)	15.0
Intersection Capacity Utilization	on		78.0%			of Service	D
Analysis Period (min)			15				
c Critical Lane Group							

	1	•	1	1	-	↓		
Movement	WBL	WBB)	NBT	NBB	SBL	SBT		
Lane Configurations	7	7	1>		ሻ	^		0.00
Volume (veh/h)	43	60	479	74	21	369		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.83	0.83	0.93	0.93	0.98	0.98		
Hourly flow rate (vph)	52	72	515	80	21	377		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	974	555			595			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	974	555			595			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)	and the best of the con-	na construction of the			and managed as			and the same of the same
tF(s)	3.5	3.3			2.2			
p0 queue free %	81	86			98			newspanishes
cM capacity (veh/h)	273	531			982			
Direction: Lane #	W8.1	W8 2	NB.ft.	SBI	SB2			The state of
Volume Total	52	72	595	21	377			
Volume Left	52	0	0	21	0			
Volume Right	0	72	80	0	0			
cSH	273	531	1700	982	1700	TORONO CONTRACTOR OF THE PARTY		
Volume to Capacity	0.19	0.14	0.35	0.02	0.22			
Queue Length 95th (ft)	17	12	0	2	0			
Control Delay (s)	21.2	12.8	0.0	8.7	0.0			
Lane LOS	С	В		Α				
Approach Delay (s)	16.3		0.0	0.5				
Approach LOS	C							
Intersection Summary			17.25	200 E		45 Zx 3, 5		
Average Delay			2.0					
Intersection Capacity Utiliza	ation		40.1%	IC	CU Level	of Service	A	
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis

13: Kulanihakoi &

14	1	4	†	-	1	+
Movement _ 35	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	*	1>		ሻ	†
Volume (veh/h)	56	27	636	59	23	510
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.78	0.78	0.96	0.96	0.84	0.84
Hourly flow rate (vph)	72	35	662	61	27	607
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1355	693			724	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1355	693			724	
tC, single (s)	*5.4	*5.2			4.1	
tC, 2 stage (s)						
tF (s)	*2.5	*2.3			2.2	
p0 queue free %	75	95			97	
cM capacity (veh/h)	282	712			879	
Direction Laneur	WBM	AWB 2	NB.1	SBL	SB 2.	
Volume Total	72	35	724	27	607	
Volume Left	72	0	0	27	0	
Volume Right	0	35	61	0	0	
cSH	282	712	1700	879	1700	
Volume to Capacity	0.25	0.05	0.43	0.03	0.36	
Queue Length 95th (ft)	25	4	0	2	0	
Control Delay (s)	22.1	10.3	0.0	9.2	0.0	
Lane LOS	C	В		Α	manage supplier and	
Approach Delay (s)	18.2		0.0	0.4		
Approach LOS	С					
Intersection/Summary		e Agents A.			16 9914	
Average Delay	111.74		1.5			
Intersection Capacity Utiliza	ation		47.1%	IC	U Level	of Service
Analysis Period (min)	C PARTICULAR PROPERTY		15			none EALINE PHILA
CONTROL OF THE PROPERTY OF THE			SVDentics sch			

User Entered Value

BEFORE THE LAND USE COMMISSION

OF THE STATE OF HAWAI'I

In the Matter of the Petition of

DOCKET NO. A11-794

DEPARTMENT OF EDUCATION, STATE OF HAWAI'I,

To Amend the Agricultural Land Use District Boundaries into the Urban Land Use District for Approximately 77.2 acres of land at Kihei, Maui, Hawai'i, Maui Tax Map Key Nos. 2-2-02: 81 and 83. CERTIFICATE OF SERVICE

CERTIFICATE OF SERVICE

I HEREBY CERTIFY THAT a copy of Supplemental Exhibit to Petitioner Department of Education, State of Hawaii's Motion to Amend the Land Use Commission's Findings of Fact, Conclusions of Law and Decision and Order Filed July 29, 2013; Exhibit "38"; was duly served via Electronic Mail upon the following at:

MOANA LUTEY (Moana.Lutey@co.maui.hi.us)
Corporation Counsel
THOMAS KOLBE (Thomas.Kolbe@co.maui.hi.us)
MICHAEL K. HOPPER (Michael.Hopper@co.maui.hi.us)
Deputies Corporation Counsel
County of Maui
200 South High Street
Kalana O Maui Building, 3rd Floor
Wailuku, Maui, Hawai'i 96793

Attorneys for Respondent
DEPARTMENT OF PLANNING, COUNTY OF MAUI
Bryan Yee (Bryan.C.Yee@hawaii.gov)
Alison Kato (Alison.S.Kato@hawaii.gov)
Deputy Attorney General
425 Queen Street

Honolulu, Hawai'i 96813

Attorneys for State Office of Planning and Sustainable Development

DATED: Honolulu, Hawai'i, August 31, 2021.

/s/ Stuart N. Fujioka STUART N. FUJIOKA RYAN W. ROYLO MELISSA J. KOLONIE CARTER K. SIU Deputy Attorneys General

Attorneys for Petitioner
DEPARTMENT OF EDUCATION,
STATE OF HAWAI'I

Docket No. A11-794; In the Matter of the Petition of Department of Education, State of Hawai'i, before the Land Use Commission of the State of Hawai'i; SUPPLEMENTAL EXHIBIT TO PETITIONER DEPARTMENT OF EDUCATION, STATE OF HAWAII'S MOTION TO AMEND THE LAND USE COMMISSION'S FINDINGS OF FACT, CONCLUSIONS OF LAW AND DECISION AND ORDER FILED JULY 29, 2013; EXHIBIT "38"; CERTIFICATE OF SERVICE

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