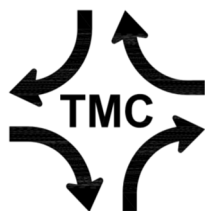


**Updated Traffic Impact Report (TIAR)  
By Traffic Management Consultant**

**Response to State DOT Comments  
Comments by State DOT**

**Response to County DPW Comments  
Comments by County DPW**



## THE TRAFFIC MANAGEMENT CONSULTANT

Randall S. Okaneku, P.E., Principal \* 1188 Bishop Street, Suite 1907 \* Honolulu, Hawaii 96813  
Telephone: (808) 536-0223 \* Facsimile: (808) 537-2985 \* Email: TMCHawaii@aol.com

TMC Job No. 201708

October 3, 2017

State of Hawaii  
Department of Transportation  
Highways Division-Kauai District

1720 Haleukana Street  
Lihu'e, Kauai, Hawai'i 96766

Attn.: Mr. Larry Dill, P.E., District Engineer

Dear Mr. Dill:

**Subject: Traffic Impact Analysis Report Update**  
**For the Proposed Hokua Place**  
**Tax Map Key: (4) 4-3-003: Portion of 001**  
**Kapa'a, Kauai, Hawaii**

Thank you for the review comments in your letter, dated September 29, 2017, on the subject traffic study. Our responses follow:

**Comment No. 1**

Noted.

**Comment No. 2**

Noted.

**Comment No. 3**

The AM and PM Peak Hour Traffic Without Project rows of Table 6 summarize the capacity analysis under existing roadway conditions. The AM and PM Peak Hour Traffic With Project rows of Table 6 summarize the capacity analysis with the recommended site access improvements under Section V.B. of the TIAR Update. The AM and PM Peak Hour Traffic With Project – Improved rows in Table 6 summarize the capacity analysis of the recommended traffic improvements under Section V.A. of the TIAR Update.

**Comment No. 4**

Noted.

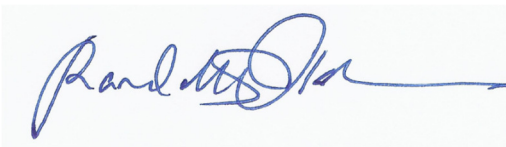
**Comment No. 5**

Noted.

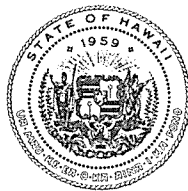
If you require clarification on any of the above material or have any other questions, please do not hesitate to call me.

Very truly yours,

**The Traffic Management Consultant**

By 

**Randall S. Okaneku, P. E.  
Principal**



STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION  
KAUAI DISTRICT  
1720 HALEUKANA STREET  
LIHUE, HAWAII 96766

September 29, 2017

FORD N. FUCHIGAMI  
DIRECTOR

Deputy Directors  
JADE T. BUTAY  
ROSS M. HIGASHI  
EDWIN H. SNIFFEN  
DARRELL T. YOUNG

IN REPLY REFER TO:

HWAY-K 4.170445

Randall S. Okaneku, P.E.  
The Traffic Management Consultant  
1188 Bishop Street, Suite 1907  
Honolulu, Hawaii 96813

Dear Mr. Okaneku:

Subject: Traffic Impact Analysis Report Update  
Hokua Place  
Kapa'a, Kawaihau District, Island of Kaua'i  
TMK: (4) 4-3-03: Por. 001

Thank you for submitting the updated Traffic Impact Analysis Report(TIAR) update that was transmitted via email on June 15, 2017. We have circulated the TIAR for comment through the Highways Division Planning Branch as well as the Traffic Branch. We have also reviewed the comments provided by the County of Kauai Department of Public Works Engineering Division on September 1, 2017.

The combined comments for the Hawaii Department of Transportation Highways Division are as follows:

1. The report discussed the projects that are proposed in the Kapaa Transportation Solutions Report dated August 2015. It should be noted that these projects may not be completed on schedule. Therefore, they should not be considered in this report.
2. It is understood that the proposed Road A will be funded and constructed by the developer.
3. Please clarify the scenarios in Table 7, Summary of Capacity Analysis. What assumed improvements are completed for AM/PM peak hour traffic without project, with project, and with project-improved.
4. Section V of the TIAR recommends traffic improvements without the project. Although these recommendations are appreciated, they are not a consideration for this development.
5. We concur with the comments provided by the County of Kauai Department of Engineering Division.




Mr. Randall Okaneku, P.E.  
September 29, 2017  
Page 2

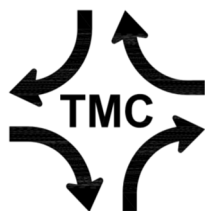
HWY-K 4.170445

Please contact Raymond McCormick at 808-241-3015 by telephone or by email at [Raymond.j.mccormick@hawaii.gov](mailto:Raymond.j.mccormick@hawaii.gov) if you have comments or questions regarding this letter.

Sincerely,



Larry Dill, P.E.  
District Engineer  
For



## THE TRAFFIC MANAGEMENT CONSULTANT

Randall S. Okaneku, P.E., Principal \* 1188 Bishop Street, Suite 1907 \* Honolulu, Hawaii 96813  
Telephone: (808) 536-0223 \* Facsimile: (808) 537-2985 \* Email: TMCHawaii@aol.com

**TMC Job No. 201708**

October 3, 2017

**Department of Public Works  
County of Kauai**

4444 Rice Street, Suite 275  
Lihu'e, Kauai, Hawai'i 96766

Attn.: Mr. Michael Moule, P.E., Chief, Engineering Division

Dear Mr. Moule:

**Subject: Traffic Impact Analysis Report Update**  
**For the Proposed Hokua Place**  
**Tax Map Key: (4) 4-3-003: Portion of 001**  
**Kapa'a, Kauai, Hawaii**

Thank you for the thorough review comments in your letter, dated September 1, 2017, on the subject traffic study. Our responses follow:

**Comment No. 1 – Introduction, Project Description**

- a. Concur. The design of the intersection between the Phase 1 access road and Oloheua Road, mauka of its intersection with Ka'apuni Road, will include the appropriate vertical and horizontal sight distances in accordance with the AASHTO A Policy on Geometric Design of Highways and Streets and the Hawaii Statewide Uniform Design Manual for Streets and Highways.

**Comment No. 2 – Existing Roadways**

- a. Concur. The stated speed limits are intended to provide guidance to the design of the intersection of Road A and the Kapa'a Bypass Road.
- b. Concur.
- c. Concur.

**Comment No. 3 – Existing Peak Hour Traffic Volumes and Operation Conditions**

- a. Noted. The traffic impact analysis is based upon the methodology presented in the Highway Capacity Manual (HCM). The HCM methodology consists of a series of mathematical calculations to determine roadway capacity, vehicle delay, vehicle queuing, etc. The LOS concept was defined in the HCM to translate the results of the complex calculations into a simplified "A" through "F" grading system.

- b. Corrected. The second sentence in the last paragraph on Page 10 should read “South of Ulu Street, Kuhio Highway carried over 1,700 vph...”.
- c. Corrected. The revised Figure 6 is attached. The PM peak hour of traffic from 3:45 PM to 4:45 PM on March 15, 2015 was selected for the intersection of Kuhio Highway and the Kapa`a Bypass Road because it corresponded with of the commuter PM peak hour traffic at the intersections in Kapa`a Town. The revised traffic data sheets for the intersection of Kuhio Highway and Kapa`a Bypass Road also are attached.
- d. LOS, by definition, is the result of a series of mathematical calculations. For the purpose of the traffic impact analysis, the HCM methodology provides a common basis for comparing future traffic conditions without the proposed project and future traffic conditions with the proposed project.

#### **Comment No. 4 – Kapa`a Transportation Solutions**

- a. Noted. The Kapa`a Transportation Solutions, cited in the TIAR Update, is dated August 2015. Please transmit the latest version of the Kapa`a traffic study.
- b. Noted.

#### **Comment No. 5 – Trip Generation Characteristics**

- a. Noted. The revised Table 6 is shown below:

<b>Table 1. Hokua Place Trip Generation Characteristics</b>							
<b>Land Use (ITE Code)</b>	<b>Units</b>	<b>AM Peak Hour (vph)</b>			<b>PM Peak Hour (vph)</b>		
		<b>Enter</b>	<b>Exit</b>	<b>Total</b>	<b>Enter</b>	<b>Exit</b>	<b>Total</b>
<b>Single-Family Phase 1 (265)</b>	16 DU	5	16	21	13	7	20
<b>Single-Family Phase 2 (265)</b>	100 DU	20	60	80	66	38	104
<b>Condominium/ Townhouse (230)</b>	700 DU	52	256	308	244	120	364
<b>Retail Center (820)</b>	8,000 SFGFA	21	13	34	53	57	110
	Pass-By	0	0	0	(-)45	(-)45	(-)90
<b>Total External Trips</b>		<b>98</b>	<b>345</b>	<b>443</b>	<b>331</b>	<b>177</b>	<b>509</b>

- b. The ITE Trip Generation Handbook cites a 9,000-square foot retail center, where 20 percent of the trip generation were primary trips. Comparing the retail center to smaller convenience markets, the Trip Generation Handbook listed sites where the primary trip percentages ranged from 8 percent to 28 percent of the PM peak period trip generation. The retail center is described in the DEIS as a neighborhood-oriented commercial center. Therefore, it is reasonable to assume that a significant portion of the retail trips will be generated from within the proposed project, which can be defined as “internal capture” or “diverted trips”.

**Comment No. 6 – Site Access Improvements**

- a. Noted. The AM and PM peak hour traffic demands at the Olohena Road intersections at the Phase 1 Driveway and at Road A do not meet the AASHTO left-turn lane guidelines. During the AM peak hour of traffic, the advancing (mauka bound) volumes on Olohena Road do not meet the AASHTO minimum requirements. The left-turn demands at Road A and at the Phase 1 Driveway do not meet the AASHTO minimum left-turn volumes, during the PM peak hour of traffic. The Olohena Road intersections at Road A and the Phase 1 Driveway are expected to operate at satisfactory LOS during the AM peak hour of traffic. The Phase 1 Driveway also is expected to operate at satisfactory LOS at Olohena Road, during the PM peak hour of traffic. Road A is expected to operate at LOS “D”, during the PM peak hour of traffic. However, the average delay of 26.7 seconds/vehicle on Road A is in the upper range of LOS “D”. Therefore, a median refuge lane at Road A was not recommended at this time. Furthermore, separate left-turn and right-turn lanes on Road A would not improve the LOS.

**Comment No. 7 – Traffic Assignment**

- a. The traffic assignment for the proposed project was primarily based upon the direction of peak hour traffic at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road, where only about one third of Olohena Road traffic turns to/from the south leg of the Kapa`a Bypass Road. The Phase 2 development is concentrated on the makai half of the project site. Only the trips generated from the mauka-most portion of the site and the estimated AM peak hour school trips are expected to use the mauka access of Road A at Olohena Road.
- b. The peak hour trip destinations, mauka of the Ka`apuni Road/Olohena Road intersection, are virtually nil, as observed in mauka bound/makai bound directional splits on Olohena Road. The retail trips generated from the mauka neighboring communities are represented in the “pass-by” trips using Road A.

**Comment No. 8 – Figures 11 through 14 (Traffic Assignment)**

- a. The diverted peak hour trips on Road A are depicted on the attached Figures 12.1 and 14.1.
- b. The revised Figure 11 is attached.
- c. The revised Figure 13 is attached.
- d. The revised Figure 14 is attached.

**Comment No. 9 – PM Peak Hour Traffic Analysis With Project**

- a. The recommendation of extending the median refuge lane/two-way left-turn lane in Section V.A.7. of the TIAR Update is expected to mitigate the “bottle-neck” on Kuhio Highway, north of Lehua Street. Ultimately, the improvement of the north leg of the Kapa`a Bypass Road from a one-way roadway to a two-way bypass road is expected to improve traffic operations in Kapa`a Town.

**Comment No. 10 – Recommendation of Traffic Improvements Without Project**

- a. Noted.

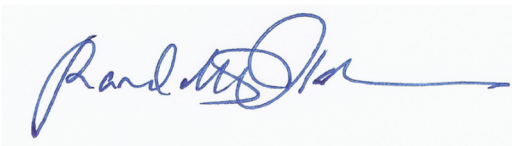
**Comment No. 11 – Recommendation of Traffic Improvements With Project**

- a. Noted. While the MUTCD does not provide warrants for roundabout intersections, it does advise that a roundabout intersection can be considered as an alternative to traffic signal control. Based upon the TIAR Update, the intersection of Olohena Road and Road A is not expected to warrant all-way stop controls or traffic signals. Therefore, a roundabout intersection was not considered. However, a reassessment of the traffic operations at the Road A intersection at Olohena Road may be considered after the project is fully built out and occupied. A roundabout intersection was considered at the intersection of Olohena Road, Ka'apuni Road, and Kaehulua Road. However, based upon a preliminary assessment of the horizontal and vertical alignments of the intersecting roadways, it was determined that a roundabout intersection would not be feasible. The realignment of Kaehulua Road to form a four-legged intersection with the Olohena Road and Ka'apuni Road was recommended in Section V.A.6.

If you require clarification on any of the above material or have any other questions, please do not hesitate to call me.

Very truly yours,

**The Traffic Management Consultant**

By   
**Randall S. Okaneku, P. E.**  
**Principal**

**Attachments:**

Figure 6-Revised  
Kuhio Hwy Kapa'a Bypass Rd Traffic Count Data-Revised  
Figure 12.1  
Figure 14.1  
Figure 11-Revised  
Figure 13-Revised  
Figure 14-Revised

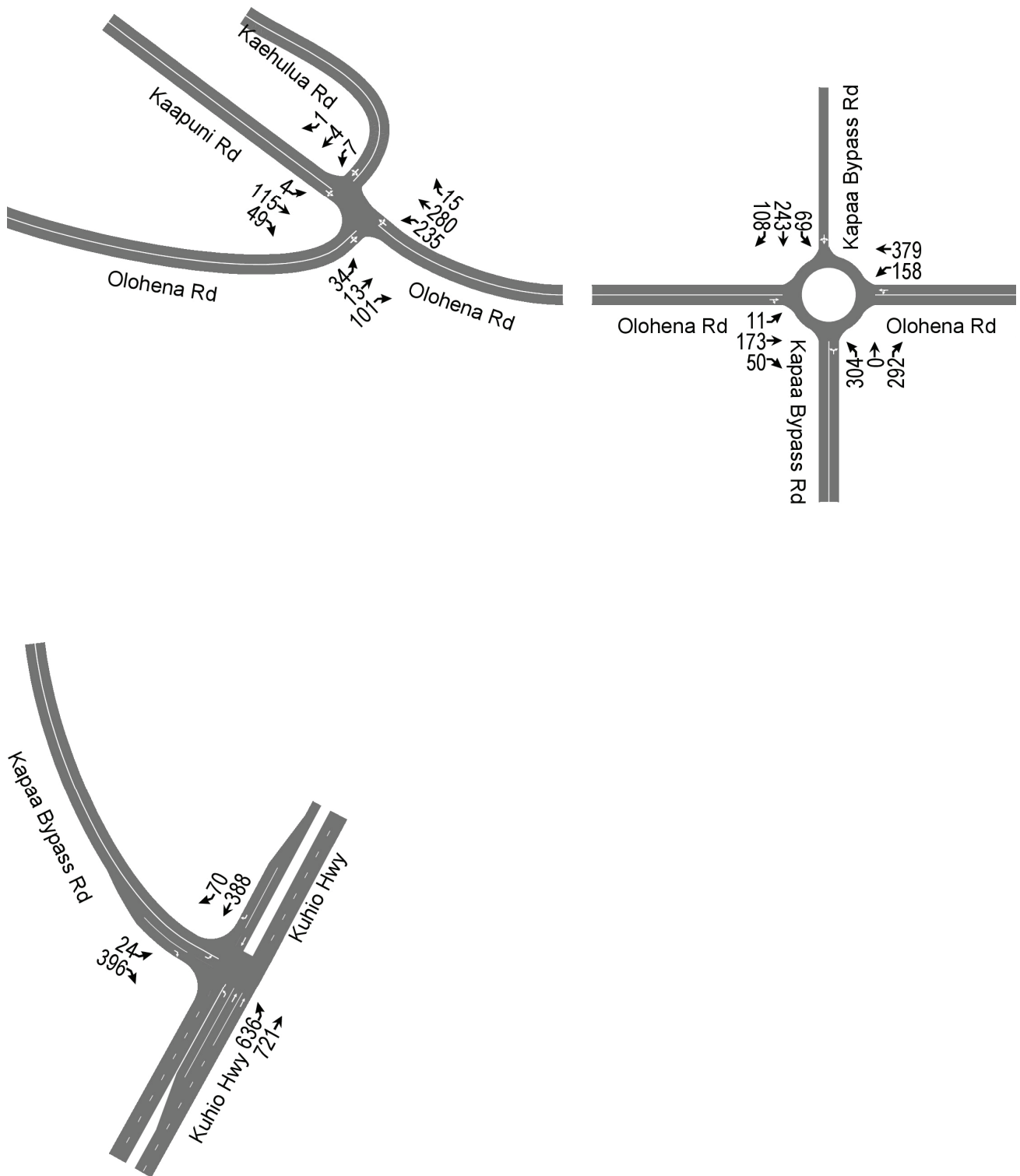
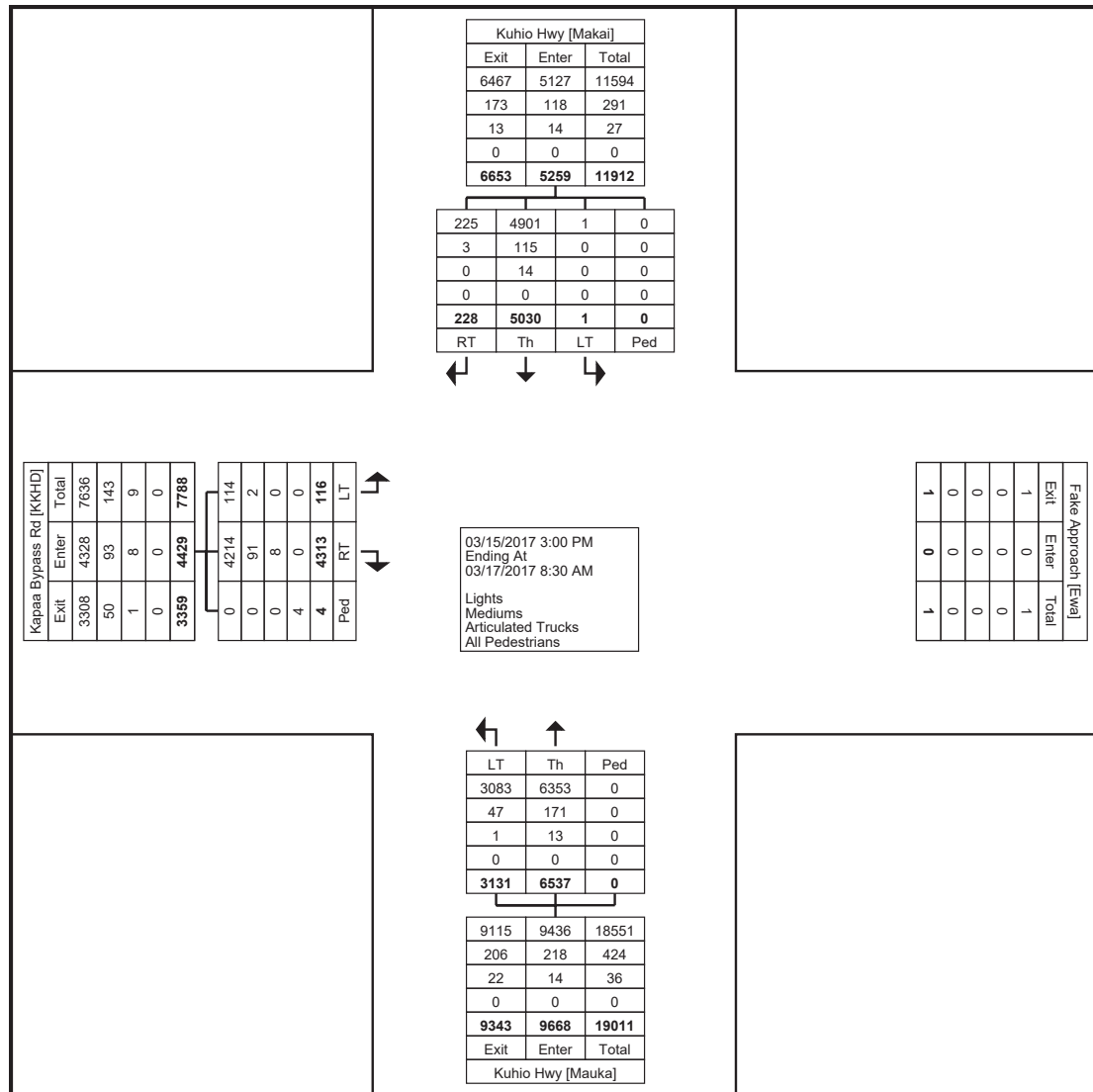


Figure 6. Existing PM Peak Hour Traffic (Cont'd.)

## Turning Movement Data

Start Time	Kapaa Bypass Rd Koko Head Bound				Kuhio Hwy Mauka Bound				Kuhio Hwy Makai Bound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
3:00 PM	1	105	0	106	99	191	0	290	0	106	5	0	111	507
3:15 PM	3	100	0	103	122	210	0	332	0	88	7	0	95	530
3:30 PM	8	93	0	101	120	207	0	327	0	73	8	0	81	509
3:45 PM	8	104	0	112	148	201	0	349	0	88	21	0	109	570
Hourly Total	20	402	0	422	489	809	0	1298	0	355	41	0	396	2116
4:00 PM	1	108	0	109	168	161	0	329	0	91	16	0	107	545
4:15 PM	9	94	0	103	154	172	0	326	0	97	14	0	111	540
4:30 PM	6	90	0	96	166	187	0	353	0	112	19	0	131	580
4:45 PM	2	95	0	97	146	176	0	322	0	112	15	0	127	546
Hourly Total	18	387	0	405	634	696	0	1330	0	412	64	0	476	2211
5:00 PM	5	88	0	93	149	232	0	381	0	138	27	0	165	639
5:15 PM	2	91	0	93	149	192	0	341	0	152	25	0	177	611
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	7	179	0	186	298	424	0	722	0	290	52	0	342	1250
6:30 AM	0	78	0	78	14	124	0	138	0	203	0	0	203	419
6:45 AM	2	116	0	118	8	124	0	132	0	190	1	0	191	441
Hourly Total	2	194	0	196	22	248	0	270	0	393	1	0	394	860
7:00 AM	1	161	0	162	20	129	0	149	0	233	0	0	233	544
7:15 AM	1	184	0	185	25	155	0	180	0	200	1	0	201	566
7:30 AM	2	152	0	154	24	152	0	176	0	167	0	0	167	497
7:45 AM	1	155	1	156	33	180	0	213	0	135	0	0	135	504
Hourly Total	5	652	1	657	102	616	0	718	0	735	1	0	736	2111
8:00 AM	0	150	0	150	24	187	0	211	0	132	1	0	133	494
8:15 AM	3	131	0	134	21	177	0	198	0	165	0	0	165	497
8:30 AM	3	130	0	133	33	191	0	224	0	161	1	0	162	519
8:45 AM	1	108	0	109	25	209	0	234	0	189	0	0	189	532
Hourly Total	7	519	0	526	103	764	0	867	0	647	2	0	649	2042
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	5	103	0	108	97	217	0	314	0	96	6	0	102	524
3:15 PM	8	117	0	125	131	156	0	287	0	84	9	0	93	505
3:30 PM	6	83	0	89	138	227	0	365	1	76	8	0	85	539
3:45 PM	2	87	1	89	119	182	0	301	0	76	7	0	83	473
Hourly Total	21	390	1	411	485	782	0	1267	1	332	30	0	363	2041
4:00 PM	2	122	0	124	126	152	0	278	0	96	7	0	103	505
4:15 PM	6	109	1	115	136	158	0	294	0	95	6	0	101	510
4:30 PM	6	96	1	102	143	174	0	317	0	78	2	0	80	499
4:45 PM	5	93	0	98	138	181	0	319	0	83	6	0	89	506
Hourly Total	19	420	2	439	543	665	0	1208	0	352	21	0	373	2020
5:00 PM	2	98	0	100	146	204	0	350	0	85	3	0	88	538
5:15 PM	4	113	0	117	121	159	0	280	0	92	2	0	94	491
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	6	211	0	217	267	363	0	630	0	177	5	0	182	1029
6:30 AM	0	82	0	82	11	115	0	126	0	185	0	0	185	393
6:45 AM	0	89	0	89	10	126	0	136	0	164	3	0	167	392
Hourly Total	0	171	0	171	21	241	0	262	0	349	3	0	352	785
7:00 AM	1	131	0	132	17	133	0	150	0	219	1	0	220	502
7:15 AM	3	168	0	171	32	158	0	190	0	182	3	0	185	546
7:30 AM	1	125	0	126	40	146	0	186	0	166	2	0	168	480
7:45 AM	1	123	0	124	30	165	0	195	0	138	0	0	138	457
Hourly Total	6	547	0	553	119	602	0	721	0	705	6	0	711	1985
8:00 AM	4	116	0	120	20	169	0	189	0	150	0	0	150	459
8:15 AM	1	125	0	126	28	158	0	186	0	133	2	0	135	447
Grand Total	116	4313	4	4429	3131	6537	0	9668	1	5030	228	0	5259	19356
Approach %	2.6	97.4	-	-	32.4	67.6	-	-	0.0	95.6	4.3	-	-	-
Total %	0.6	22.3	-	22.9	16.2	33.8	-	49.9	0.0	26.0	1.2	-	27.2	-
Lights	114	4214	-	4328	3083	6353	-	9436	1	4901	225	-	5127	18891
% Lights	98.3	97.7	-	97.7	98.5	97.2	-	97.6	100.0	97.4	98.7	-	97.5	97.6
Mediums	2	91	-	93	47	171	-	218	0	115	3	-	118	429
% Mediums	1.7	2.1	-	2.1	1.5	2.6	-	2.3	0.0	2.3	1.3	-	2.2	2.2
Articulated Trucks	0	8	-	8	1	13	-	14	0	14	0	-	14	36
% Articulated Trucks	0.0	0.2	-	0.2	0.0	0.2	-	0.1	0.0	0.3	0.0	-	0.3	0.2
All Pedestrians	-	-	4	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-

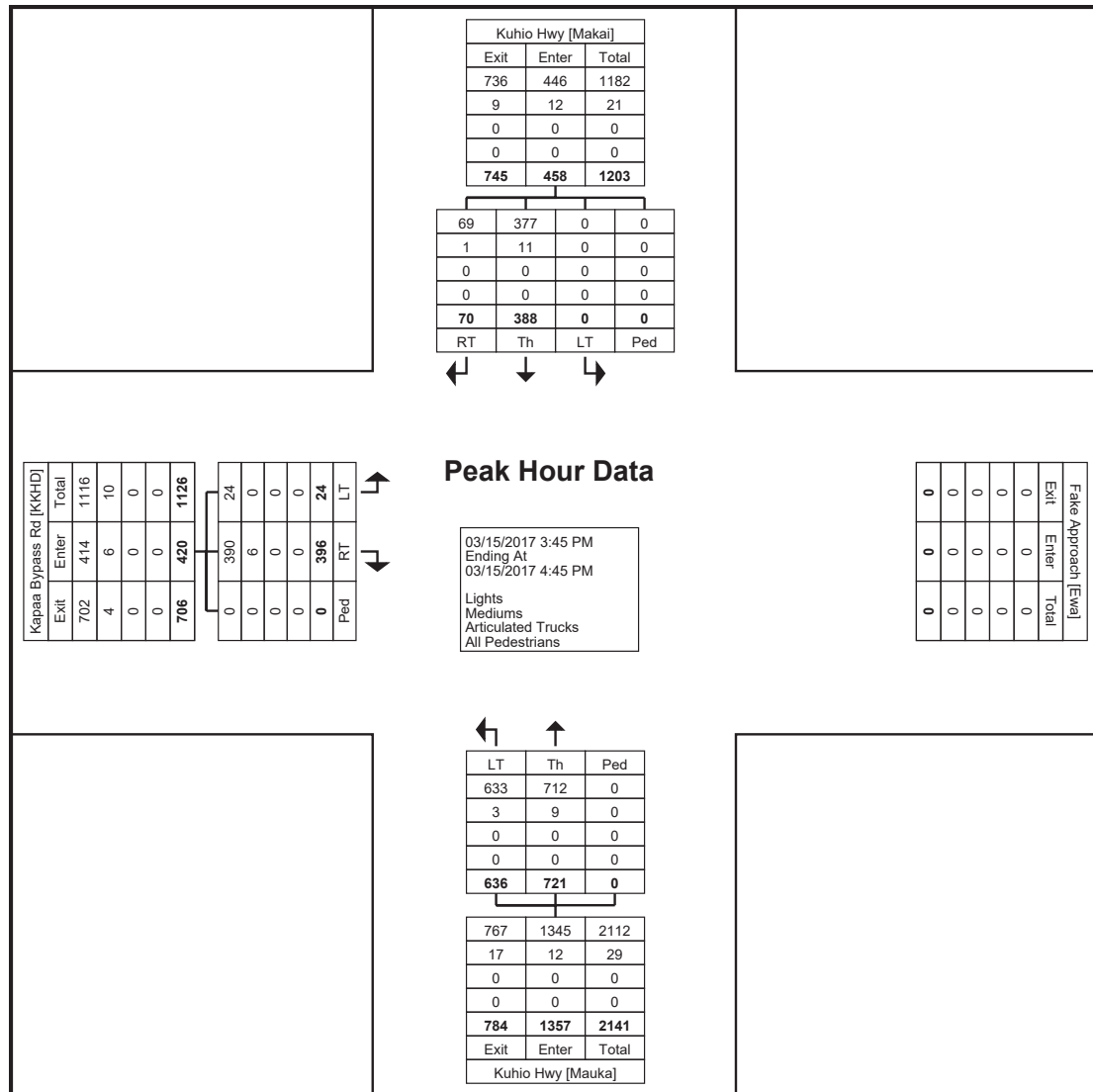


Turning Movement Data Plot



### Turning Movement Peak Hour Data (3:45 PM)

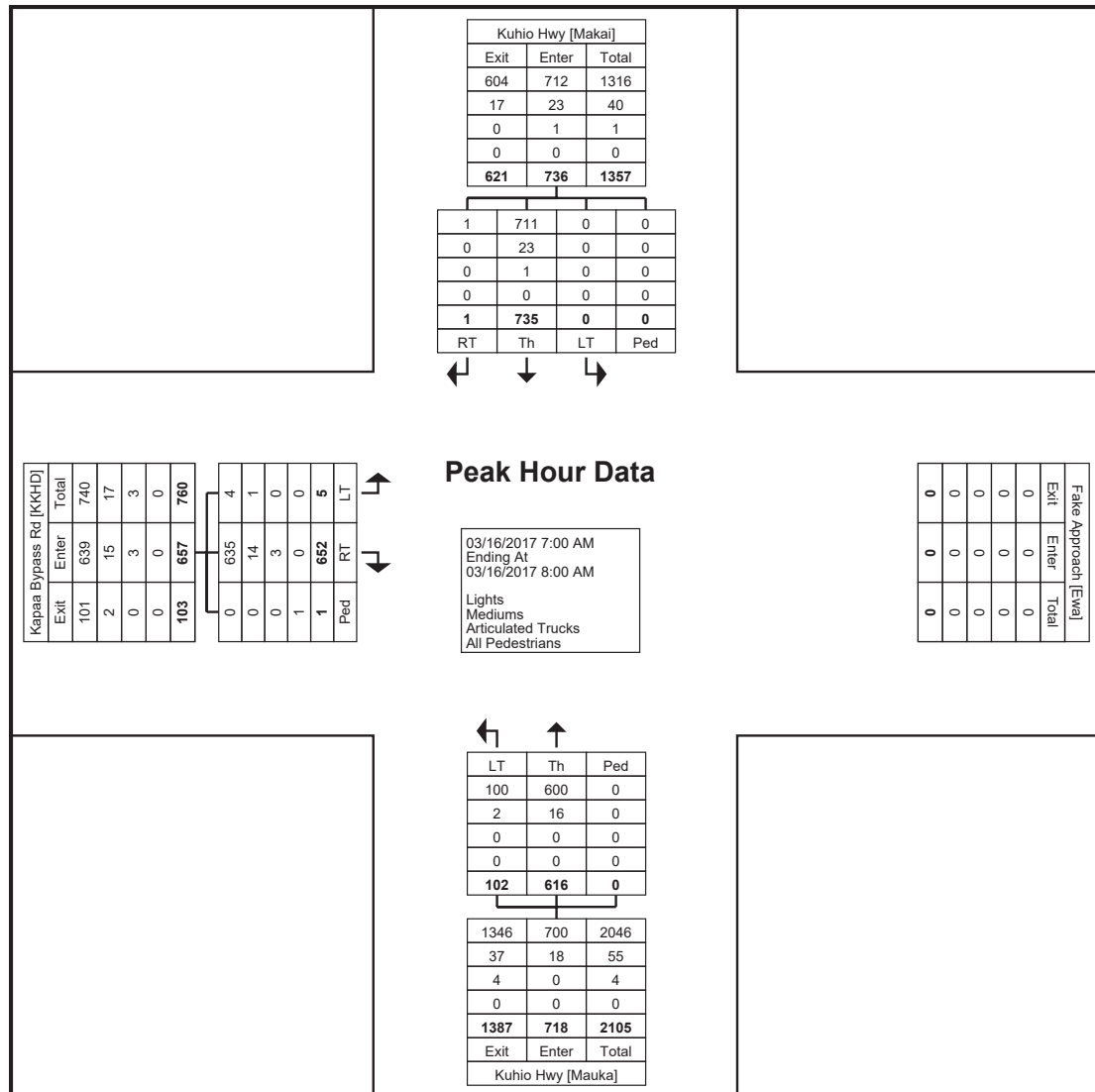
Start Time	Kapaa Bypass Rd Koko Head Bound				Kuhio Hwy Mauka Bound				Kuhio Hwy Makai Bound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
3:45 PM	8	104	0	112	148	201	0	349	0	88	21	0	109	570
4:00 PM	1	108	0	109	168	161	0	329	0	91	16	0	107	545
4:15 PM	9	94	0	103	154	172	0	326	0	97	14	0	111	540
4:30 PM	6	90	0	96	166	187	0	353	0	112	19	0	131	580
Total	24	396	0	420	636	721	0	1357	0	388	70	0	458	2235
Approach %	5.7	94.3	-	-	46.9	53.1	-	-	0.0	84.7	15.3	-	-	-
Total %	1.1	17.7	-	18.8	28.5	32.3	-	60.7	0.0	17.4	3.1	-	20.5	-
PHF	0.667	0.917	-	0.938	0.946	0.897	-	0.961	0.000	0.866	0.833	-	0.874	0.963
Lights	24	390	-	414	633	712	-	1345	0	377	69	-	446	2205
% Lights	100.0	98.5	-	98.6	99.5	98.8	-	99.1	-	97.2	98.6	-	97.4	98.7
Mediums	0	6	-	6	3	9	-	12	0	11	1	-	12	30
% Mediums	0.0	1.5	-	1.4	0.5	1.2	-	0.9	-	2.8	1.4	-	2.6	1.3
Articulated Trucks	0	0	-	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
All Pedestrians	-	-	0	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (3:45 PM)

### Turning Movement Peak Hour Data (7:00 AM)

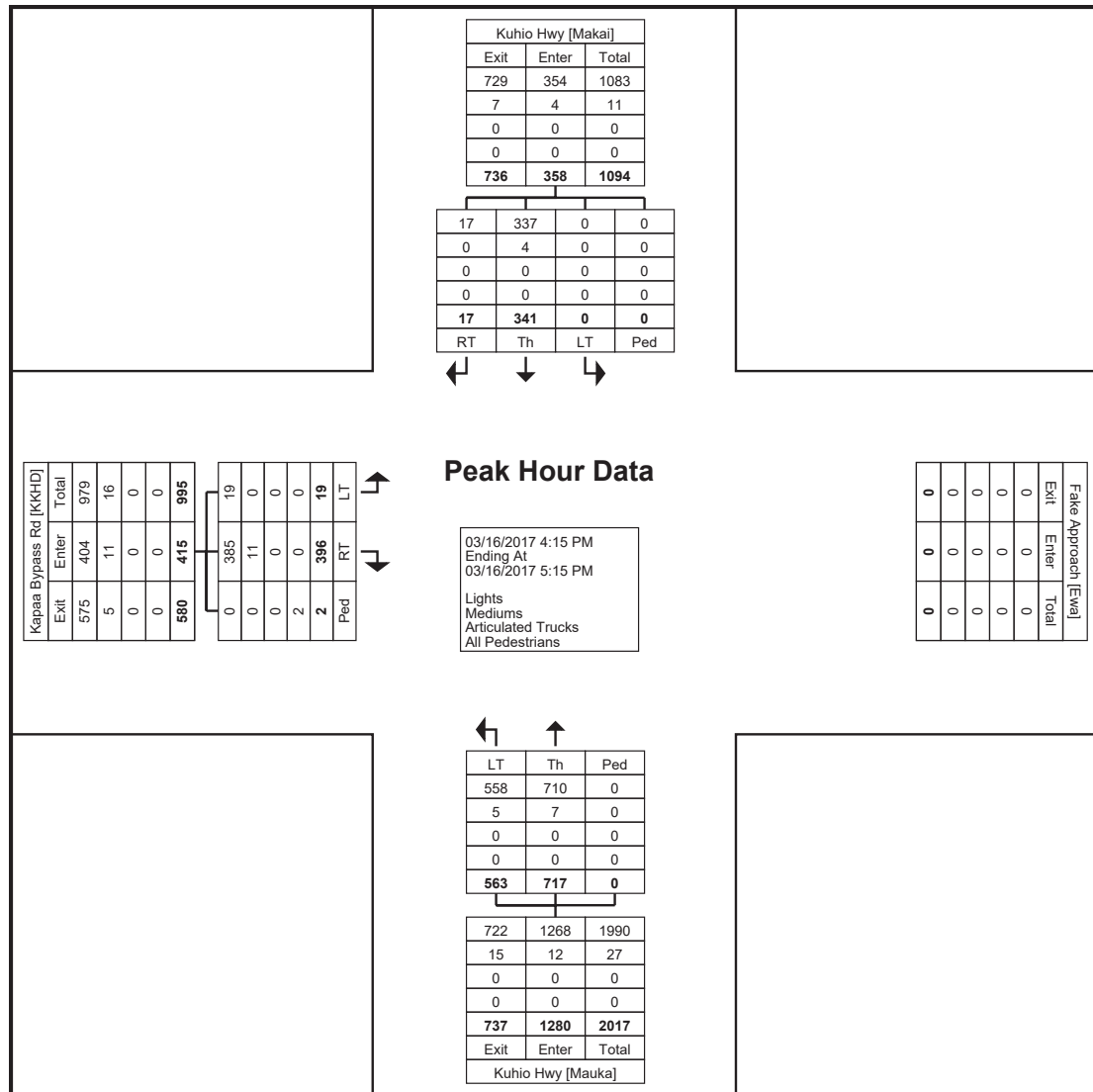
Start Time	Kapaa Bypass Rd Koko Head Bound				Kuhio Hwy Mauka Bound				Kuhio Hwy Makai Bound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:00 AM	1	161	0	162	20	129	0	149	0	233	0	0	233	544
7:15 AM	1	184	0	185	25	155	0	180	0	200	1	0	201	566
7:30 AM	2	152	0	154	24	152	0	176	0	167	0	0	167	497
7:45 AM	1	155	1	156	33	180	0	213	0	135	0	0	135	504
Total	5	652	1	657	102	616	0	718	0	735	1	0	736	2111
Approach %	0.8	99.2	-	-	14.2	85.8	-	-	0.0	99.9	0.1	-	-	-
Total %	0.2	30.9	-	31.1	4.8	29.2	-	34.0	0.0	34.8	0.0	-	34.9	-
PHF	0.625	0.886	-	0.888	0.773	0.856	-	0.843	0.000	0.789	0.250	-	0.790	0.932
Lights	4	635	-	639	100	600	-	700	0	711	1	-	712	2051
% Lights	80.0	97.4	-	97.3	98.0	97.4	-	97.5	-	96.7	100.0	-	96.7	97.2
Mediums	1	14	-	15	2	16	-	18	0	23	0	-	23	56
% Mediums	20.0	2.1	-	2.3	2.0	2.6	-	2.5	-	3.1	0.0	-	3.1	2.7
Articulated Trucks	0	3	-	3	0	0	-	0	0	1	0	-	1	4
% Articulated Trucks	0.0	0.5	-	0.5	0.0	0.0	-	0.0	-	0.1	0.0	-	0.1	0.2
All Pedestrians	-	-	1	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (7:00 AM)

### Turning Movement Peak Hour Data (4:15 PM)

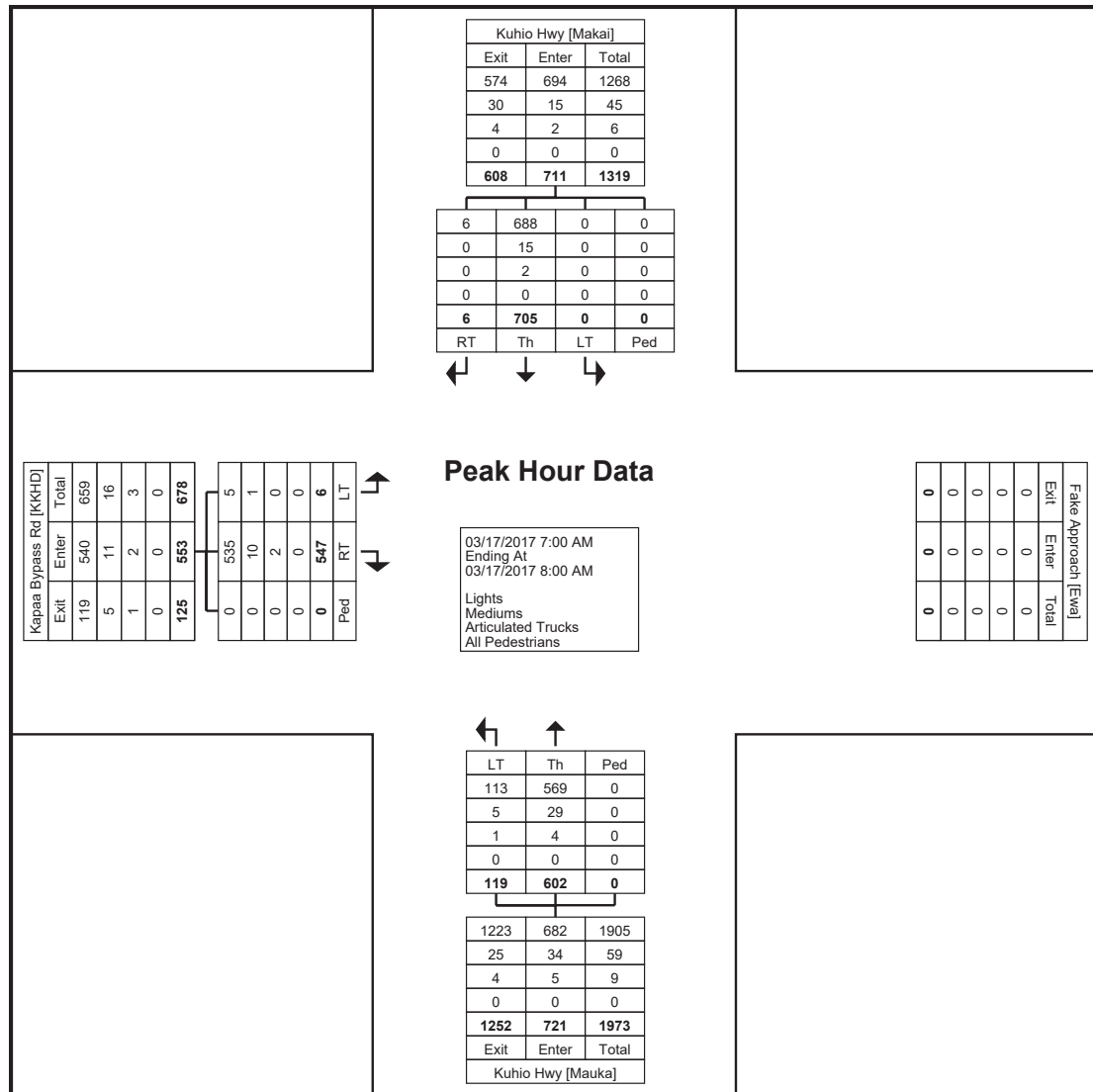
Start Time	Kapaa Bypass Rd Koko Head Bound				Kuhio Hwy Mauka Bound				Kuhio Hwy Makai Bound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
4:15 PM	6	109	1	115	136	158	0	294	0	95	6	0	101	510
4:30 PM	6	96	1	102	143	174	0	317	0	78	2	0	80	499
4:45 PM	5	93	0	98	138	181	0	319	0	83	6	0	89	506
5:00 PM	2	98	0	100	146	204	0	350	0	85	3	0	88	538
Total	19	396	2	415	563	717	0	1280	0	341	17	0	358	2053
Approach %	4.6	95.4	-	-	44.0	56.0	-	-	0.0	95.3	4.7	-	-	-
Total %	0.9	19.3	-	20.2	27.4	34.9	-	62.3	0.0	16.6	0.8	-	17.4	-
PHF	0.792	0.908	-	0.902	0.964	0.879	-	0.914	0.000	0.897	0.708	-	0.886	0.954
Lights	19	385	-	404	558	710	-	1268	0	337	17	-	354	2026
% Lights	100.0	97.2	-	97.3	99.1	99.0	-	99.1	-	98.8	100.0	-	98.9	98.7
Mediums	0	11	-	11	5	7	-	12	0	4	0	-	4	27
% Mediums	0.0	2.8	-	2.7	0.9	1.0	-	0.9	-	1.2	0.0	-	1.1	1.3
Articulated Trucks	0	0	-	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
All Pedestrians	-	-	2	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (4:15 PM)

### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Kapaa Bypass Rd Koko Head Bound				Kuhio Hwy Mauka Bound				Kuhio Hwy Makai Bound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:00 AM	1	131	0	132	17	133	0	150	0	219	1	0	220	502
7:15 AM	3	168	0	171	32	158	0	190	0	182	3	0	185	546
7:30 AM	1	125	0	126	40	146	0	186	0	166	2	0	168	480
7:45 AM	1	123	0	124	30	165	0	195	0	138	0	0	138	457
Total	6	547	0	553	119	602	0	721	0	705	6	0	711	1985
Approach %	1.1	98.9	-	-	16.5	83.5	-	-	0.0	99.2	0.8	-	-	-
Total %	0.3	27.6	-	27.9	6.0	30.3	-	36.3	0.0	35.5	0.3	-	35.8	-
PHF	0.500	0.814	-	0.808	0.744	0.912	-	0.924	0.000	0.805	0.500	-	0.808	0.909
Lights	5	535	-	540	113	569	-	682	0	688	6	-	694	1916
% Lights	83.3	97.8	-	97.6	95.0	94.5	-	94.6	-	97.6	100.0	-	97.6	96.5
Mediums	1	10	-	11	5	29	-	34	0	15	0	-	15	60
% Mediums	16.7	1.8	-	2.0	4.2	4.8	-	4.7	-	2.1	0.0	-	2.1	3.0
Articulated Trucks	0	2	-	2	1	4	-	5	0	2	0	-	2	9
% Articulated Trucks	0.0	0.4	-	0.4	0.8	0.7	-	0.7	-	0.3	0.0	-	0.3	0.5
All Pedestrians	-	-	0	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (7:00 AM)



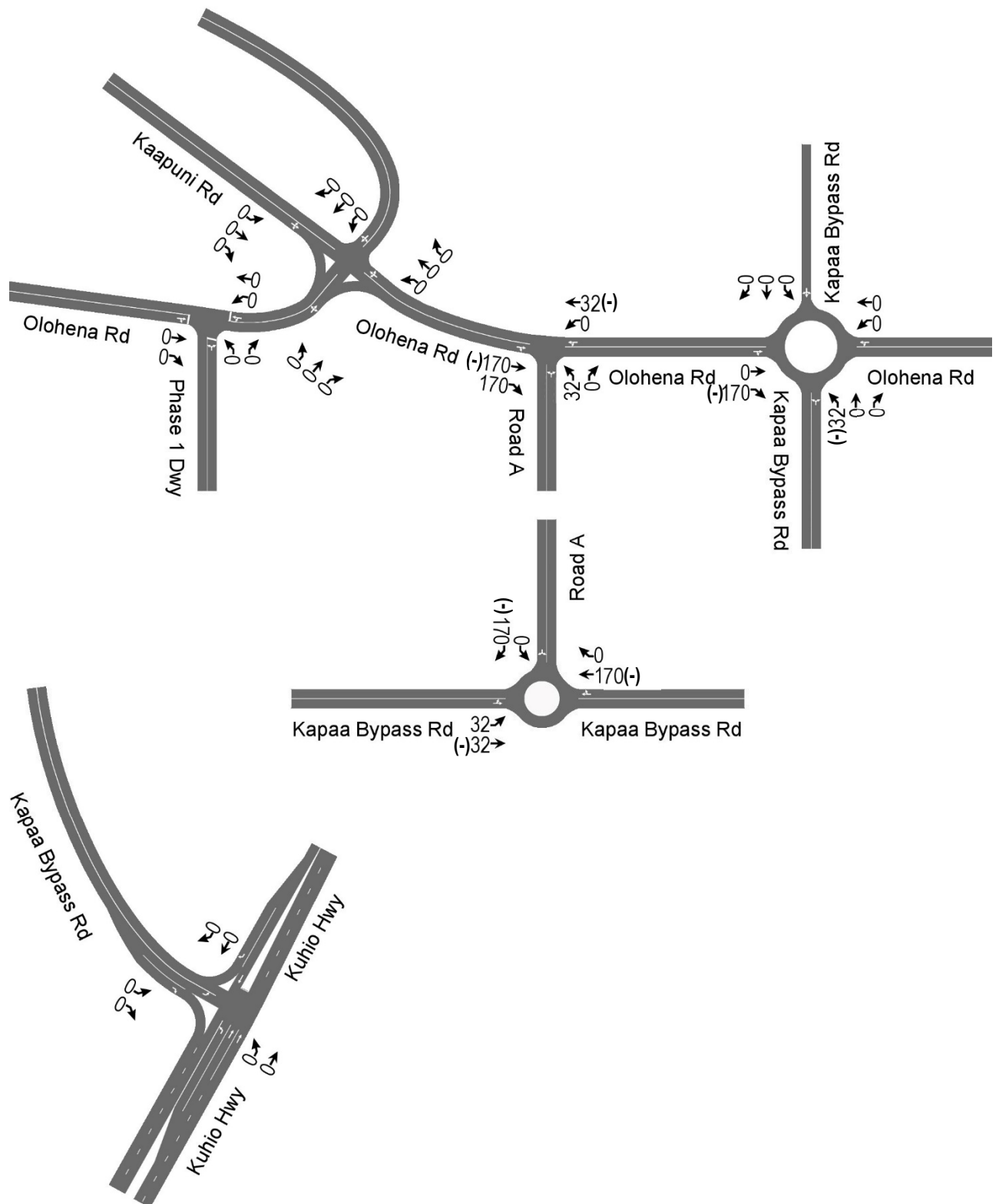
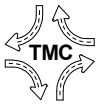


Figure 12.1 AM Peak Hour Diverted Traffic Assignment

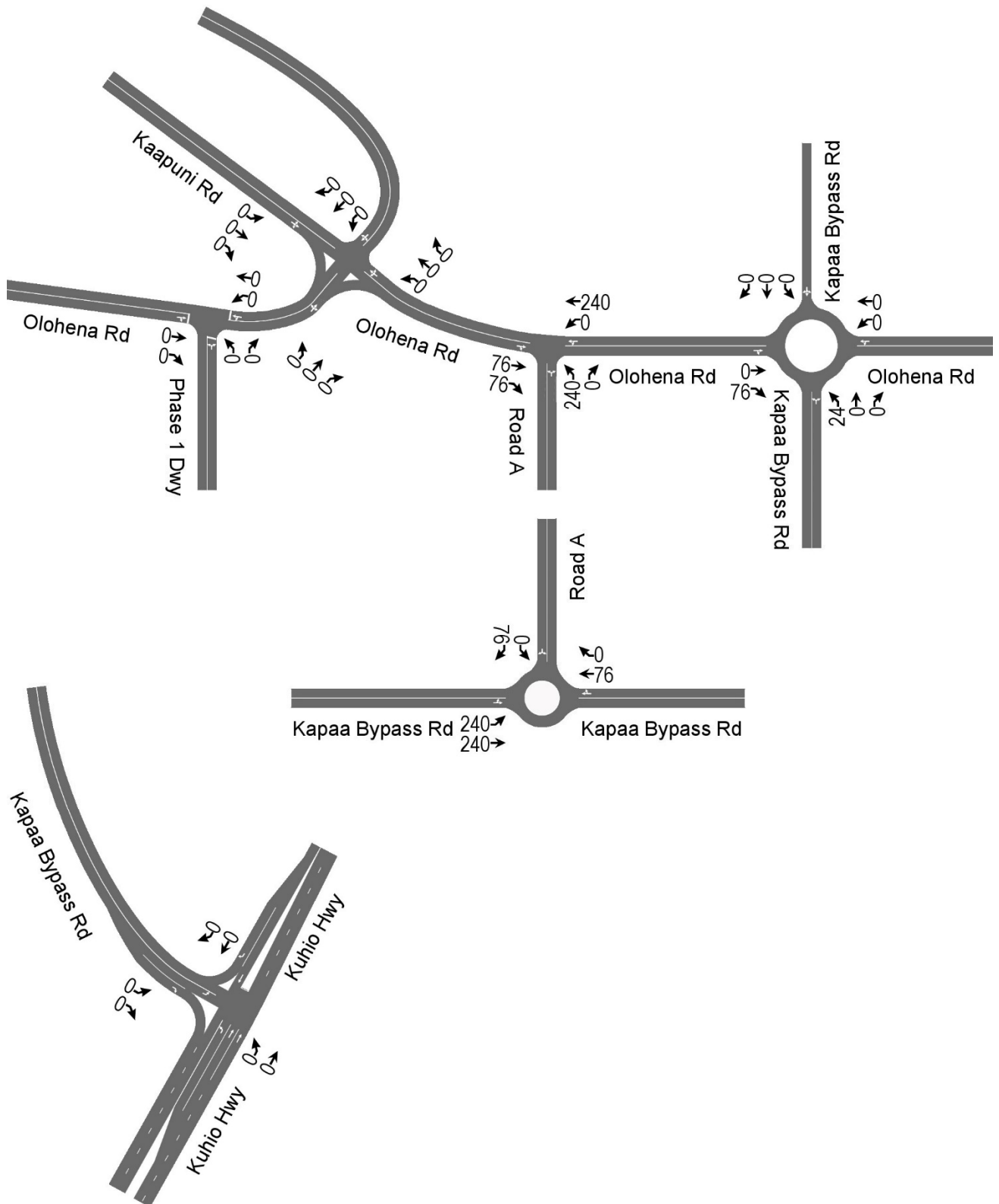
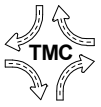


Figure 14.1 PM Peak Hour Diverted Traffic Assignment

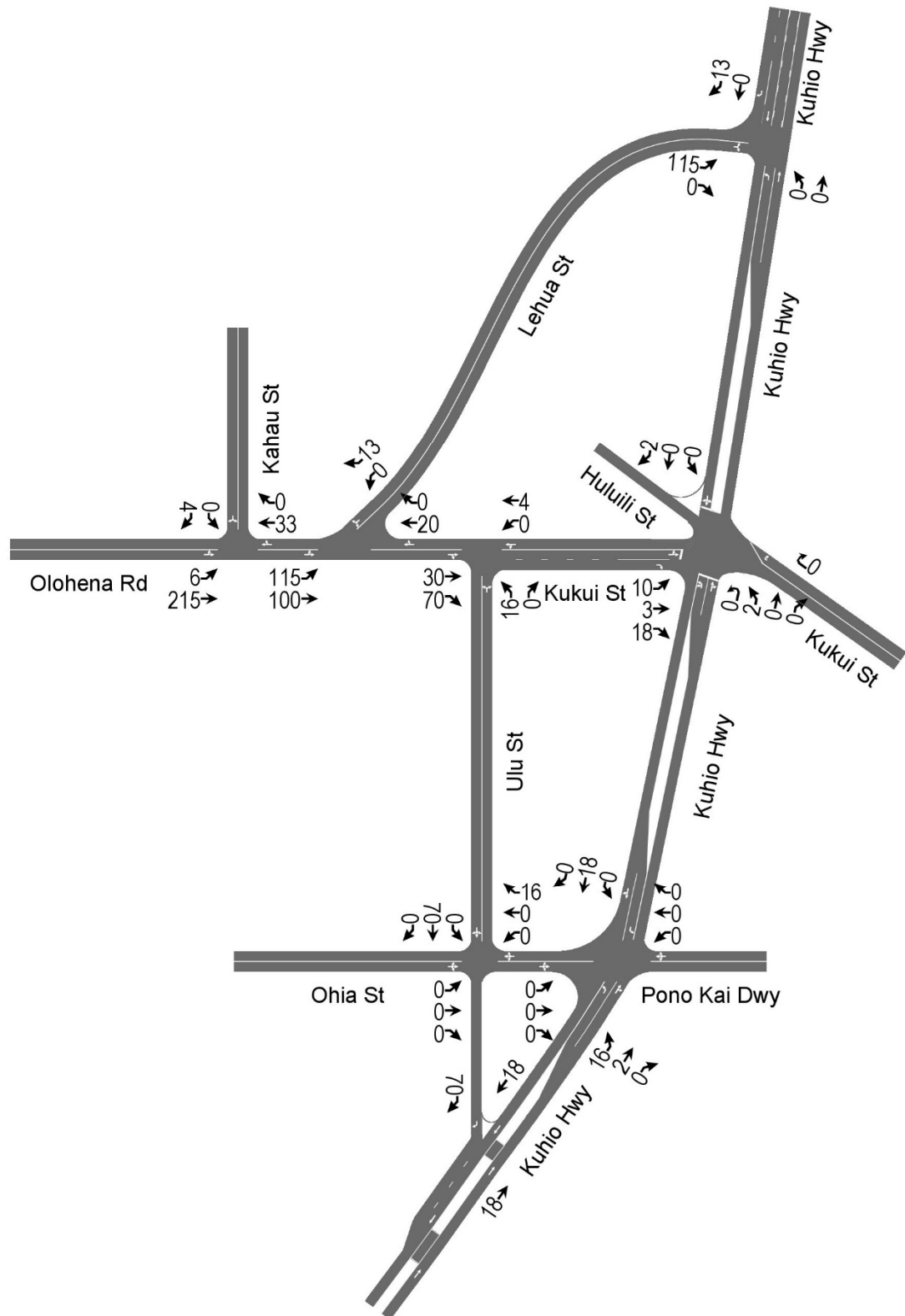
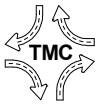


Figure 11. AM Peak Hour Site Traffic Assignment

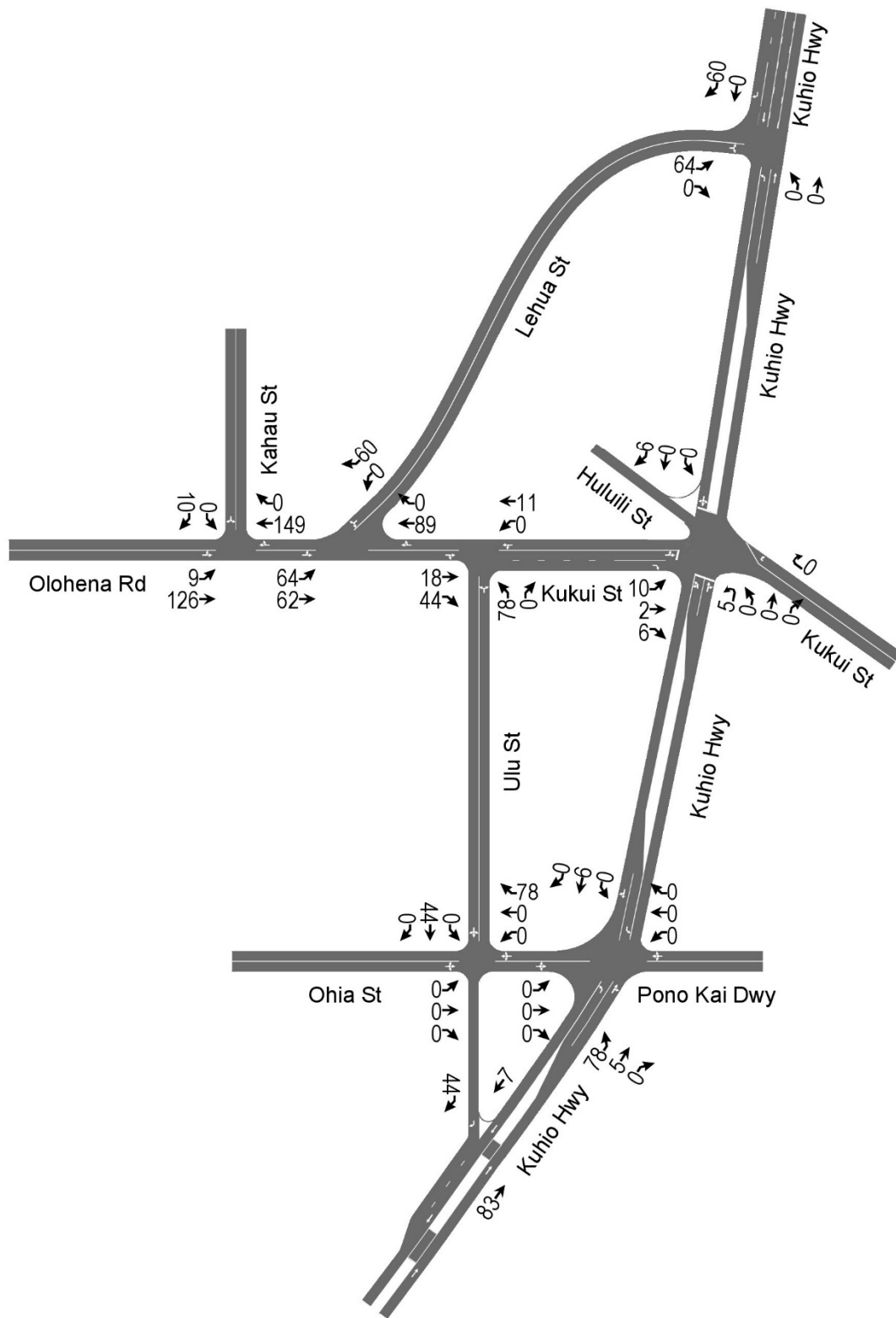


Figure 13. PM Peak Hour Site Traffic Assignment



30

**Bernard P. Carvalho, Jr.**  
Mayor



**Lyle Tabata**  
Acting County Engineer

**Wallace G. Rezentes, Jr.**  
Managing Director

**DEPARTMENT OF PUBLIC WORKS**

**County of Kaua'i, State of Hawai'i**

4444 Rice Street, Suite 275, Lihu'e, Hawai'i 96766  
TEL (808) 241-4992 FAX (808) 241-6604

September 1, 2017

Randall S. Okaneku, P. E.  
The Traffic Management Consultant  
1188 Bishop Street, Suite 1907  
Honolulu, Hawaii 96813

**SUBJECT:** Traffic Impact Analysis Report Update  
For the Proposed Hokua Place  
Kapa'a, Kawaihau District, Island of Kaua'i  
TMK: (4) 4-3-03: Por. 001

Dear Mr. Okaneku:

The Engineering Division of the Department of Public Works received the subject Traffic Impact Analysis Report (TIAR) Update that was transmitted via email on June 15, 2017. We appreciate the opportunity to review the TIAR and offer the following comments on the TIAR:

**1. Introduction, Project Description:**

- a. The TIAR indicates that the driveway for phase 1 is proposed to be located on Oloheua Road mauka of its intersection with Ka'apuni Road. We have concerns with a proposed intersection at this location, including the proximity to the intersection of Ka'apuni Road as well as concerns about intersection sight distance due to nearby horizontal and vertical curves. Prior to approval of a driveway at this location, additional information will need to be provided about this driveway location, to show that appropriate sight lines can be achieved and that no safety or other problems will be created by the proximity to the intersection of Oloheua Road and Ka'apuni Road.

**2. Existing Conditions, Roadways:**

- a. The report states that the Kapa'a Bypass Road speed limit is reduced to 25 mph south of the proposed intersection with Road A. The report should also mention that further south the speed limit is again increased to 35 mph.
- b. The report incorrectly indicates that the posted speed limit for Oloheua Road is reduced to 15 mph as it approaches Kapa'a Middle School. The correct statement should be that there is a 15 mph school zone within the vicinity of Kapa'a Middle School during school hours.
- c. Kukui Street and Ulu Street should both be described as collector streets.

**3. Existing Conditions, Existing Peak Hour Traffic Volumes and Operating Conditions:**

- a. The language throughout this segment of the TIAR indicates that intersections “operated at LOS....” However, if we understand correctly, the LOS values given are based on the analysis of the traffic conditions, not actual empirical observations of delay for vehicles at these intersections. The TIAR should instead use language such as “calculated to operate at LOS ....” This is an important distinction given that observations of Kūhiō Highway during peak hours of traffic appear to show LOS along the highway worse than the LOS A for movements along Kūhiō Highway as reported in the TIAR, potentially due to other factors than the control delay at the intersections.
- b. Check the traffic volume of 1,500 shown on page 10 for Kūhiō Highway south of Ulu Street in the PM Peak. The volumes shown in Figure 6 do not match.
- c. Figure 6 (Existing PM Peak Hour Traffic) has an error for the southbound through movement on Kūhiō Highway at the Kapa‘a Bypass Road. The figure shows an hourly volume of 38, which is way too low for this through movement. The data shown for this intersection in figure 6 does not appear to match either of the two PM peak hour traffic count plots (or their average) in the appendix.
- d. Related to comment “a” above recommending different language for the calculated LOS values, we recommend that the TIAR include some statements comparing the observed traffic conditions with the calculated delays and level of service, ideally offering explanations for the difference in observed level of service and calculated level of service.

**4. Future Traffic Conditions, Kapa‘a Transportation Solutions:**

- a. Page 17 of the TIAR refers to removal of on-street parking on Kūhiō Highway. The Kapa‘a Transportation Solutions study rejected any potential solutions that removed parking on Kūhiō Highway, since such a change would be detrimental to the economic vitality, multimodal, and safety goals of the study. Removal of parking should not be discussed in the TIAR, as HDOT is not considering removal of parking to add travel lanes or turn lanes.
- b. With respect to a new connector road in the approximate location of Road A, page 18 of the TIAR states, “The construction cost of the connector road was estimated at \$25,824,000.” The costs in the Kapa‘a Transportation Solutions report include right-of-way costs as well as construction cost; therefore it is misleading to state that the full cost shown in the study is the estimated construction cost.

**5. Traffic Impact Analysis, Trip Generation Characteristics:**

- a. The project description in the TIAR’s introduction states that there are 700 multi-family dwelling units, but the trip generation calculations are based on 800 multi-family dwelling units. This discrepancy must be corrected, and the accurate trip generation should be reflected in the study.
- b. The pass-by trip percentage of 81.2% is too high, especially given the relatively small amount of traffic traveling through the development on Road A. The diverted volume of 45 vehicles represents approximately 15% of the estimated through vehicles on Road A during the PM Peak Hour. The 8,000 square feet of the Hokua Place shopping center is outside of the sample size in the pass-by trip

chart for shopping centers in the ITE Trip Generation Handbook. A pass-by trip percentage of approximately 30% or 40% would be more reasonable, given the data available in the Trip Generation Handbook. It would also be reasonable for the TIAR to include a calculation of an internal capture rate for trips between the retail portion and the residential portion of the Hokua Place development. However, the combination of the traffic reduction for internal capture and pass-by trips should still be less than 81%.

**6. Traffic Impact Analysis, Site Access Improvements:**

- a. The recommendations for the stop controlled Tee-intersections of Olohena Road with Road A and the phase 1 driveway do not include any statements regarding the recommended lane assignments for these new intersections. The methodologies section of the report describes the use of AASHTO Left-Turn Lane Guidelines, but no such analyses are included in the TIAR for left turn lanes on Olohena Road at these intersections. We believe that at a minimum, a left turn lane would be necessary on Olohena Road at Road A, but analyses must be provided for both intersections. A median refuge lane should also be included on Olohena Road to facilitate the left-turn movement from Road A to Olohena Road. In addition, we believe that Road A should have two approach lanes at Olohena Road, one for right turn movements and one for left turn movements.

**7. Traffic Impact Analysis, Traffic Assignment:**

- a. In the previous TIAR for this project, no traffic was assigned to the left turn movement from southbound Road A to eastbound Kapa'a Bypass (and likewise for the right turn from the Kapa'a Bypass to Road A). In our earlier comments, we recommended that some traffic be assigned to these movements. In almost a complete reversal, the current TIAR assigned nearly all of the traffic to these movements. In the current TIAR, only about 5% to 10% of the project traffic that goes through the existing Kapa'a Bypass roundabout is assigned to go through the intersection of Road A and Olohena Road. A more equitable distribution of traffic should be made, to accurately represent the traffic impact on Olohena Road.
- b. The TIAR assigns no traffic between the project and Olohena Road or Ka'apuni Road north of the project (Wailua Homesteads and Upper Kapahi area). There are relatively few destinations on those roads for the residential traffic from the project, but a small amount of residential traffic is likely to travel to those areas. In addition, much of the traffic generated by the retail portion of the development would have its origin or destination in the residential areas of Wailua Homesteads and Upper Kapahi area. A reasonable (albeit small) amount of traffic must be assigned to those areas.

**8. Figures 11 Through 14 (Traffic Assignment)**

- a. For clarity, the TIAR must show the reassignment of existing traffic on separate figures from the figures for traffic assignment from this project.
- b. On Figure 11, the 989 vehicles shown for northbound Kūhiō Highway at Ulu Street is incorrect. It appears that this volume should be 20.
- c. On Figure 13, the 1,274 vehicles shown for northbound Kūhiō Highway at Ulu Street is incorrect. It appears that this volume should be 92.
- d. On Figure 14, the 30 vehicles shown for the Kapa'a Bypass Road left turn and the



447 vehicles for the Kapa‘a Bypass Road right turn appear to be incorrect.

**9. Traffic Impact Analysis, PM Peak Hour Traffic Analysis With Project:**

- a. We recommend that the TIAR further analyze and discuss the impact of the project on the intersection of Kūhiō Highway and Lehua Street and recommend measures to mitigate this impact. The TIAR states that “Makai bound Lehua Street is expected to continue at LOS F at Kūhiō Highway during the PM peak hour of traffic with the proposed project.” However, Table 7 shows the PM peak hour of traffic without the project to be LOS E. Additionally, while the AM peak hour of traffic with the project continues to be LOS F, the delay increases significantly.

**10. Recommendations and Conclusions, Recommended Traffic Improvements Without Project:**

- a. Item number 3 recommends restricting parking along Kūhiō Highway within Kapa‘a Town in order to provide additional through lanes or left turn lanes on Kūhiō Highway. This should not be recommended in the TIAR, because HDOT is not considering removal of parking to add travel lanes. Removal of parking has been determined to be detrimental to businesses and the economic vitality of Kapa‘a Town. Discussion of parking removal on Kūhiō Highway in Kapa‘a Town should also be removed from other sections of the report, including the conclusions.

**11. Recommendations and Conclusions, Recommended Traffic Improvements With Project:**

- a. Our comments above include several concerns about the intersection of Road A and Olohena Road, including the possibility that additional traffic should be assigned to this intersection. We are concerned that the one-way stop control Tee-intersection proposed will not be sufficient to address traffic operations and safety at intersection. The installation of a roundabout at this intersection shall be evaluated as part of the TIAR, including traffic operations analysis for a roundabout as well as a safety comparison of a roundabout and a one-way stop control intersection. The federal Manual on Uniform Traffic Control Devices (MUTCD) does not include traffic warrants for roundabouts. However, evaluation of the MUTCD’s multi-way stop control warrants and/or signal warrants would be instructive with respect to evaluating whether a one-way stop control intersection would be sufficient or if a roundabout is needed instead. Alternatively, we may also accept an evaluation of the need for a roundabout based on roundabout evaluation guidelines from another jurisdiction or research document. Consideration should also be given to the construction of a roundabout that combines the intersections of Olohena Road with Ka‘apuni Road and Road A (with Kaehulua Road designed as a T intersection with either Ka‘apuni Road or Olohena Road). Traffic operations analysis of a roundabout that combines these intersections shall be included in the TIAR.

The comments in this letter should not be construed to be inclusive of all County of Kaua‘i recommendations for road improvements required to be constructed as part of the Hokua Place

Mr. Randall Okaneku  
September 1, 2017  
Page 5

project. Recommendations and requirements for road improvements will be included as part of future review phases for the project, such as zoning amendments, subdivision applications, and construction plan review. If you have any questions or need additional information, please contact me at (808) 241-4891 or Stanford Iwamoto at (808) 241-4896.

Very truly yours,

A handwritten signature in black ink, appearing to read 'Michael Moule', is written over a horizontal line.

MICHAEL MOULE, P.E.  
Chief, Engineering Division

MM/SI

Copies to: DPW-Design & Permitting  
Lyle Tabata, Acting County Engineer  
Larry Dill, HDOT Kaua'i District Engineer

**TRAFFIC IMPACT ANALYSIS REPORT UPDATE**

**FOR THE PROPOSED**

**HOKUA PLACE**

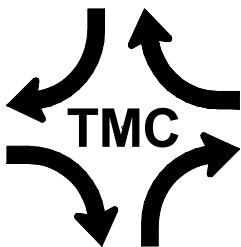
**KAPA`A, KAUAI, HAWAII**

**TAX MAP KEY: (4) 4-3-03: 01**

**PREPARED FOR**

**HG KAUAI JOINT VENTURE, LLC**

**MAY 22, 2017**



**PREPARED BY**

**THE TRAFFIC MANAGEMENT CONSULTANT**

# TRAFFIC IMPACT ANALYSIS REPORT UPDATE

FOR THE PROPOSED

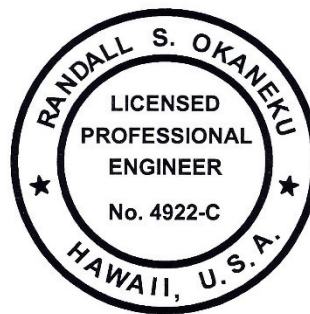
## HOKUA PLACE

KAPA`A, KAUAI, HAWAII  
TAX MAP KEY: (4)4-3-03: 01

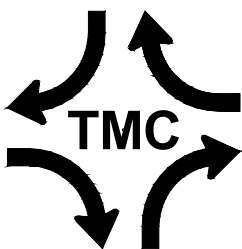
PREPARED FOR

HG KAUAI JOINT VENTURE, LLC

MAY 22, 2017



PREPARED BY



**THE TRAFFIC MANAGEMENT CONSULTANT**

RANDALL S. OKANEKU, P.E., PRINCIPAL \* 1188 BISHOP STREET, SUITE 1907 \* HONOLULU, HI 96813

## **Table of Contents**

	<b><u>Page</u></b>
Executive Summary.....	<i>iv</i>
I. Introduction.....	1
A. Project Description.....	1
B. 2015 Draft Environmental Impact Statement .....	1
C. Purpose and Scope of the Study.....	4
D. Methodologies.....	4
1. Capacity Analysis .....	4
2. Trip Generation.....	6
3. AASHTO Left-Turn Lane Guidelines .....	6
II. Existing Conditions.....	6
A. Roadways.....	6
B. Public Transit.....	8
C. Existing Peak Hour Traffic Volumes and Operating Conditions .....	8
1. Field Investigation and Data Collection .....	8
2. Existing AM Peak Hour Traffic.....	9
3. Existing PM Peak Hour Traffic .....	10
III. Future Traffic Conditions .....	13
A. Background Growth in Traffic.....	13
B. Daily and Seasonal Adjustment Factors .....	16
C. Kuhio Highway Widening.....	16
D. Kapa`a Transportation Solutions .....	16

## **Table of Contents (Cont'd.)**

	<b><u>Page</u></b>
E. Peak Hour Traffic Analysis Without Project .....	18
1. AM Peak Hour Traffic Without Project.....	18
2. PM Peak Hour Traffic Without Project .....	19
IV. Traffic Impact Analysis .....	24
A. Trip Generation Characteristics .....	24
B. Site Access Improvements.....	25
C. Traffic Assignment .....	25
D. AM Peak Hour Traffic Analysis With Project.....	25
E. PM Peak Hour Traffic Analysis With Project .....	30
V. Recommendations and Conclusions .....	35
A. Recommended Traffic Improvements Without Project.....	35
B. Recommended Traffic Improvements With Project.....	36
C. Conclusions.....	36

## **List of Figures**

	<b><u>Page</u></b>
Figure 1. Location and Vicinity Map.....	2
Figure 2. Hokua Place Site Plan .....	3
Figure 3. Existing AM Peak Hour Traffic .....	11
Figure 4. Existing AM Peak Hour Traffic (Cont'd.) .....	12
Figure 5. Existing PM Peak Hour Traffic.....	14
Figure 6. Existing PM Peak Hour Traffic (Cont'd.) .....	15
Figure 7. AM Peak Hour Traffic Without Project .....	20
Figure 8. AM Peak Hour Traffic Without Project (Cont'd.) .....	21
Figure 9. PM Peak Hour Traffic Without Project.....	22
Figure 10. PM Peak Hour Traffic Without Project (Cont'd.) .....	23
Figure 11. AM Peak Hour Site Traffic Assignment .....	26
Figure 12. AM Peak Hour Site Traffic Assignment (Cont'd.) .....	27
Figure 13. PM Peak Hour Site Traffic Assignment.....	28
Figure 14. PM Peak Hour Site Traffic Assignment (Cont'd.) .....	29
Figure 15. AM Peak Hour Traffic With Project .....	31
Figure 16. AM Peak Hour Traffic With Project (Cont'd.) .....	32
Figure 17. PM Peak Hour Traffic With Project.....	33
Figure 18. PM Peak Hour Traffic With Project (Cont'd.) .....	34

# **EXECUTIVE SUMMARY**

## **TRAFFIC IMPACT ANALYSIS REPORT UPDATE**

### **FOR THE PROPOSED**

# **HOKUA PLACE**

#### **Project Description**

The proposed Hokua Place will be developed into an 816-unit residential subdivision in Kapa`a, Kauai, Hawaii. The project is situated immediately to the south of Kapa`a Middle School and to the west (mauka) of Kapa`a Town. The primary access will be provided by a new connector roadway between Olohena Road, immediately mauka of Kapa`a Middle School, and the Kapa`a Bypass Road, southwest of its roundabout intersection with Olohena Road.

The Draft Environmental Impact Statement for the Proposed Hokua Place (DEIS) was published in May 2015. The Traffic Impact Assessment Report Kapa`a Highlands Subdivision, dated December 9, 2013, was attached to the DEIS. The purpose of this Traffic Impact Analysis Report Update is to update the DEIS traffic study, and to respond to comments received from the State of Hawaii Department of Transportation and the County of Kauai Department of Public Works, during their review of the DEIS traffic study.

#### **Existing Traffic Conditions**

The field investigation was conducted in March 2017, to update the existing traffic conditions from the DEIS traffic study. The study area was expanded to include Lehua Street and Ulu Street. The field investigation indicated that Lehua Street and Ulu Street were used as alternate routes between Kuhio Highway and Olohena Road/Kukui Street to avoid the delays at the intersection of Kuhio Highway and Kukui Street.

Since the preparation of the DEIS traffic study, the peak hour traffic at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road increased by about 12 percent and 22 percent, during the AM and PM peak hours of traffic, respectively.



### **Trip Generation**

Hokua Place is expected to generate 487 vehicle trips per hour (vph) and 560 vph, during the AM and PM peak hours of traffic, respectively. The AM and PM peak hour trip generation characteristics for Hokua Place were increased by about 90± vph over the DEIS traffic study, primarily due to the use of the average peak hour trip rates for the multi-family dwelling units.

### **Traffic Impact Analysis**

The construction of the connector roadway through Hokua Place, between Olohena Road and the Kapa`a Bypass Road, is expected to mitigate the project's traffic impacts at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road. The other intersections in the study area will require the following traffic improvements to mitigate the traffic impacts without and with the proposed project.

### **Recommendations Without Project**

1. Widen Kuhio Highway between the Kapa`a Bypass Road (South Junction) and Kuamoo Road to provide two through lanes in each direction.
2. Restripe the median on the north leg of Kuhio Highway at the Kapa`a Bypass Road (South Junction) to provide a median refuge lane.
3. Restripe parking and shoulder lanes on Kuhio Highway through Kapa`a Town to provide additional through and/or left-turn lanes.
4. Modify the traffic signal operations at the intersection of Kuhio Highway and Kukui Street to reduce queuing and delays.
5. Add a right-turn bypass lane from southbound Kapa`a Bypass Road to mauka bound Olohena Road at their roundabout intersection.
6. Realign Kaehulua Road to intersect Olohena Road and Kaapuni Road to create a four-legged, channelized intersection.
7. Extend the median refuge lane/two-way left-turn lane on the north leg of Kuhio Highway at Lehua Street.

### **Recommendations With Project**

1. Construct Road A from Olohena Road to the Kapa`a Bypass Road.
2. Construct a roundabout at the intersection of Road A and the Kapa`a Bypass Road.

## **Conclusions**

The existing traffic congestion on Kuhio Highway through Kapa`a Town can be mitigated by restricting on-street parking and restriping the shoulder lanes to provide for additional through lanes/median left-turn lanes. The existing southbound traffic demand through Kapa`a Town is reduced by the Kapa`a Bypass Road. Dedication of the Kapa`a Bypass Road right-of-way along the Hokua Place frontage would assure the continued usage of the existing Kapa`a Bypass Road.

The construction of the proposed Road A will provide additional mauka-makai roadway capacity between Kapa`a Valley and the Kapa`a Bypass Road. Road A is expected to mitigate the Hokua Place traffic impacts at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road. The Hokua Place access intersections on Olohena Road and on the Kapa`a Bypass Road are expected to operate at satisfactory Levels of Service, during the AM and PM peak hours of traffic.

# TRAFFIC IMPACT ANALYSIS REPORT UPDATE

## FOR THE PROPOSED

# HOKUA PLACE

KAPA`A, KAUAI, HAWAII

TAX MAP KEY: (4) 4-3-03: 01

## I. Introduction

### A. Project Description

Hokua Place is planned as an 816-unit residential development in Kapa`a, Kauai, Hawaii. Hokua Place will consist of 116 single-family detached units, 700 multi-family condominiums, a neighborhood retail center consisting of 8,000 square feet of gross floor area (SFGFA), and a community park and recreation center. The project site is located on the southwest quadrant of the roundabout intersection of the Kapa`a Bypass Road and Olohena Road. The project is situated immediately to the south of Kapa`a Middle School and to the west (mauka) of Kapa`a Town. Figure 1 depicts the project location and vicinity map.

Phase 1 of Hokua Place will consist of 16 single-family detached units, which will be located on the mauka portion of the project site. The Phase 1 access driveway is proposed on Olohena Road, mauka of its intersection with Kaapuni Road. Phase 2 will consist of the remaining 800 dwelling units. Phase 2 access is proposed via a collector street between Olohena Road, immediately mauka of Kapa`a Middle School, and the Kapa`a Bypass Road, about 3,000 feet southwest of its intersection with Olohena Road (hereinafter referred to as Road A). The project site is depicted on Figure 2.

The construction of Hokua Place is expected to begin by the Year 2020. For the purpose of this Traffic Impact Analysis Report Update, full occupancy is assumed to occur by the Year 2030.

### B. 2015 Draft Environmental Impact Statement

The Draft Environmental Impact Statement for the Proposed Hokua Place (DEIS) was published in May 2015. Hokua Place was formerly known as the Kapa`a Highlands Subdivision. The Traffic Impact Assessment Report Kapa`a Highlands Subdivision was prepared by Phillip Rowell and Associates, dated December 9, 2013, and was attached to the DEIS as Exhibit H.



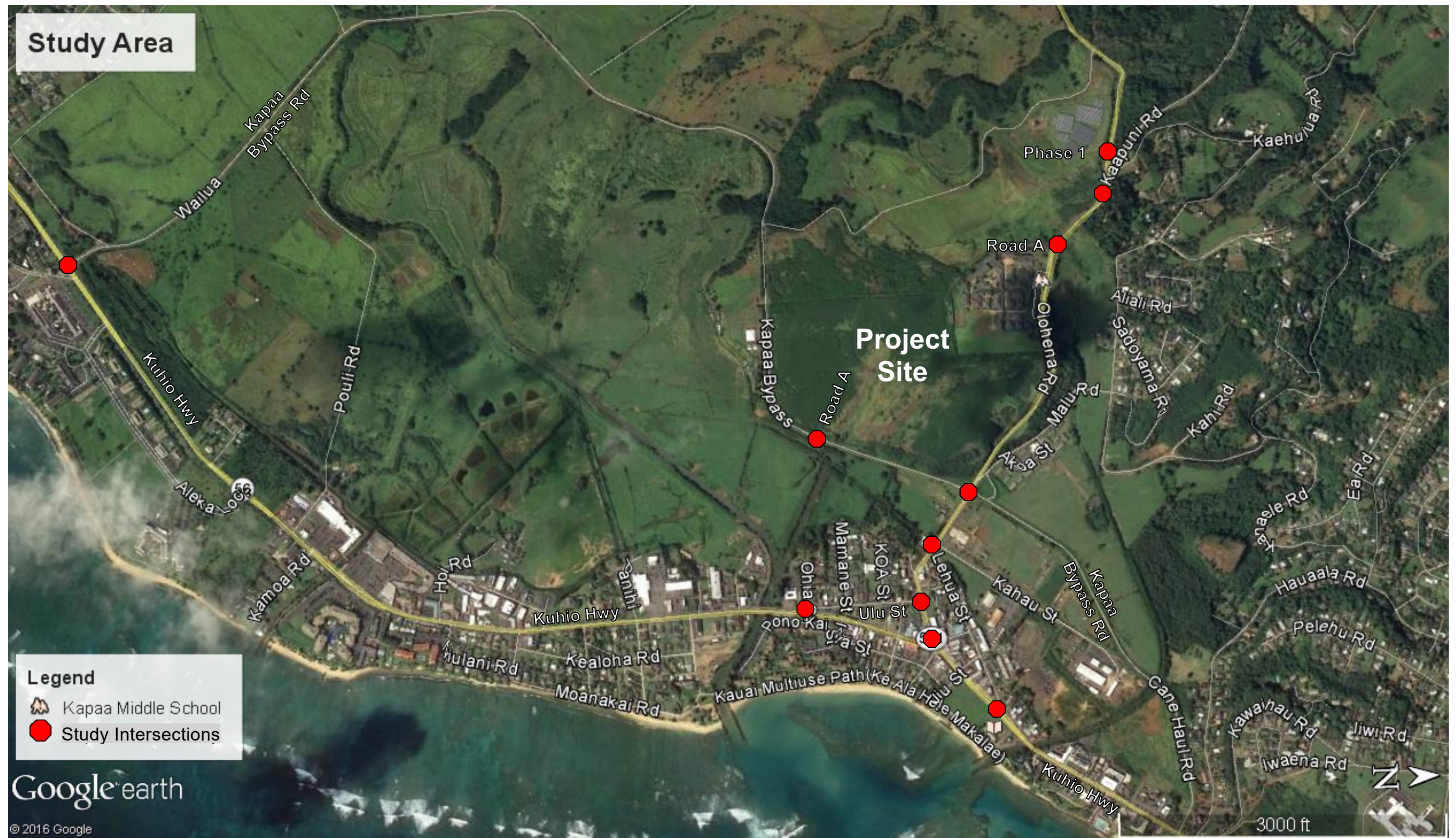


Figure 1. Location Map and Vicinity Map



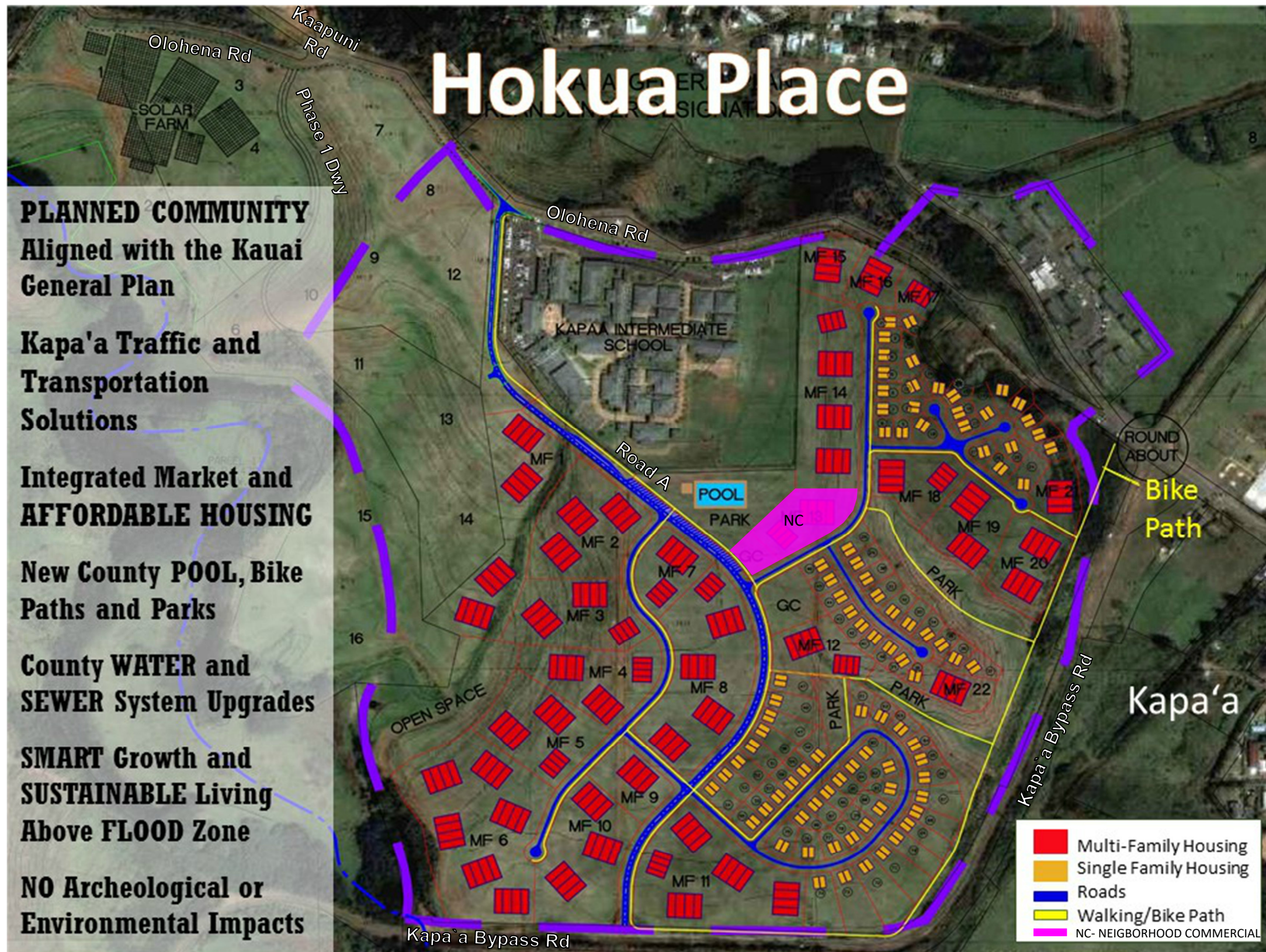
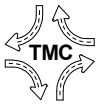


Figure 2. Proposed Site Plan





The State of Hawaii Department of Transportation (DOT) issued comments on the Rowell study in a letter dated March 26, 2014 (HWY-PS 2.6887). Responses to DOT's comments were transmitted via email from Mr. Greg Allen on April 9, 2014. The responses were acceptable to DOT per its letter, dated June 6, 2014 (HWY-PS 2.7311).

The County of Kauai Department of Public Works (DPW) issued its comments on the DEIS in its letter dated June 22, 2015. This TIAR addresses DPW's comments on the DEIS.

### **C. Purpose and Scope of the Study**

The purpose of this study is to update the traffic impact analysis resulting from the development of the proposed Hokua Place. This report presents the findings and recommendations of the study, the scope of which includes:

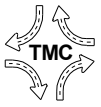
1. A description of the proposed project.
2. An evaluation of existing roadways and traffic conditions.
3. The analysis of the future traffic conditions without the proposed project.
4. The development of trip generation characteristics of the proposed project.
5. The identification and analysis of the traffic impacts resulting from the development of the proposed project.
6. The recommendation of roadway improvements, which would mitigate the traffic impacts identified in this study.

### **D. Methodologies**

#### **1. Capacity Analysis**

The highway capacity analysis, performed in this study, is based upon procedures presented in the Highway Capacity Manual 6<sup>th</sup> Edition (HCM), published by the Transportation Research Board. HCM defines the Level of Service (LOS) as “a quantitative stratification of a performance measure or measures representing quality of service.” HCM defines the six (6) Levels of Service from the traveler's perspective, ranging from the best LOS “A” to the worst LOS “F”. LOS translates the complex mathematical results of highway capacity analysis into an A through F system for the purpose of simplifying the roadway performance for non-technical decision makers.

The HCM 6<sup>th</sup> Edition has updated the highway capacity analysis since the HCM 2010 methodology, utilized in the DEIS traffic study. The most significant change in the HCM 6<sup>th</sup> Edition occurred in the analysis of roundabouts. The widespread construction of roundabouts throughout the United States, since the development of the HCM 2010, resulted in changes in driver behavior, entering and exiting a roundabout.



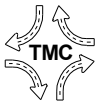
The data collected at United States roundabouts improved the HCM 6<sup>th</sup> Edition methodology for analyzing roundabouts, where the calculated delays were reduced by about one half, when compared with the previous HCM 2010 methodology.

LOS's "A", "B", and "C" are considered satisfactory Levels of Service. LOS "D" is generally considered a "desirable minimum" operating Level of Service. LOS's "E" and "F" are undesirable conditions. Intersection LOS is primarily based upon average delay (d) in seconds per vehicle (sec/veh). The delays at unsignalized intersections, which includes stop-controlled intersections and roundabouts, are generally longer than signalized intersections, due to the drivers' expectation and acceptance of longer delays at higher-volume signalized intersections. Table 1 summarizes the HCM LOS criteria.

Table 1. Intersection Level of Service Criteria (HCM)			
LOS	Signalized Control	Unsignalized Control	Description
	Delay d (sec/veh)		
A	d≤10	d≤10	Control delay is minimal.
B	10<d ≤20	10<d≤15	Control delay is not significant.
C	20<d≤35	15<d≤25	Stable operation. Queuing begins to occur.
D	35<d≤55	25<d≤35	Less stable condition. Increase in delays, decrease in travel speeds.
E	55<d≤80	35<d≤50	Unstable operation, significant delays.
F	d>80	d>50	High delays, extensive queuing.

HCM utilizes a peak hour factor (PHF) to convert the peak 15-minute traffic into an hourly volume. For the purpose of this study, the peak hour traffic analysis is based directly upon the peak 15-minute traffic flows entering the study intersection, which is multiplied by four (4) to convert the 15-minute peak volumes into the peak hour volumes.

Synchro is a traffic analysis software that was developed by Trafficware Corporation. Synchro is an intersection analysis program that is based upon the HCM 6<sup>th</sup> Edition methodology. Synchro was used to calculate the Levels of Service for the intersections in the study area. Worksheets for the capacity analysis, performed throughout this report, are compiled in the Appendix.



## 2. Trip Generation

The trip generation methodology is based upon generally accepted techniques developed by the Institute of Transportation Engineers (ITE) and published in Trip Generation Manual, 9<sup>th</sup> Edition, 2012. The ITE trip generation methodology has been updated since the Trip Generation, 7<sup>th</sup> Edition, utilized in the DEIS traffic study. The ITE trip rates were developed by correlating the total vehicle trip generation data with various land use activities/characteristics, such as the vehicle trips per hour (vph) per dwelling unit (DU).

A portion of the peak hour trips generated by a retail center is considered to be “pass-by” trips, i.e., traffic already on the roadway stopping by at a “secondary” destination enroute to its primary destination. The percentages of pass-by trips were compared with the gross leasable floor areas of the shopping centers, which were collected from traffic studies and compiled by ITE. The results of the analysis were published in the Trip Generation Handbook, 3rd Edition, dated August 2014. The percentage of pass-by trips is generally inversely proportional to the size of the shopping center, e.g., a regional shopping center is a primary destination with a low pass-by trip percentage, while a convenience store is a secondary destination with a high pass-by trip percentage. About 81.2 percent of the total PM peak hour trips generated by the proposed 8,000 square foot retail center are expected to be pass-by trips. The AM peak hour pass-by trip rate for a retail center was not published by ITE.

## 3. AASHTO Left-Turn Lane Guidelines

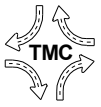
The left-turn lane assessment on a two-lane highway is based upon A Policy on Geometric Design of Highways and Streets, 2011, published by the American Association of State Highway and Transportation Officials (AASHTO). The AASHTO guide analyzes the combination of the left-turn volume (minimum 5%), the advancing volume (left-turn, through and right-turn volumes), the opposing volume (left-turn, through and right-turn volumes), and the operating speed. The AASHTO guide is based upon the “Volume Warrants for Left-Turn Storage Lanes at Unsignalized Grade Intersections”, Highway Research Record 211, Highway Research Board, 1967, by M. D. Harmelink. The Harmelink left-turn volume warrant analyzes the probability of the arrival of an advancing vehicle slowing and/or stopping behind a vehicle, which is waiting to turn left from the through lane.

## II. Existing Conditions

### A. Roadways

Kuhio Highway is the primary arterial highway along the east coast of Kauai. Through Kapa`a Town, Kuhio Highway is a two-lane roadway with on-street parking on both sides of the roadway. Kuhio Highway is signalized at its intersection with Kukui Street.





Exclusive left-turn lanes are provided on Kuhio Highway at major intersections in Kapa`a Town. The posted speed limit on Kuhio Highway in Kapa`a Town is 25 miles per hour (mph).

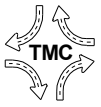
The Kapa`a Bypass Road provides an alternative southbound route around Kapa`a Town. The Kapa`a Bypass Road is a one-lane, one-way, southbound roadway between its north junction at Kuhio Highway and Olohena Road, with a posted speed limit of 25 mph. The Kapa`a Bypass Road intersects Olohena Road at a single-lane roundabout. South of Olohena Road, the Kapa`a Bypass Road becomes a two-way, two-lane roadway, with a posted speed limit of 35 mph. A 3,700± foot section of the Kapa`a Bypass Road, south of Olohena Road, was constructed on a roadway easement, which is currently owned by the developer of Hokua Place. Hokua Place, LLC has a Memorandum of Understanding with the State of Hawaii Department of Transportation (DOT) to dedicate the roadway easement to State DOT upon the approval of the Hokua Place subdivision.

South of the proposed intersection with Road A, the posted speed limit on the Kapa`a Bypass Road is reduced to 25 mph. At its south junction, the Kapa`a Bypass Road intersects Kuhio Highway at an unsignalized Tee-intersection. The Kapa`a Bypass Road provides separate left-turn and right-turn lanes at its south junction with Kuhio Highway. Exclusive left-turn and right-turn lanes are provided on Kuhio Highway at the Kapa`a Bypass Road in the northbound and southbound directions, respectively. A median refuge lane is not delineated on the north leg of Kuhio Highway at the Kapa`a Bypass Road. However, the striped median provide sufficient refuge space for one vehicle turning left from the Kapa`a Bypass Road.

South of the Kapa`a Bypass Road, the center northbound lane of Kuhio Highway is coned to provide a southbound contra-flow lane, during the AM peak period of weekday traffic, resulting in two lanes in the southbound direction and one lane in the northbound direction. During the field investigation, the contra-flow operation occurred from 5:45 AM to 10:30 AM. The contra-flow lane provides a “free” right-turn movement from the Kapa`a Bypass Road onto southbound Kuhio Highway, during the AM peak period of weekday traffic.

Olohena Road is a two-way, two-lane collector roadway with a posted speed limit of 25 mph. The posted speed limit on Olohena Road is reduced to 15 mph as it approaches Kapa`a Middle School. Olohena Road intersects the Kapa`a Bypass Road at a single-lane roundabout. Makai of Lehua Street, Olohena Road continues as Kukui Street to Kuhio Highway.

Kaapuni Road is a two-way, two-lane, collector road which intersects Olohena Road at a stop-controlled, skewed Tee-intersection. The Kaapuni Road approach has a limited sight distance to the right, due to the vertical alignment of the mauka leg of Olohena Road. Immediately mauka of Olohena Road, the two-way, two-lane Kaehulua Road intersects Kaapuni Road at a stop-controlled, skewed Tee-intersection.



Kukui Street is a two-way, two-lane roadway between Kuhio Highway and Ulu Street with a posted speed limit of 15 mph. Kukui Street is signalized at its intersection with Kuhio Highway with a shared left-turn lane and exclusive right-turn lane.

Ulu Street is a two-way, two-lane local street between Kukui Street and Ohia Street. South of Ohia Street, Ulu Street becomes a one-lane, one-way southbound roadway to Kuhio Highway. Ohia Street is a local street, which intersects Ulu Street and Kuhio Highway at stop-controlled intersections. Exclusive left-turn lanes are provided in both directions on Kuhio Highway at Ohia Street/Pono Kai Driveway. Ulu Street provides an alternate route to the south between Kuhio Highway and Kukui Street.

Lehua Street is a two-way, two-lane local street between Olohena Road and Kuhio Highway. Lehua Street intersects Olohena Road at a stop-controlled Tee-intersection. Lehua Street intersects Kuhio Highway at a stop-controlled, channelized Tee-intersection. Lehua Street provides an alternate route to the north between Kuhio Highway and Olohena Road.

Kahau Street is a two-way, two-lane cul-de-sac street. Kahau Street intersects Olohena Road at a stop-controlled Tee-intersection, immediately mauka of Lehua Street.

## **B. Public Transit**

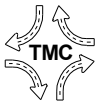
The Kauai County Transportation Agency operates a public bus service in the region with a stop on Olohena Road at the Kapa`a New Town Park, between the Kapa`a Bypass Road and Kahau Street. The Kauai bus service also stops at Kapa`a Middle School. On Kuhio Highway, the Kauai Bus service stops at Lehua Street, at Ohia Street, and at the Coconut Marketplace near the Kapa`a Bypass Road (South Junction). The Kauai Bus service is provided at hourly intervals Monday through Friday from 6 AM to 9 PM and on weekends and holidays every two hours from 8 AM to 5 PM.

## **C. Existing Peak Hour Traffic Volumes and Operating Conditions**

### **1. Field Investigation and Data Collection**

Turning movement traffic count surveys were conducted at the following intersections in the study area, during the week of March 13, 2017:

- a. Kapa`a Bypass Road and Olohena Road
- b. Olohena Road and Kaapuni Road
- c. Kaapuni Road and Kaehulua Road
- d. Kuhio Highway and Kukui Street
- e. Kuhio Highway and Kapa`a Bypass Road (South Junction)
- f. Kuhio Highway and Lehua Street



- g. Olohena Road and Lehua Street
- h. Olohena Road and Kahau Street
- i. Kukui Street and Ulu Street
- j. Ulu Street and Ohia Street
- k. Kuhio Highway and Ohia Street/Pono Kai Driveway
- l. Kuhio Highway and Ulu Street

Each intersection was surveyed during the peak periods of traffic over a two-day period. On March 14, 2017, a stalled vehicle partially blocked the circulatory roadway of the roundabout intersection of Olohena Road and the Kapa`a Bypass Road from 3:00 PM to 4:00 PM. The blockage limited traffic flows, and this data were excluded from the analysis. Otherwise, the higher peak hour volumes on the survey days at each study intersection were selected for the analysis to establish the existing conditions. The peak hours of traffic varied from intersection to intersection and from day to day.

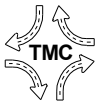
## 2. Existing AM Peak Hour Traffic

The existing AM peak hour of traffic in the study area generally occurred from 7:15 AM to 8:15 AM. Table 2 summarizes the changes in the AM peak hour traffic between the DEIS traffic study and the existing AM peak hour traffic data.

Table 2. AM Peak Hour Traffic Comparison			
Study Intersection	Intersection Volumes (vph)		Increase (+) Decrease (-)
	2012-2013	2017	
Olohena Road/Kapa`a Bypass Road	1,447	1,628	+181
Kuhio Highway/Kukui Street	1,441	1,410	-31
Kuhio Hwy/Kapa`a Bypass Road	1,990	2,111	+121

In Kapa`a Town, Kuhio Highway carried about 1,400 vehicles per hour (vph), total for both directions, during the AM peak hour of traffic. South of Ulu Street, Kuhio Highway carried over 1,750 vph, total for both directions. The Kapa`a Bypass Road carried about 800 vph, total for both directions, south of Olohena Road. Mauka of the Kapa`a Bypass Road, Olohena Road carried about 1,000 vph, total for both directions. South of the Kapa`a Bypass Road (South Junction), Kuhio Highway carried about 2,100 vph.

The traffic signal timing cycle lengths at the intersection of Kuhio Highway and Kukui Street resulted in long delays on Kukui Street. Makai bound traffic on Olohena Road and Kukui Street were diverted to alternate routes to Kuhio Highway. About 54 percent of makai bound traffic on Olohena Road turned left onto Lehua Street to continue in the northbound direction. About 33 percent of makai bound traffic turned right onto Ulu Street to continue in the southbound direction. The remaining 13 percent



of the makai bound traffic on Olohena Road continued onto Kukui Street to Kuhio Highway.

During the existing AM peak hour of traffic, the overall intersection of Kuhio Highway and Kukui Street operated at LOS “A”. However, the left-turn movement on makai bound Kukui Street operated at LOS “F”, with a relatively low traffic demand (32 vph). All the traffic movements in both directions on Kuhio Highway operated at LOS “A” at Kukui Street, during the existing AM peak hour of traffic.

The left-turn movement on makai bound Lehua Street operated at LOS “E” at Kuhio Highway, during the existing AM peak hour of traffic. Makai bound Ohia Street also operated at LOS “E” at Kuhio Highway at a very low volume.

Makai bound Olohena Road operated at LOS “D” at the Kapa`a Bypass Road. Kaapuni Road operated at LOS “D” at Olohena Road. The other intersections in the study area operated at satisfactory Levels of Service, i.e., LOS “C” or better, during the existing AM peak hour of traffic. Figures 3 and 4 depict the existing AM peak hour traffic data.

### 3. Existing PM Peak Hour Traffic

The existing PM peak hour of traffic in the study area varied between the hours of 3:00 PM and 6:00 PM. Table 3 summarizes the changes in the PM peak hour traffic between the DEIS traffic study and the existing (2017) PM peak hour traffic data.

<b>Table 3. PM Peak Hour Traffic Comparison</b>			
<b>Study Intersection</b>	<b>Intersection Volumes (vph)</b>		<b>Increase (+) Decrease (-)</b>
	<b>2012-2013</b>	<b>2017</b>	
Olohena Rd/Kapa`a Bypass Rd	1,459	1,787	+328
Kuhio Hwy/Kukui St	1,370	1,295	-75
Kuhio Hwy/Kapa`a Bypass Rd	2,176	2,235	+62

During the existing PM peak hour of traffic, Kuhio Highway carried about 1,200 vph, total for both directions in Kapa`a Town. South of Ulu Street, Kuhio Highway carried over 1,500 vph, total for both directions. The Kapa`a Bypass Road carried over 1,000 vph, total for both directions, south of Olohena Road. Mauka of the Kapa`a Bypass Road, Olohena Road carried about 1,000 vph, total for both directions. Kuhio Highway carried over 2,100 vph, total for both directions, south of the Kapa`a Bypass Road.

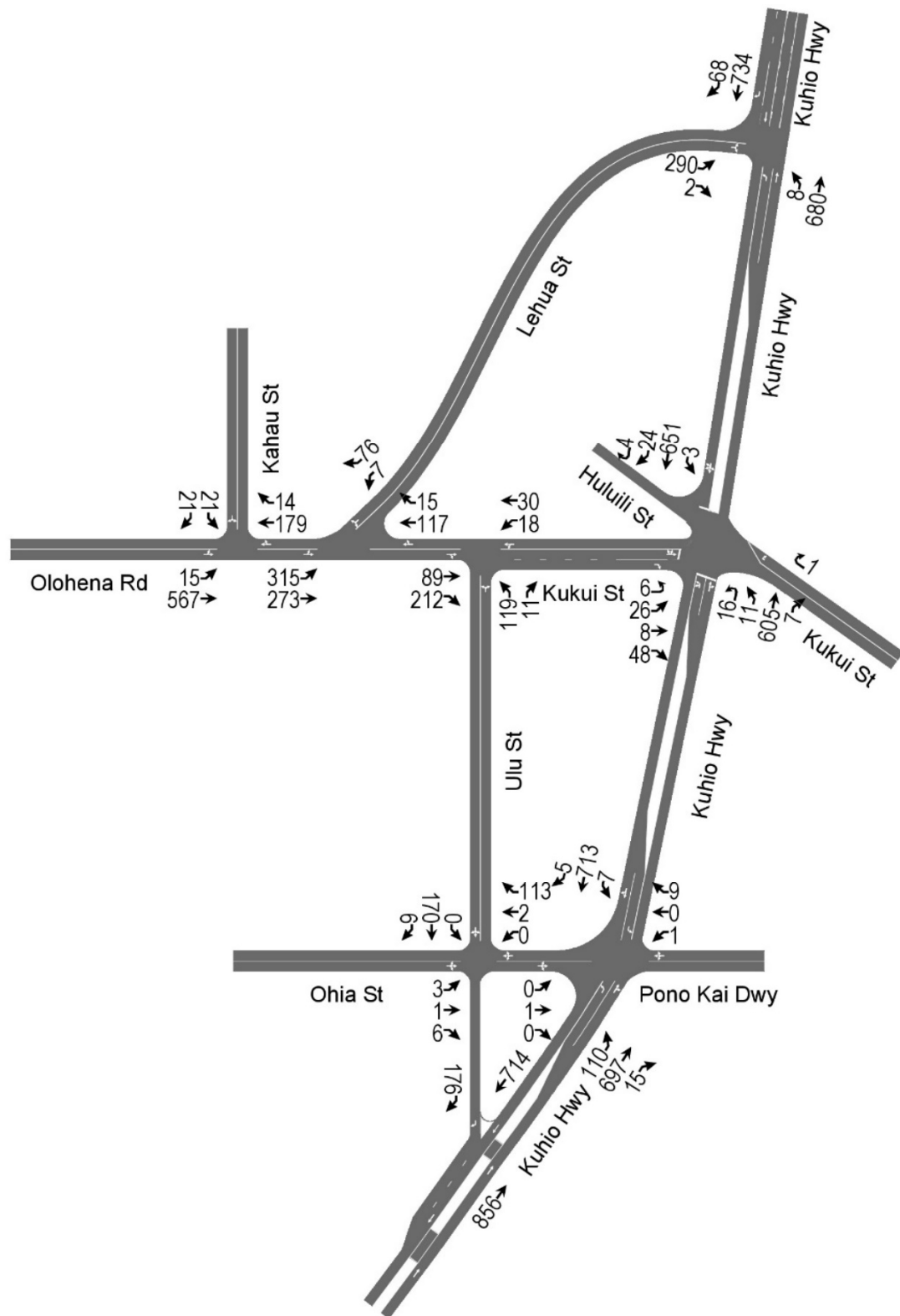
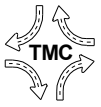


Figure 3. Existing AM Peak Hour Traffic

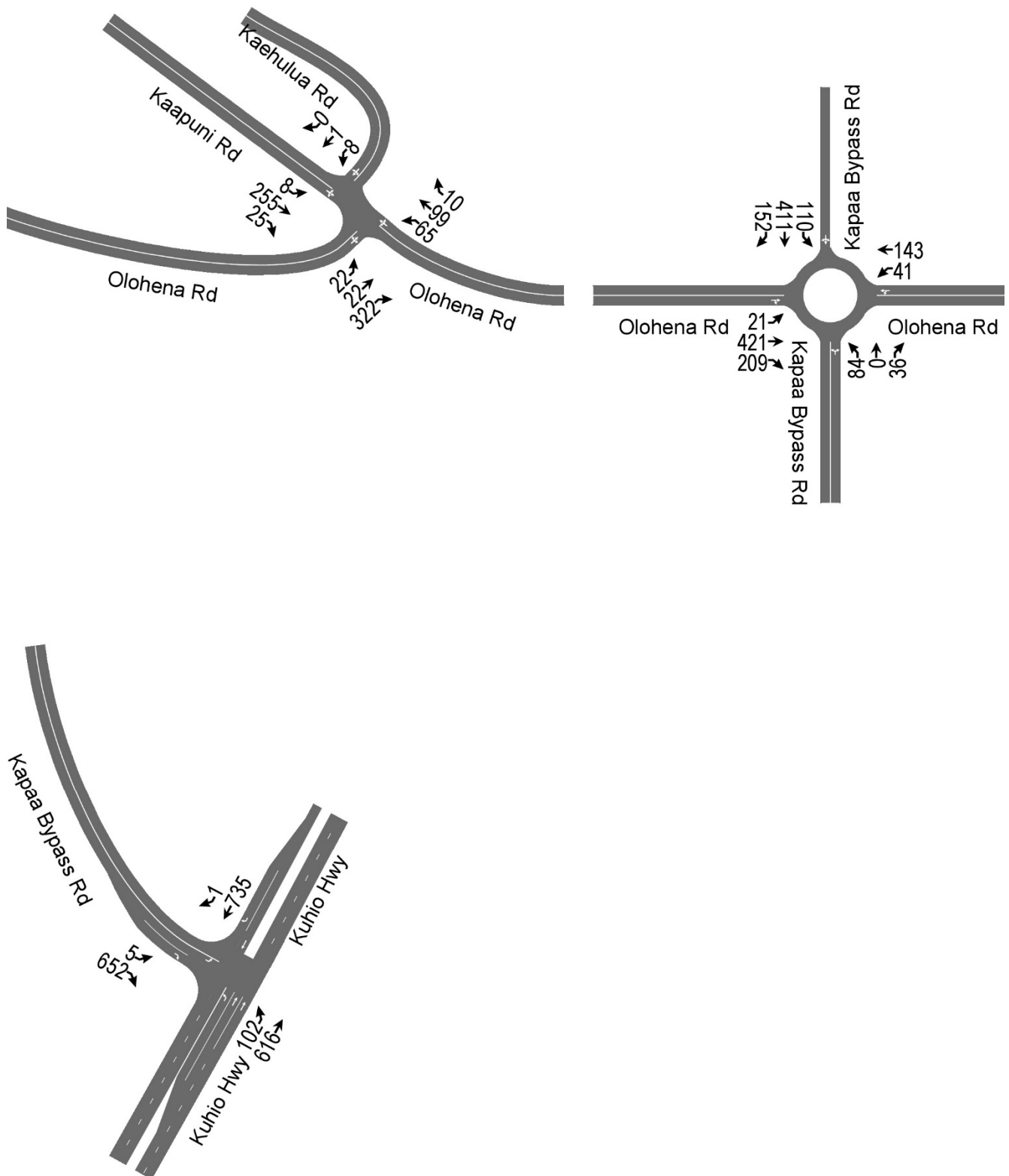
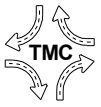
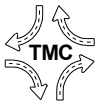


Figure 4. Existing AM Peak Hour Traffic (Cont'd.)



The northbound and southbound traffic on Kuhio Highway avoided the traffic signal delays at Kukui Street by diverting to alternate routes to Olohena Road. Less than 10 percent of the mauka bound traffic on Olohena Road at the Kapa`a Bypass Road turned from Kuhio Highway via Kukui Street. About 35 percent of the mauka bound traffic on Olohena Road turned right from Lehua Street to continue in the mauka bound direction, during the existing PM peak hour of traffic. About 55 percent of the mauka bound traffic turned left from Ulu Street onto Kukui Street to continue in the mauka bound direction on Olohena Road.

The overall intersection of Kuhio Highway and Kukui Street operated at LOS “A”, during the existing PM peak hour of traffic. The left-turn movement on makai bound Kukui Street operated at LOS “E” with a relatively low traffic demand (36 vph). The other traffic movements at the intersection operated at LOS “A”, during the existing PM peak hour of traffic.

The left-turn movement on makai bound Lehua Street operated at LOS “D” at Kuhio Highway, during the existing PM peak hour of traffic. Makai bound Ohia Street operated at LOS “F” at Kuhio Highway with a very low volume. The mauka bound Pono Kai Driveway operated at LOS “D”, also with a very low volume.

Southbound Lehua Street operated at LOS “E” at Olohena Road, during the existing PM peak hour of traffic. Southbound Kapa`a Bypass Road operated at LOS “D” at Olohena Road. The other intersections in the study area operated at satisfactory Levels of Service, during the existing PM peak hour of traffic. The existing PM peak hour traffic data are depicted on Figures 5 and 6.

### **III. Future Traffic Conditions**

#### **A. Background Growth in Traffic**

The Kauai Long-Range Land Transportation Plan (KLRLTP) was prepared by the State of Hawaii Department of Transportation (DOT), in cooperation with the Kauai County Department of Public Works and Planning Department. The KLRLTP developed long-range travel forecasts for the island of Kauai. The KLRLTP anticipated that traffic in the Kapa`a area would increase by over 30 percent between the Base Year 2007 and the Horizon Year 2035. For the purpose of this analysis, an average growth factor of 1.14 was uniformly applied to the existing (Year 2017) AM and PM peak hour traffic volumes to estimate the Year 2030 peak hour traffic without the proposed project.

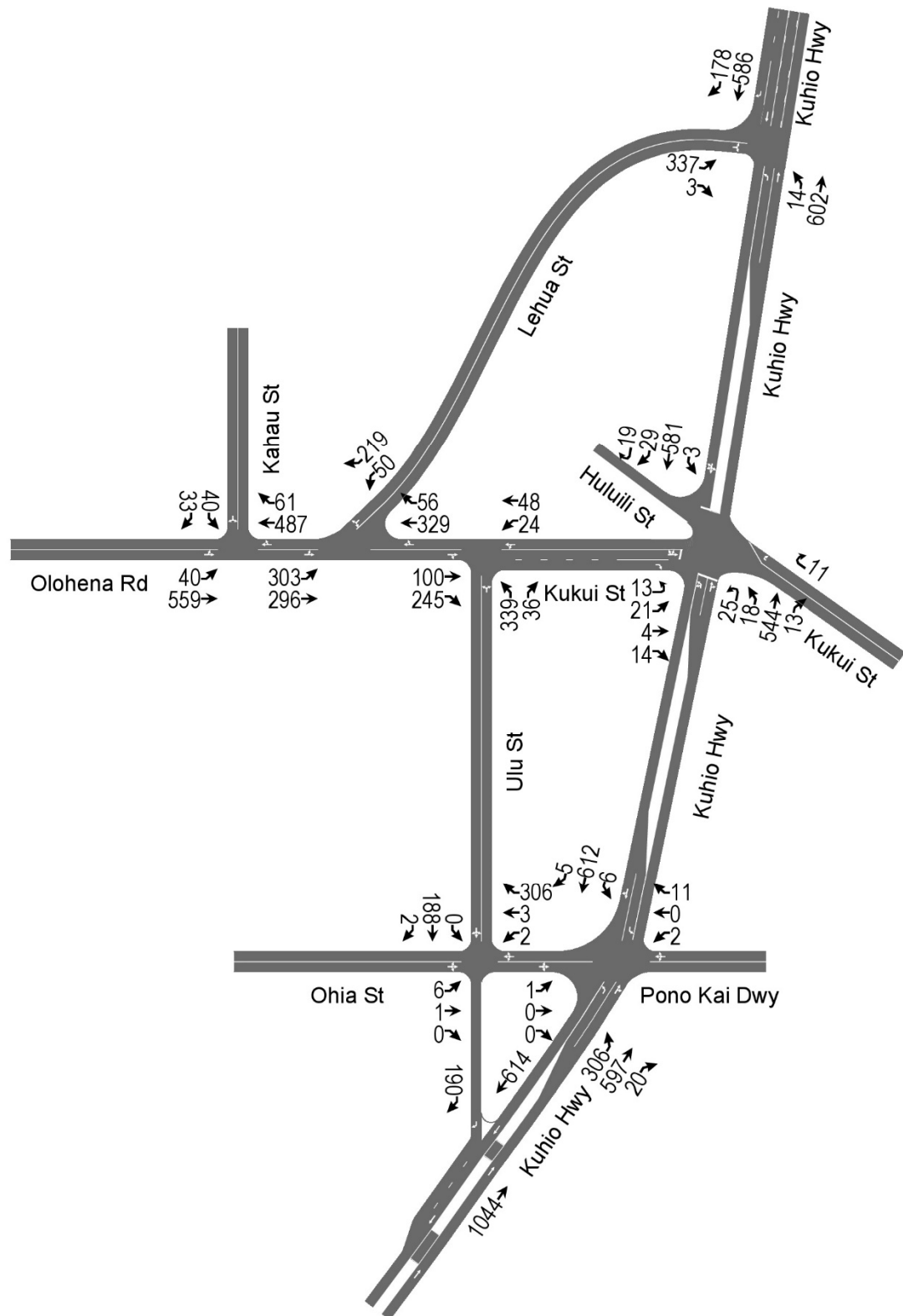
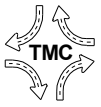


Figure 5. Existing PM Peak Hour Traffic



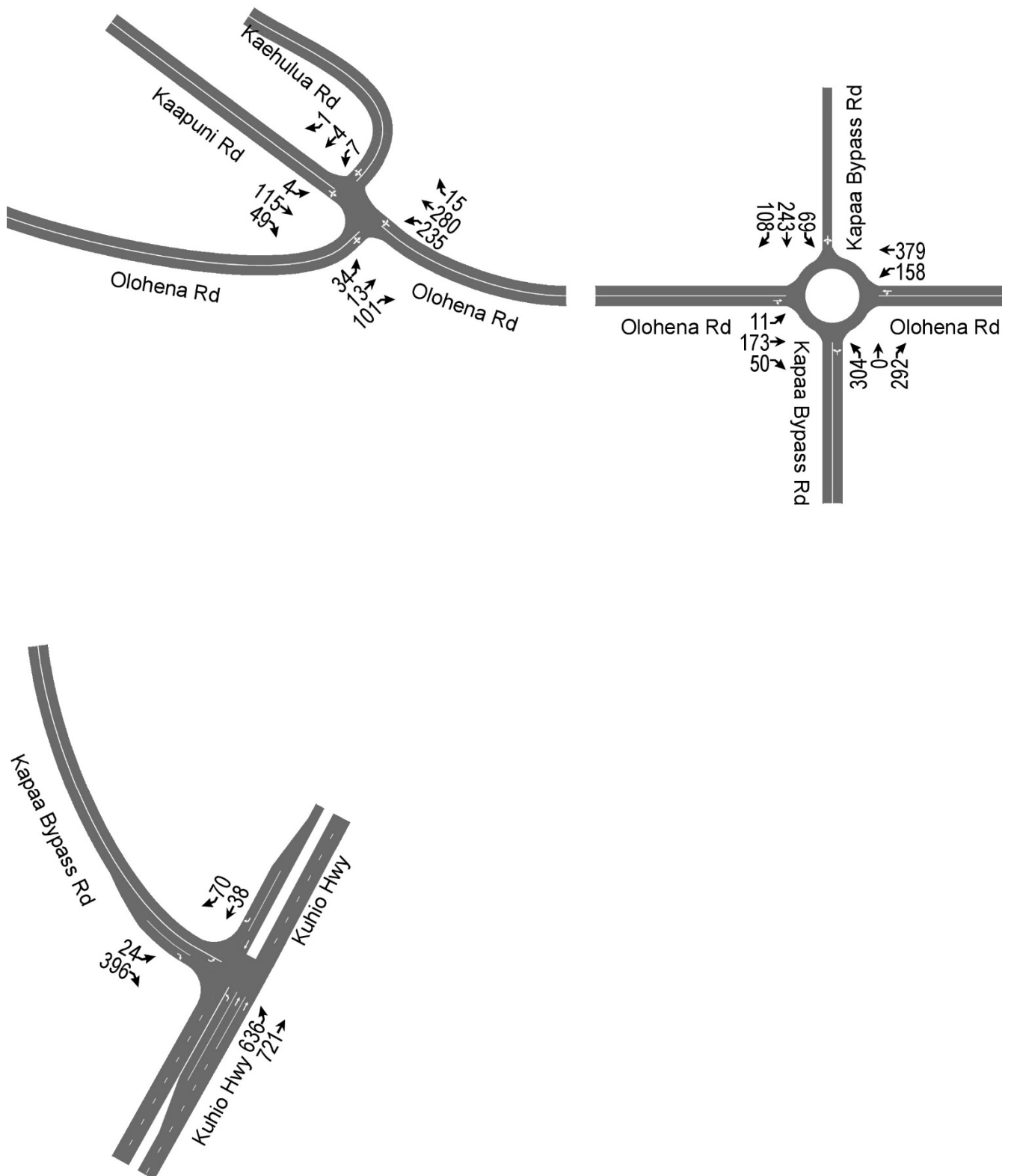
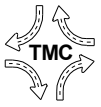
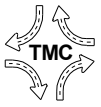


Figure 6. Existing PM Peak Hour Traffic (Cont'd.)



## B. Daily and Seasonal Adjustment Factors

The existing peak hour traffic data were adjusted for the daily and seasonal variation in traffic in the region. The adjustment factors were based upon the 2016 traffic count data, which were collected at DOT's continuous traffic count station at Mile Post 2.4 on Kuhio Highway (Route 56) in Hanamaulu, which is located about 6 miles south of Kapa'a Town. Table 4 summarizes the adjustment factors, which were applied to the existing AM and PM peak hour traffic data, to account for the daily and seasonal variation in traffic from the annual average weekday traffic (AAWDT).

Table 4. Day of the Week and Seasonal Adjustment Factors			
Date	Day	24-Hour Data	Adjustment Factors
3/14/2016	Monday	15,881	1.03
3/15/2016	Tuesday	15,824	1.03
3/16/2016	Wednesday	16,611	0.98
3/17/2016	Thursday	16,467	0.99
3/18/2016	Friday	16,652	0.98
2016 AAWDT		16,301	1.00

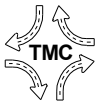
## C. Kuhio Highway Widening

The Final Environmental Assessment Kuhio Highway Short-Term Improvements Kuamoo Road to Temporary Bypass Road (Kuhio Highway EA), was prepared for DOT, by Wilson Okamoto Corporation, dated September 2009. The Kuhio Highway EA recommended the widening of Kuhio Highway from three lanes to four lanes to provide a permanent second southbound lane between the Kapa'a Bypass Road and Kuamoo Road. The additional lane will provide a "free" right-turn movement from the Kapa'a Bypass Road onto southbound Kuhio Highway throughout the day.

DOT is planning to complete the widening of Kuhio Highway by the Year 2019. The widening of Kuhio Highway from the Kapa'a Bypass Road to Kuamoo Road is included in this traffic impact analysis.

## D. Kapa'a Transportation Solutions

The Kapa'a Transportation Solutions (KTS) was prepared for the State Department of Transportation, dated August 2015. The KTS was prepared for DOT in cooperation with the Kauai County Department of Public Works, Planning Department, and Transportation Agency, and the Federal Highways Administration. The KTS included input from the Kapa'a Citizens Advisory Committee, which is comprised of the Kapa'a Business Association, Kapa'a High School and Middle School, Wailua-Kapa'a Neighborhood Association, Kauai Visitors and Convention Bureau, and Kauai Path.



The KTS cited traffic congestion in the downtown/historic district of Kapa`a Town, which resulted from on-street parking in the curb lanes in both directions on Kuhio Highway. In addition, to the delays caused by vehicles maneuvering into and out of the parallel parking stalls along Kuhio Highway, the on-street parking occupies valuable highway space, which could otherwise provide additional through traffic lanes and/or median left-turn lanes. Table 5 summarizes the roadway improvements relevant to this traffic study, which were prioritized in the Kapa`a Transportation Solutions.

<b>Table 5. Potential Traffic Solutions</b>		
<b>Location</b>	<b>Description</b>	<b>Priority</b>
Kapa`a Bypass Road	Widen the Kapa`a Bypass Road to provide one lane in the northbound direction from Olohena Road to Kuhio Highway.	<5 Years
Kuhio Highway and Kukui Street	Modify traffic signal timings.	<5 Years
Kuhio Hwy and Kapa`a Bypass Road	Intersection improvements.	<5 Years
Olohena Road at Kapa`a Middle School	Improve crosswalk.	<5 Years
Kapa`a Bypass Road and Olohena Road Roundabout	Add a separate (bypass) right-turn lane at the roundabout from makai bound Olohena Road to southbound Kapa`a Bypass Road.	<5 Years
Kuhio Highway	Provide an additional southbound lane on Kuhio Highway from Kapa`a Bypass Road to Kuamoo Road (scheduled for construction).	<5 Years
Kuhio Highway and Kukui Street	Close the makai leg of Kukui Street to provide business parking. Implement vehicular and pedestrian improvements on Kukui Street (mauka leg) and Huluhulu Street at Kuhio Highway.	5-10 Years
Kapa`a New Town Park	Provide direct access from the Kapa`a New Town Park to the Kapa`a Bypass Road.	5-10 Years
Kuhio Highway and Lehua Street	Improve the left-turn movement from Lehua Street onto Kuhio Highway.	5-10 Years
Kapa`a Bypass Rd and Kuhio Highway	Re-align the Kapa`a Bypass Road (South Junction) to intersect Kuhio Highway opposite Aleka Loop or Papaloa Road.	5-10 Years

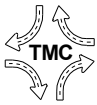


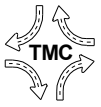
Table 5. Potential Traffic Solutions (Cont'd.)		
Location	Description	Priority
Kapa`a Bypass Road South of Olohena Road	Improve the horizontal alignment and shoulders of the Kapa`a Bypass Road, south of Olohena Road, to Kuhio Highway.	5-10 Years
Kuhio Highway Between Kawaihau Road and Lehua Street	Provide a two-way median left-turn lane along Kuhio Highway.	5-10 Years
Olohena Rd at Kahau St and Lehua St	Implement intersection improvements and bicycle/pedestrian improvements to Kuhio Highway.	5-10 Years
Olohena Rd at Kaapuni Rd and Kaehulua Rd	Implement intersection improvements	5-10 Years
Kaapuni Road	Upgrade/improve Kaapuni Road to major collector standards, including bicycle lanes.	5-10 Years
Olohena Road Between Kuhio Highway and Kamalu Road	Improve Olohena Road to accommodate non-motorized modes.	5-10 Years
Kapa`a Bus Hub	Relocate the Kapa`a bus hub from its existing location near the skate park to a new location on or near the Kuhio Highway mainline, with amenities.	5-10 Years

Improving the horizontal alignment and providing shoulders on the Kapa`a Bypass Road, south of Olohena Road may impact the proposed Hokua Place frontage. Any widening and realignment should be coordinated with Hokua Place. The Kapa`a Transportation Solutions also identifies Road A as a new connector road between Olohena Road and the Kapa`a Bypass Road, which was prioritized beyond the 10-year time frame. The construction cost of the connector road was estimated at \$25,824,000.

## E. Peak Hour Traffic Analysis Without Project

### 1. AM Peak Hour Traffic Without Project

During the AM peak hour of traffic without the proposed project, the overall intersection of Kuhio Highway and Kukui Street is expected to continue to operate at LOS "A". The left-turn movement on makai bound Kukui Street is expected to continue to operate at LOS "F". The traffic movements in both directions on Kuhio Highway are expected to continue to operate at LOS "A" at Kukui Street, during the AM peak hour of traffic without the proposed project.



Makai bound Lehua Street is expected to operate at LOS “F” at Kuhio Highway, during the AM peak hour of traffic without the proposed project. Makai bound Ohia Street is expected to operate at LOS “E” at Kuhio Highway.

During the AM peak hour of traffic without the proposed project, makai bound Olohena Road is expected to operate at LOS “F” at the Kapa`a Bypass Road. Southbound Kapa`a Bypass Road is expected to operate at LOS “D” at Olohena Road. Kaapuni Road is expected to operate at LOS “F” at Olohena Road. The other intersections in the study area are expected to operate at satisfactory Levels of Service, during the AM peak hour of traffic without the proposed project. Figures 7 and 8 depict the AM peak hour volumes without the proposed project.

## **2. PM Peak Hour Traffic Without Project**

The overall intersection of Kuhio Highway and Kukui Street is expected to operate at LOS “A”, during the PM peak hour of traffic without the proposed project. The left-turn movement on makai bound Kukui Street is expected to continue to operate at LOS “E”. The other traffic movements at the intersection are expected to operate at LOS “A”, during the PM peak hour of traffic without the proposed project.

The left-turn movement on makai bound Lehua Street is expected to operate at LOS “E” at Kuhio Highway, during the PM peak hour of traffic without the proposed project. Makai bound Ohia Street also is expected to operate at LOS “F” at Kuhio Highway. Mauka bound Pono Kai Driveway is expected to operate at LOS “E”, during the PM peak hour of traffic without the proposed project.

Southbound Lehua Street is expected to continue to operate at LOS “F” at Olohena Road, during the PM peak hour of traffic without the proposed project. Southbound Kapa`a Bypass Road is expected to operate at LOS “F” at Olohena Road. The right-turn movement from the Kapa`a Bypass Road onto Kuhio Highway is expected to operate LOS “D”. The other intersections in the study area are expected to operate at satisfactory Levels of Service, during the PM peak hour of traffic without the proposed project.

The PM peak hour traffic demands at the intersection of Olohena Road and Kaapuni Road without the proposed project are expected to meet the AASHTO guideline for an exclusive left-turn lane on makai bound Olohena Road. The PM peak hour volumes without the proposed project is depicted on Figures 9 and 10.

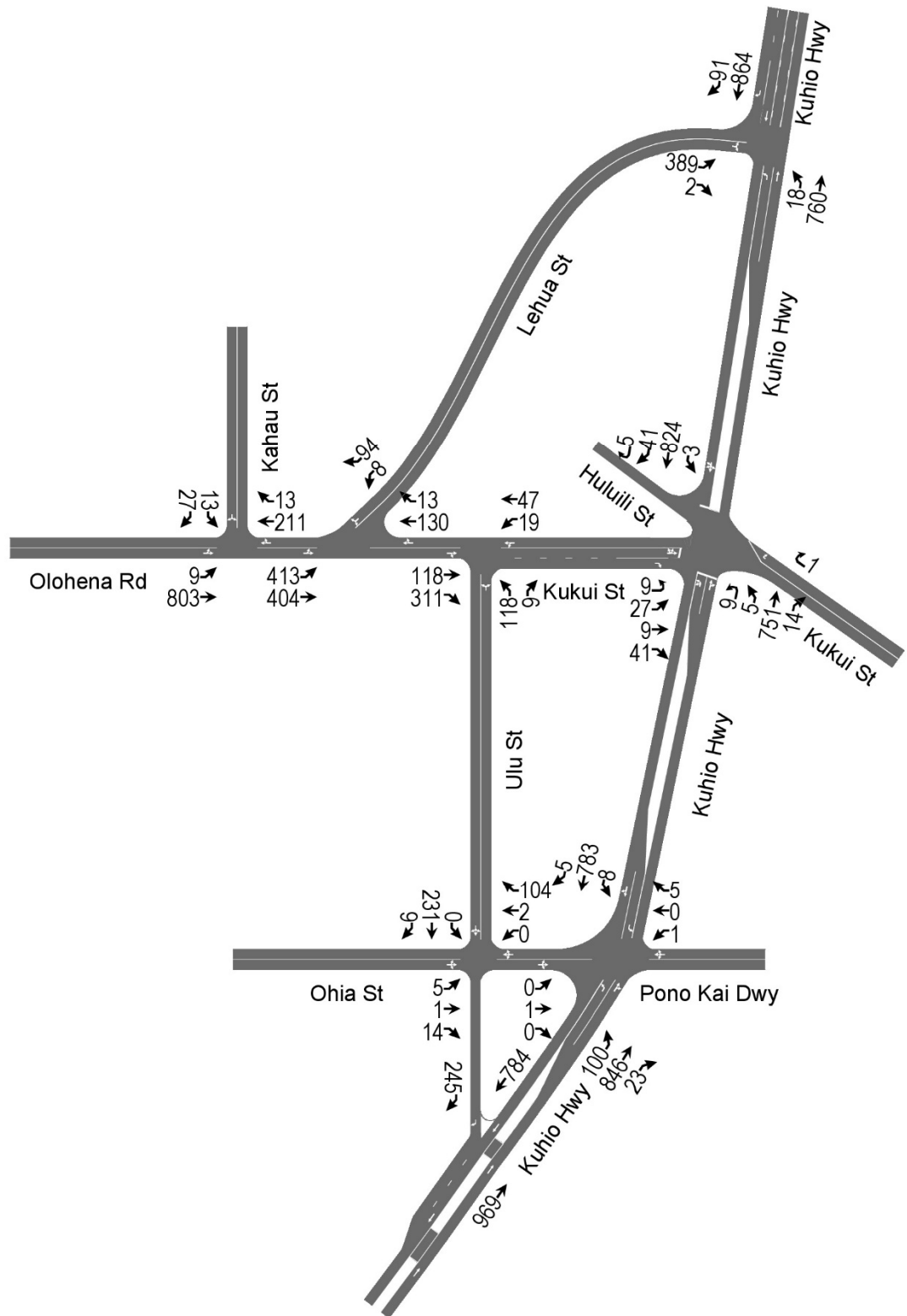
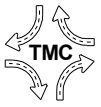


Figure 7. AM Peak Hour Volumes Without Project

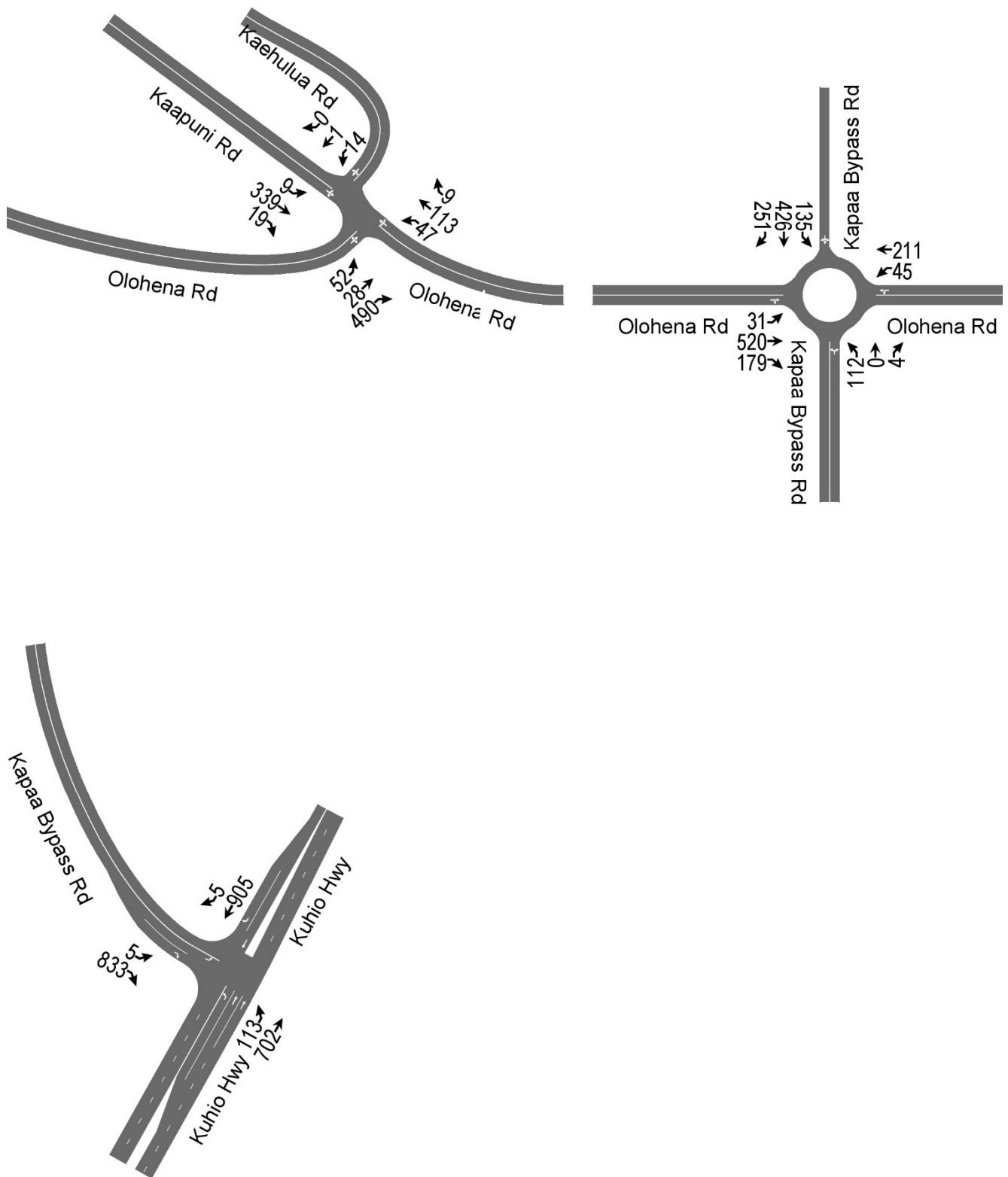
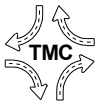


Figure 8. AM Peak Hour Volumes Without Project (Cont'd.)

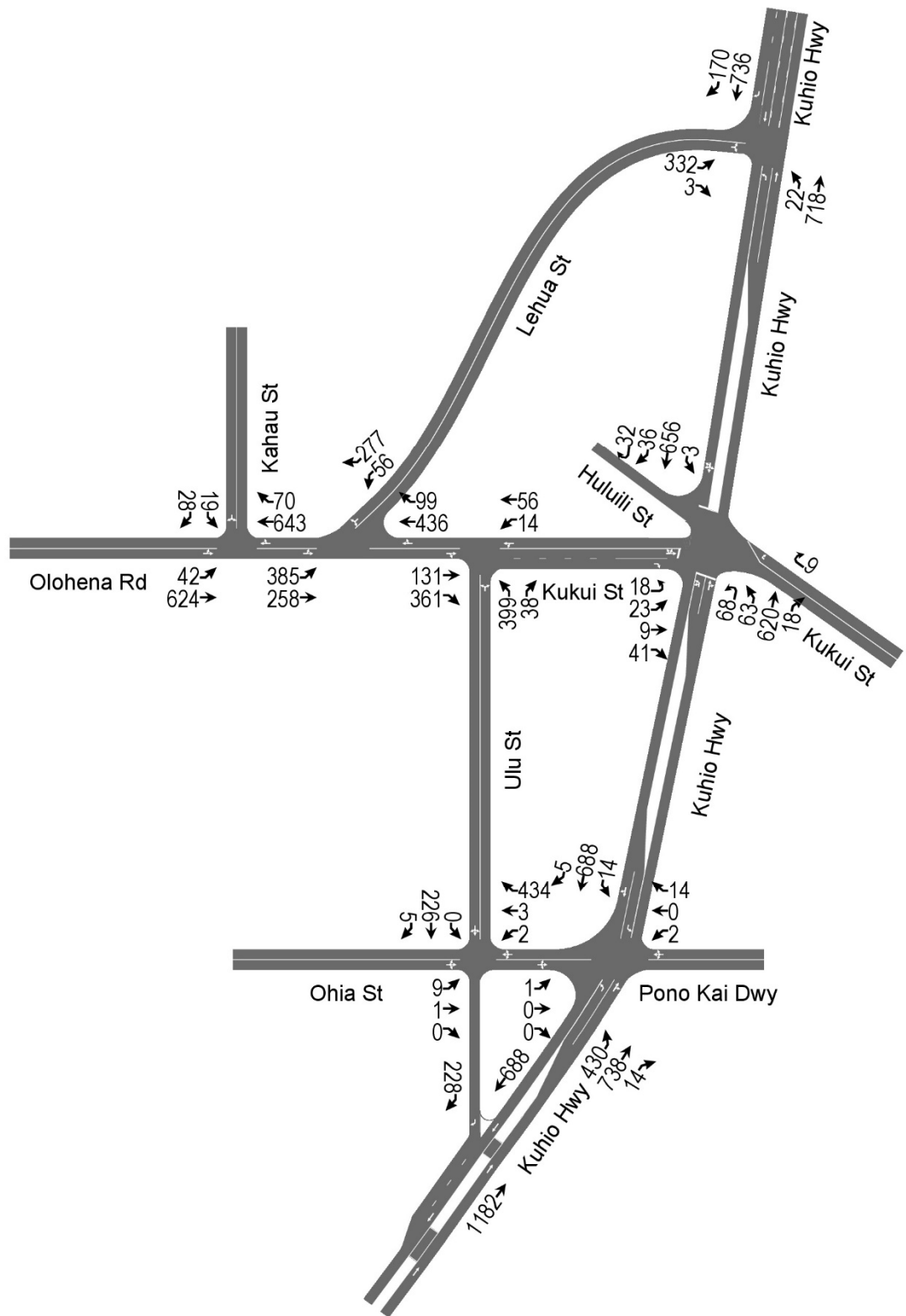
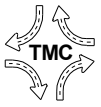


Figure 9. PM Peak Hour Volumes Without Project



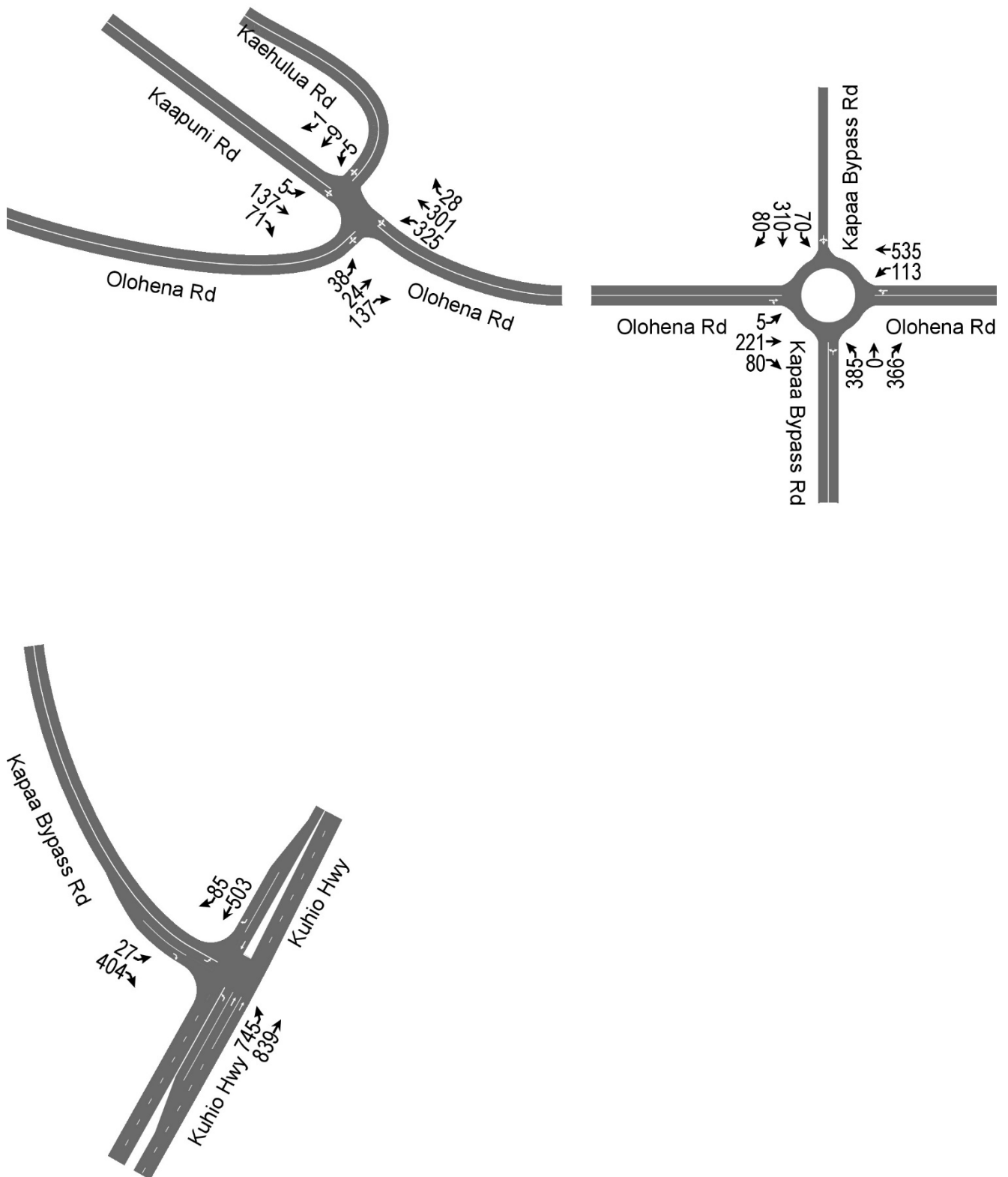
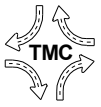
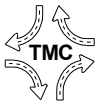


Figure 10. PM Peak Hour Volumes Without Project (Cont'd.)



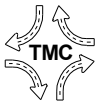
#### IV. Traffic Impact Analysis

##### A. Trip Generation Characteristics

The trip generation characteristics were based upon the ITE trip rates for single-family detached dwelling units (DU) and residential condominium/townhouse units. The weekday ITE trip rates, during the AM peak hour and the PM peak hour of adjacent street traffic, were used for this traffic impact analysis. The ITE regression equations were used to derive the trip rates for the single-family detached dwellings in this analysis. Although ITE recommends the use of the regression equations to derive trip rates, the average peak hour trips rates for the residential condominium/townhouse were used in this analysis. The 800 DU is outside the range of the ITE trip generation data that were utilized to develop the regression equations for condominiums. Furthermore, the average condominium/townhouse rates are higher (more conservative) than the rates that are derived by the regression equations.

The ITE trip generation rates for a shopping center were developed from the regression equations to estimate the trip generation from the proposed 8,000 SFGFA retail center. The pass-by trip rate of 81.2 percent was applied to the PM peak hour trip generation. The ITE pass-by trip rate is reasonable given the size of Hokua Place and the volume of through traffic on Road A. Hokua Place is expected to generate totals of 487 vph and 560 vph, during the AM and PM peak hours of traffic, respectively. The trip generation characteristics for the proposed project are summarized in Table 6.

Table 6. Hokua Place Trip Generation Characteristics							
Land Use (ITE Code)	Units	AM Peak Hour (vph)			PM Peak Hour (vph)		
		Enter	Exit	Total	Enter	Exit	Total
Single-Family Phase 1 (265)	16 DU	5	16	21	13	7	20
Single-Family Phase 2 (265)	100 DU	20	60	80	66	38	104
Condominium/ Townhouse (230)	800 DU	60	292	352	279	137	416
Retail Center (820)	8,000 SFGFA	21	13	34	53	57	110
	Pass-By	0	0	0	(-)45	(-)45	(-)90
Total External Trips		106	381	487	366	194	560



## **B. Site Access Improvements**

A conventional channelized, Tee-intersection was considered at the intersection of Road A and the Kapa`a Bypass, with left-turn and right-turn deceleration/storage lanes and a median refuge lane on the Kapa`a Bypass Road. Under unsignalized traffic control, the left-turn lane from Road A onto the Kapa`a Bypass Road is expected to operate at LOS “F”, during the PM peak hour of traffic. As an alternative to traffic signalization, a roundabout intersection is recommended Road A and the Kapa`a Bypass Road. The following site access improvements are recommended for the proposed project:

1. Construct a stop-controlled Tee-intersection between Road A and Olohena Road.
2. Construct a stop-controlled Tee-intersection between the Phase 1 Driveway and Olohena Road.
3. Construct a single-lane roundabout at the intersection of Road A and the Kapa`a Bypass Road.

## **C. Traffic Assignment**

The traffic assignments were based upon the existing traffic patterns along Olohena Road and Kukui Street. The traffic assignments also included through traffic demands, which are expected to be diverted from makai bound Olohena Road and from northbound Kapa`a Bypass Road to the proposed Road A. Road A is expected to reduce the traffic demands at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road. Figures 11 and 12 depict the AM peak hour traffic assignments. The PM peak hour traffic assignments are depicted on Figures 13 and 14.

## **D. AM Peak Hour Traffic Analysis With Project**

The roundabout intersection of the Kapa`a Bypass Road and Road A is expected to operate at satisfactory Levels of Service, during the AM peak hour of traffic with the proposed project. Road A is expected to operate at LOS “C” at Olohena Road. The Phase 1 driveway on Olohena Road is expected to operate at LOS “B”.

The overall intersection of Kuhio Highway and Kukui Street is expected to continue to operate at LOS “A”, during the AM peak hour of traffic with the proposed project. The left-turn movement on makai bound Kukui Street is expected to continue to operate at LOS “F”. The traffic movements in both directions on Kuhio Highway are expected to operate at LOS “A” at Kukui Street, during the AM peak hour of traffic with the proposed project.

Makai bound Lehua Street is expected to continue to operate at LOS “F” at Kuhio Highway, during the AM peak hour of traffic with the proposed project. Makai bound Ohia Street also is expected to operate at LOS “F” at Kuhio Highway. The Pono Kai Driveway is expected to operate at LOS “D”.

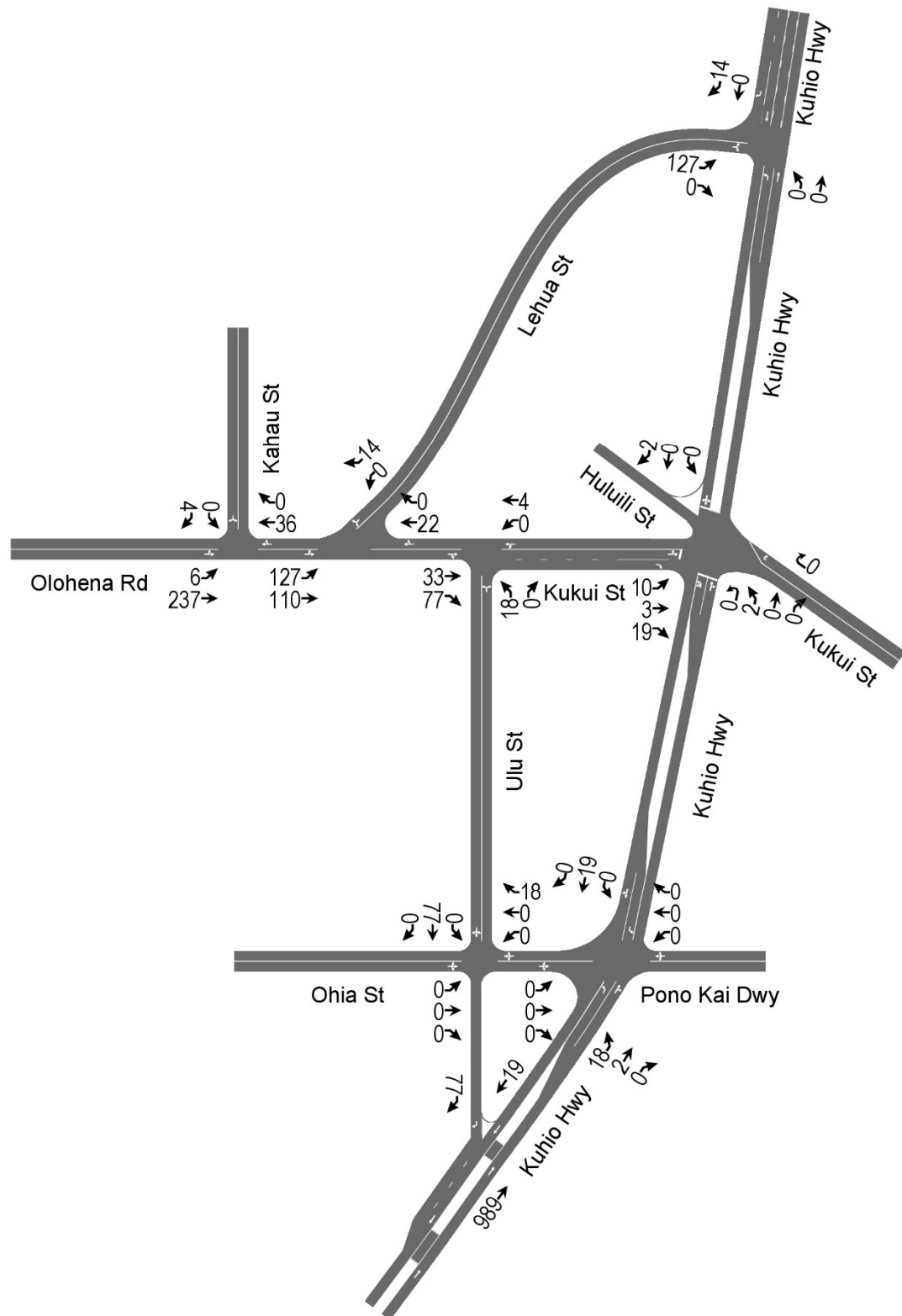
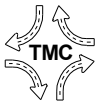
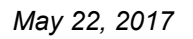


Figure 11. AM Peak Hour Site Traffic Assignment



27

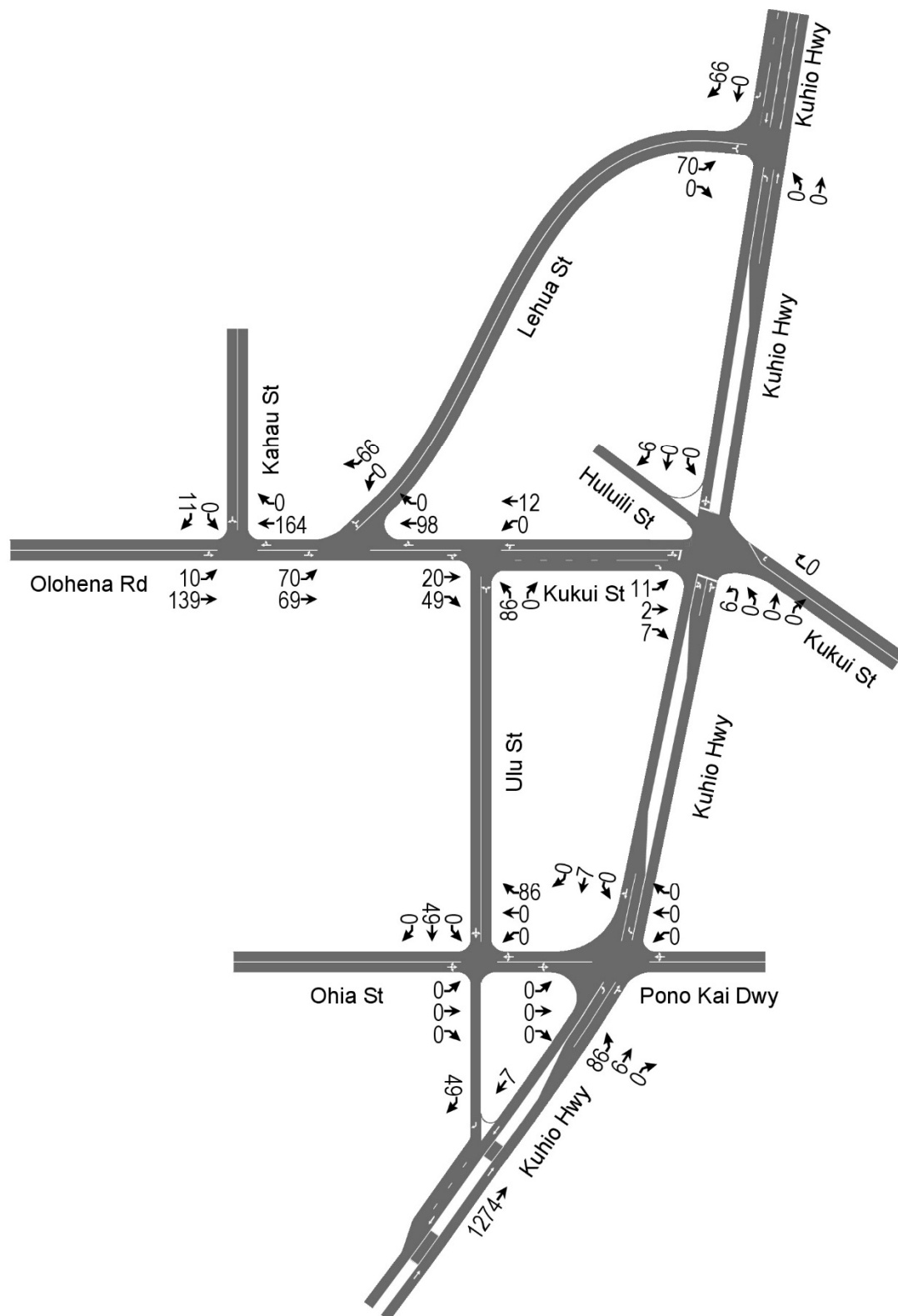
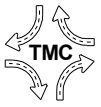


Figure 13. PM Peak Hour Site Traffic Assignment

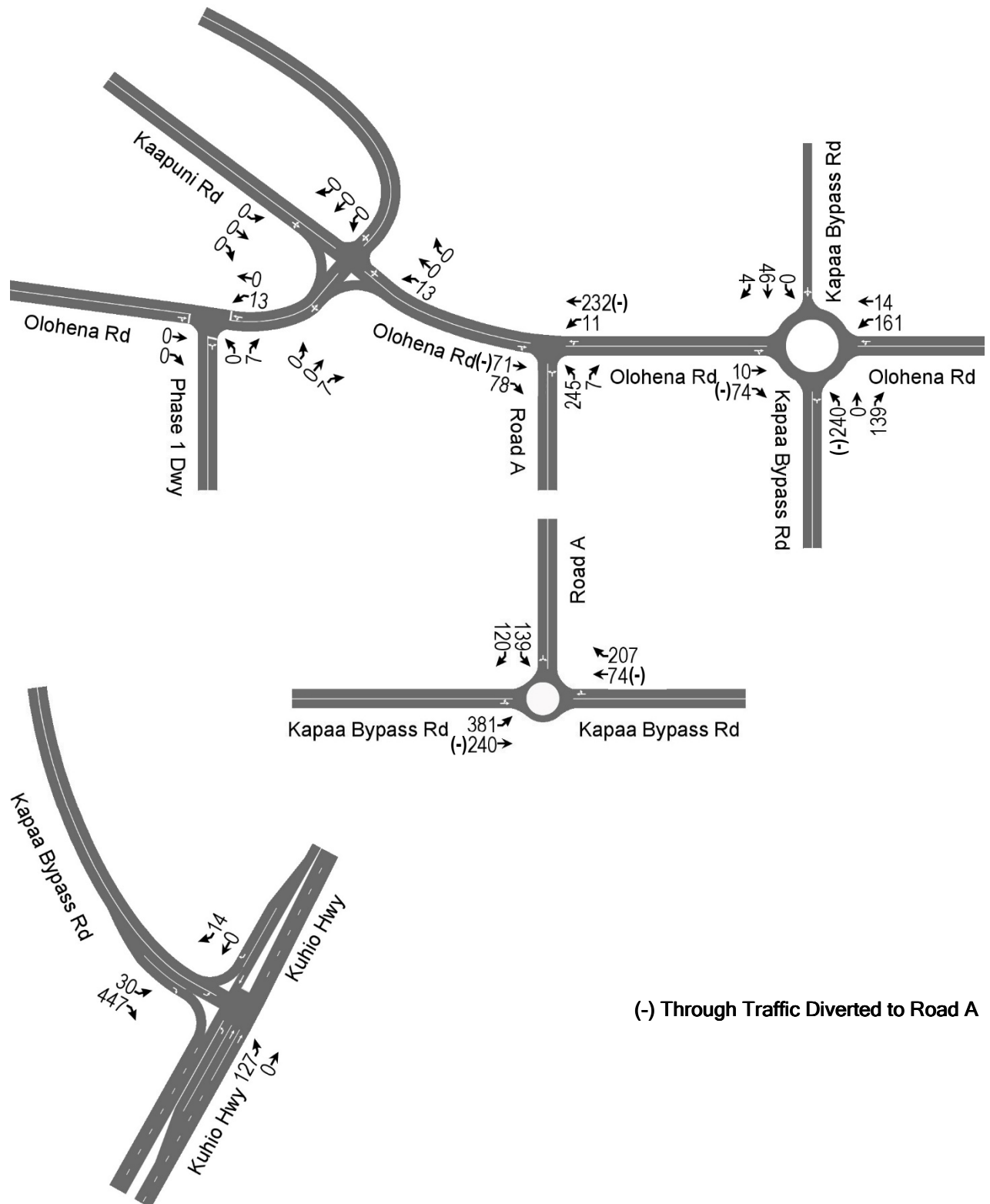
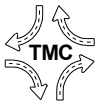
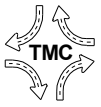


Figure 14. PM Peak Hour Site Traffic Assignment (Cont'd.)



During the AM peak hour of traffic with the proposed project, the overall roundabout intersection of the Kapa`a Bypass Road and Olohena Road is expected to improve from LOS “E” to LOS “D”, during the AM peak hour of traffic with the proposed project. Makai bound Olohena Road is expected to improve from LOS “F” to LOS “E”, due to the diversion of makai bound traffic to Road A. Southbound Kapa`a Bypass Road is expected to worsen from LOS “D” to LOS “E” at Olohena Road.

Kaapuni Road is expected to continue to operate at LOS “F” at Olohena Road. The left-turn movement from the Kapa`a Bypass Road onto Kuhio Highway is expected to operate at LOS “F”, during the AM peak hour of traffic with the proposed project. Figures 15 and 16 depict the AM peak hour volumes with the proposed project.

### **E. PM Peak Hour Traffic Analysis With Project**

During the PM peak hour of traffic with the proposed project, the roundabout intersection of the Kapa`a Bypass Road and Road A is expected to operate at satisfactory Levels of Service. Road A is expected to operate at LOS “D” at Olohena Road. The Phase 1 driveway on Olohena Road is expected to operate at LOS “A”.

The overall intersection of Kuhio Highway and Kukui Street is expected to continue to operate at LOS “A”, during the PM peak hour of traffic with the proposed project. The left-turn movement on makai bound Kukui Street is expected to continue to operate at LOS “F”. The traffic movements in both directions on Kuhio Highway are expected to operate at LOS “A” at Kukui Street, during the PM peak hour of traffic with the proposed project.

Makai bound Lehua Street is expected to continue to operate at LOS “F” at Kuhio Highway, during the PM peak hour of traffic with the proposed project. Makai bound Ohia Street also is expected to operate at LOS “F” at Kuhio Highway. The Pono Kai Driveway is expected to operate at LOS “D” at Kuhio Highway.

During the PM peak hour of traffic with the proposed project, southbound Kapa`a Bypass Road is expected to continue to operate at LOS “F” at its roundabout intersection with Olohena Road. The left-turn and right-turn movements on the Kapa`a Bypass Road (South Junction) at Kuhio Highway are expected to operate at LOS “E” and LOS “D”, respectively. The other intersections in the study area are expected to operate at satisfactory Levels of Service, during the PM peak hour of traffic with the proposed project. Figures 17 and 18 depict the PM peak hour volumes with the proposed project.



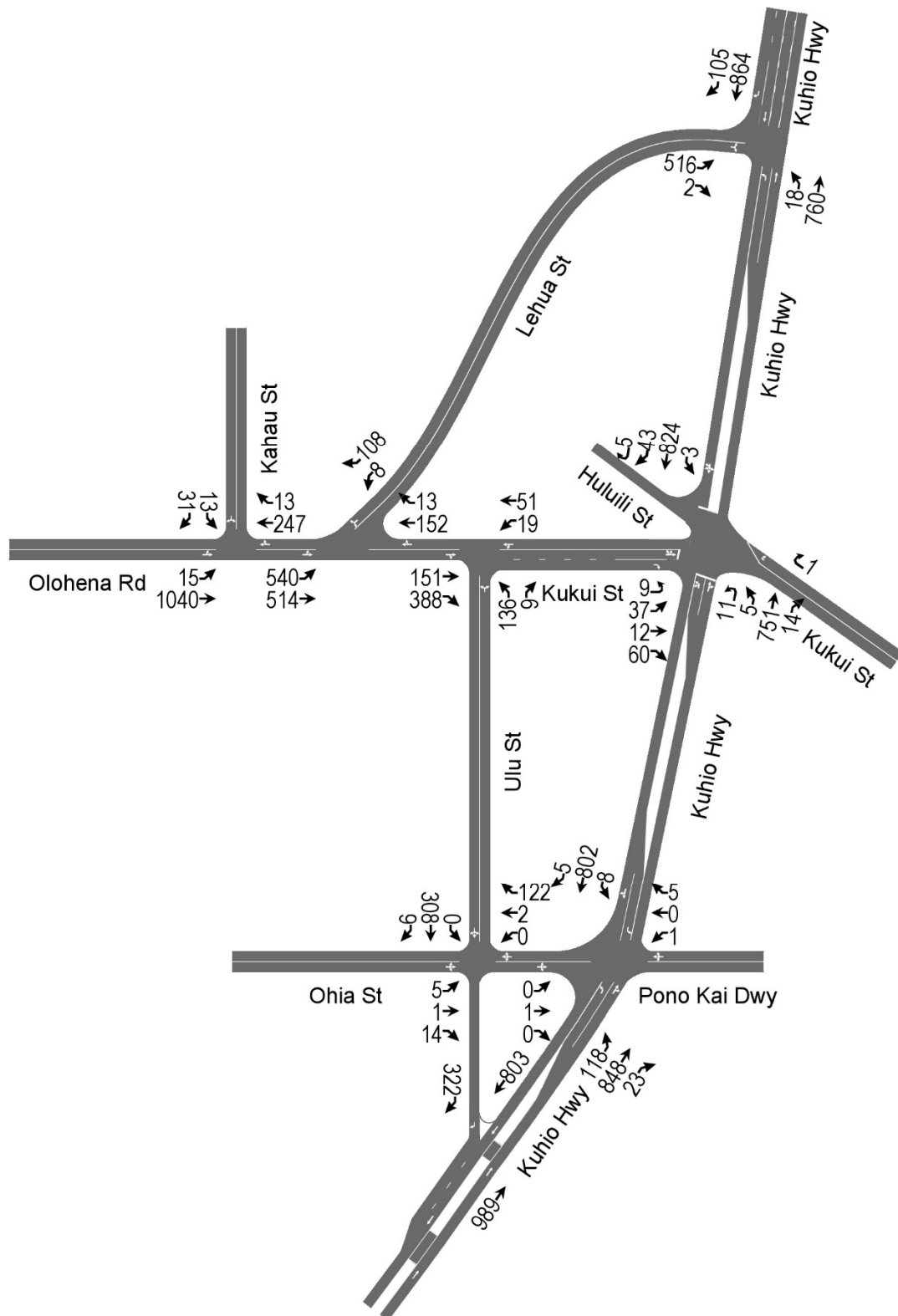
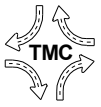


Figure 15. AM Peak Hour Volumes With Project

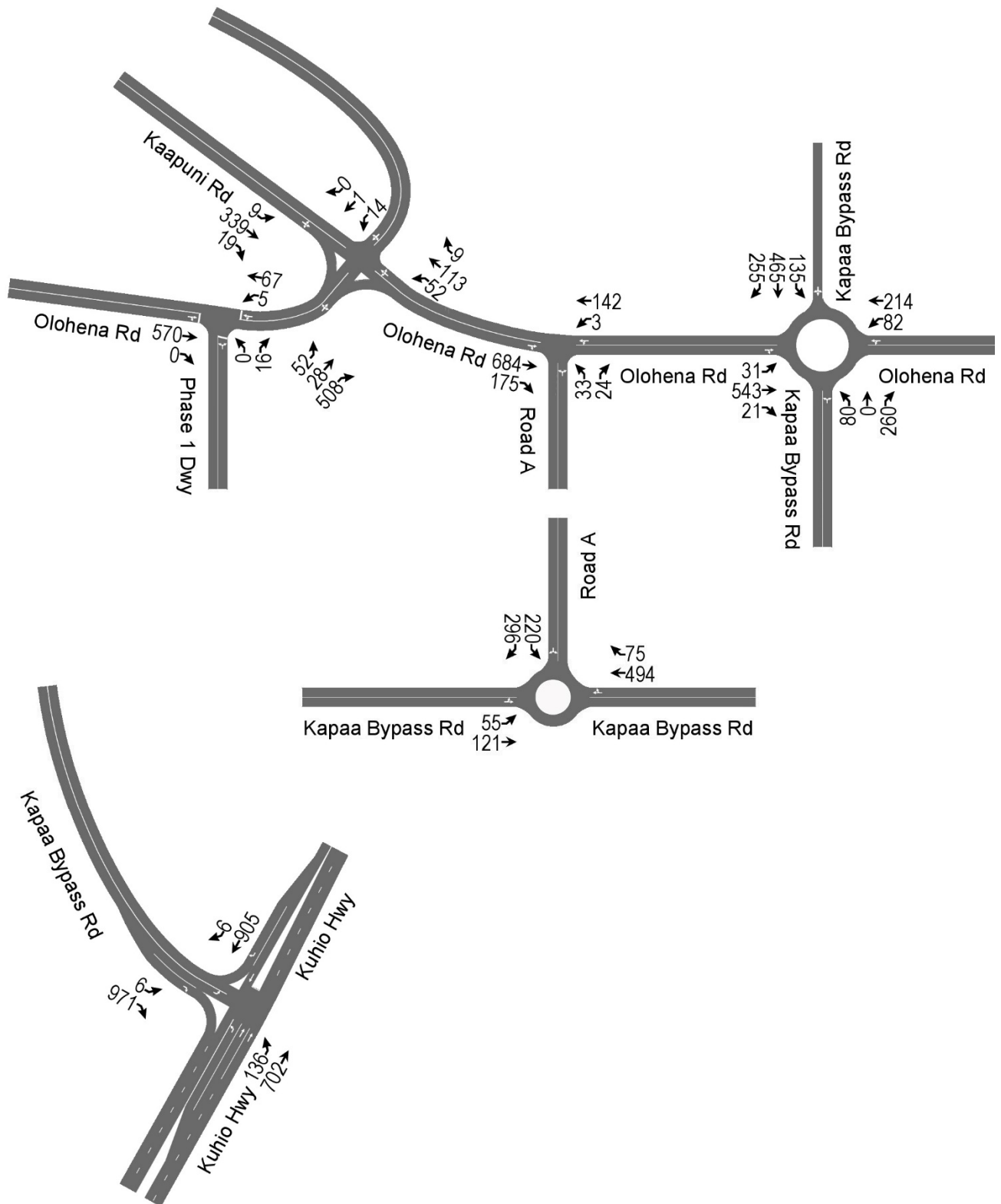
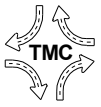


Figure 16. AM Peak Hour Volumes With Project (Cont'd.)

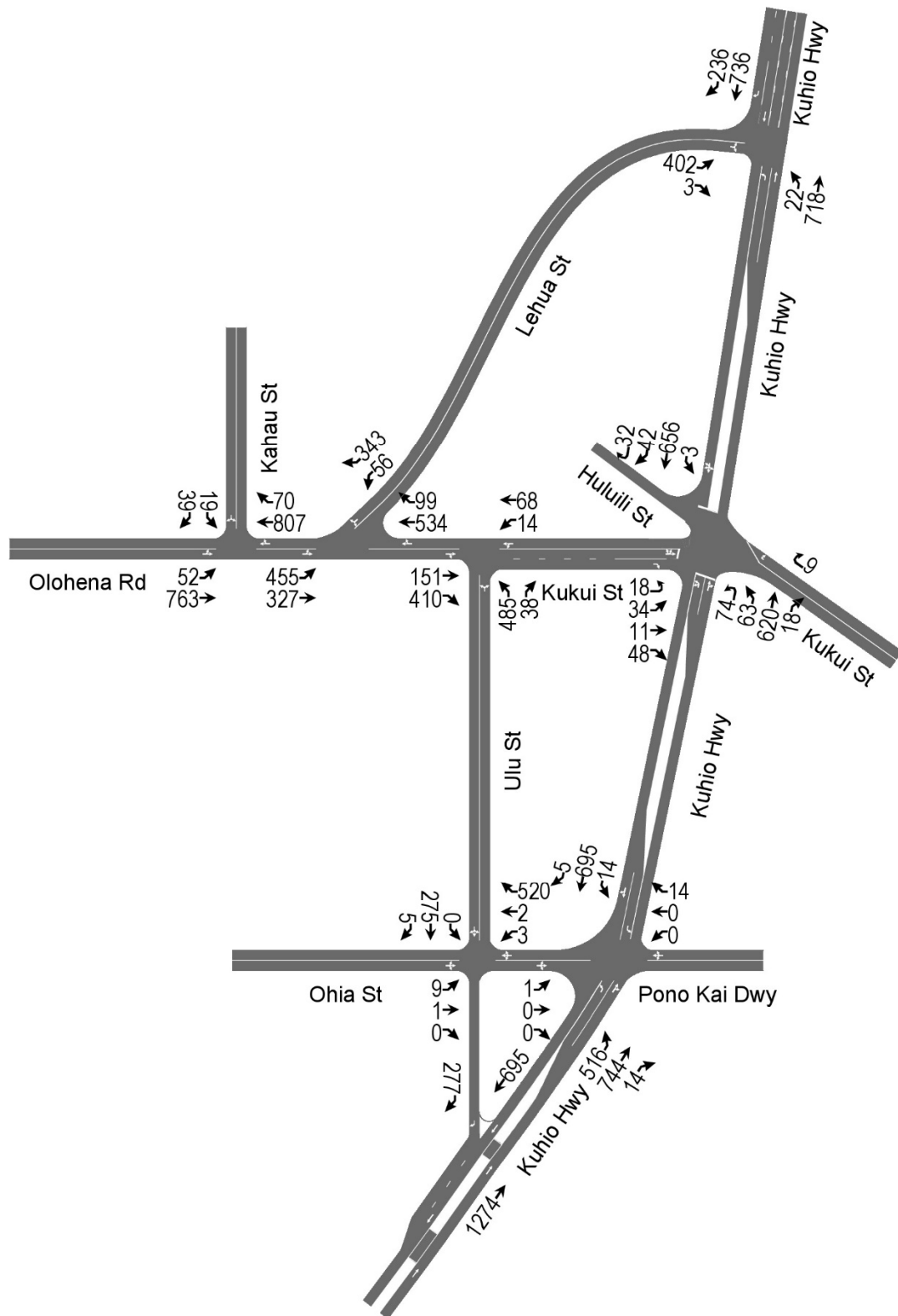
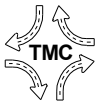


Figure 17. PM Peak Hour Volumes With Project

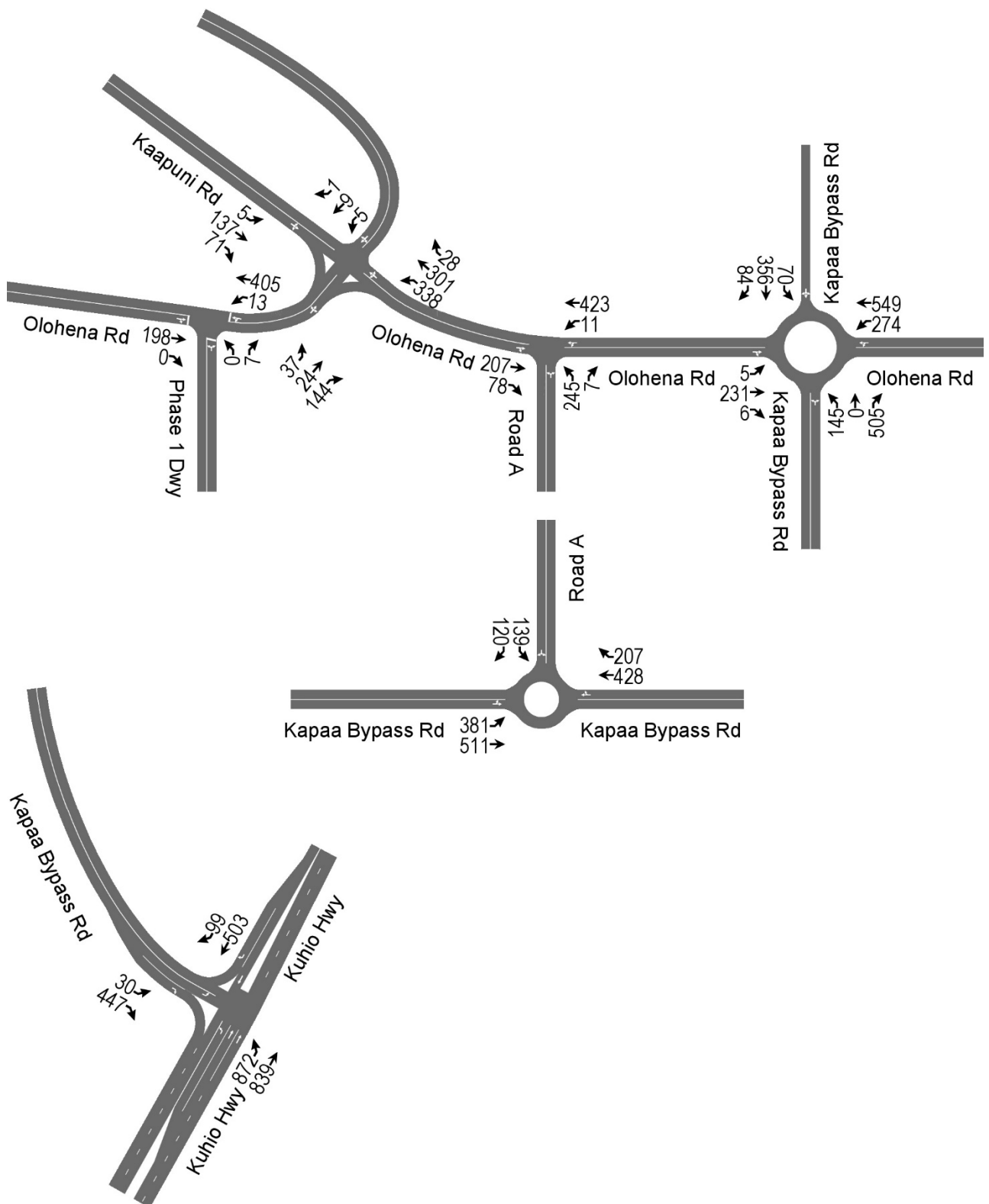
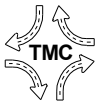
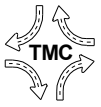


Figure 18. PM Peak Hour Volumes With Project (Cont'd.)



## V. Recommendations and Conclusions

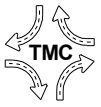
### A. Recommended Traffic Improvements Without Project

The following traffic improvements expand upon the potential traffic solutions, which were cited in the Kapa`a Transportation Solutions, and are recommended to mitigate the existing and expected traffic congestion without the proposed project:

1. Widen Kuhio Highway between the Kapa`a Bypass Road (South Junction) and Kuamoo Road to provide two through lanes in each direction (DOT).
2. Restripe the median on the north leg of Kuhio Highway at the Kapa`a Bypass Road (South Junction) to provide a median refuge lane to facilitate the left-turn movement from the Kapa`a Bypass Road onto northbound Kuhio Highway.
3. Restrict on-street parking along Kuhio Highway within Kapa`a Town. Provide off-street business parking to replace the restricted parking along Kuhio Highway. Restripe Kuhio Highway to provide additional through and/or left-turn lanes.
4. Modify the traffic signal traffic operations at the intersection of Kuhio Highway and Kukui Street to reduce queuing and delays.
5. Add a right-turn bypass lane at the roundabout intersection from southbound Kapa`a Bypass Road to mauka bound Olohena Road.
6. Realign Kaehulua Road to intersect Olohena Road and Kaapuni Road opposite the mauka leg of Olohena Road to create a four-legged intersection with stop-controls on Kaehulua Road and the mauka leg of Olohena Road. Realign/channelize the mauka leg of Olohena Road to intersect Kaapuni Road and the makai leg of Olohena Road to improve the intersection sight distance. Channelize the right-turn movements on the makai bound approaches of Kaapuni Road and Olohena Road.
7. Extend the median refuge lane/two-way left-turn lane on Kuhio Highway from Lehua Street to Kawaihau Road.

DOT is in the process of widening Kuhio Highway from the Kapa`a Bypass Road to Kuamoo Road (Item No. 1 above). The above Item Nos. 2, 3, and 7 are expected to improve the capacity of Kuhio Highway through Kapa`a Town.

Consolidating the intersections of Olohena Road, Kaapuni Road, and Kaehulua Road (Item No. 6 above) into a single four-legged intersection is expected to improve the traffic operations and safety at the intersection. A roundabout intersection was considered for Olohena Road, Kaapuni Road, and Kaehulua Road. However, the existing roadway slopes would have required extensive grading to provide adequate sight distances at a roundabout intersection.



## B. Recommended Traffic Improvements With Project

The following traffic improvements are recommended to mitigate traffic impacts with the proposed project:

1. Construct Road A from Olohena Road to the Kapa`a Bypass Road, as recommended in the Kapa`a Transportation Solutions.
2. Construct a single-lane roundabout at the intersection of Road A and the Kapa`a Bypass Road.

## C. Conclusions

An interim solution to the existing traffic congestion in Kapa`a Town is recommended in the Kapa`a Transportation Solutions. Constructing additional off-street parking areas would provide the opportunity to restripe the existing on-street parking lanes and striped shoulders along Kuhio Highway to provide for additional through traffic lanes and/or median left-turn lanes.

The existing southbound traffic demand in Kapa`a Town is reduced by the Kapa`a Bypass Road. Dedication of the Kapa`a Bypass Road right-of-way along the Hokua Place frontage would assure the continued usage of the existing Kapa`a Bypass Road. Any horizontal realignment and/or widening of the Kapa`a Bypass Road along the project frontage should be coordinated with the development of Hokua Place. Widening of the north leg of the Kapa`a Bypass Road between Olohena Road and Kuhio Highway (North Junction) to provide at a two-way, two-lane roadway would provide additional capacity in the northbound direction.

The construction of the proposed Road A is recommended in the Kapa`a Transportation Solutions to provide additional mauka-makai roadway capacity between Kapa`a Valley and the Kapa`a Bypass Road. By diverting through traffic between Olohena Road and the Kapa`a Bypass Road, Road A is expected to mitigate the project's traffic impacts, during the AM and PM peak hour of traffic with the proposed project at the roundabout intersection of the Kapa`a Bypass Road and Olohena Road.

The roundabout at the intersection of the Kapa`a Bypass Road and Road A will increase the intersection capacity, in anticipation of the increase in demand resulting from the future two-lane widening of the Kapa`a Bypass Road between Olohena Road and Kuhio Highway (North Junction). The proposed roundabout intersection of the Kapa`a Bypass Road and Olohena Road is expected to operate at satisfactory Levels of Service, during the AM and PM peak hours of traffic with the proposed project. Table 7 summarizes the measures of effectiveness (MOE) from the traffic analysis of the intersections in the study area.

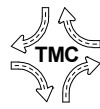


Table 7. Summary of Capacity Analysis															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
Existing AM Peak Hour Traffic	Kuhio Hwy & Kukui St & Huluili St	LOS	F		C	N/A	N/A	A	A	A		A			A
		Delay	115.8		31.5	N/A	N/A	1.3	1.3	2.8		4.5			7.3
		v/c	0.49		0.34	N/A	N/A	0.02	0.02	0.42		0.48			0.49 (maximum)
	Ulu St & Kukui St	LOS	N/A	N/A	N/A	A	A	N/A	B			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.3	0.0	N/A	11.3			N/A	N/A	N/A	2.6
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.16			N/A	N/A	N/A	N/A
	Olohena Rd & Lehua St	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	8.3	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	11.7			4.4
		v/c	0.25	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.15			N/A
	Olohena Rd & Kahau St	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	7.6	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	13.2			0.6
		v/c	0.01	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.08			N/A
	Kuhio Hwy & Lehua St	LOS	E			N/A	N/A	N/A	A	A	N/A	N/A	A	A	A
		Delay	46.6			N/A	N/A	N/A	9.3	0.0	N/A	N/A	0.0	0.0	8.7
		v/c	0.85			N/A	N/A	N/A	0.02	0.40	N/A	N/A	0.45	0.04	N/A
	Kuhio Hwy & Ohia St/Pono Kai Dwy	LOS	E			C			A	N/A	N/A	A	N/A	N/A	A
		Delay	48.9			23.7			9.60	N/A	N/A	9.4	N/A	N/A	0.7
		v/c	0.012			0.025			0.10	N/A	N/A	0.01	N/A	N/A	N/A
	Ulu St & Ohia St	LOS	B			A						A			A
		Delay	10			8.8						0.0			3.1
		v/c	0.023			0.089						N/A			N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	D			A			A			C			C
		Delay	30			5.1			7.1			18.2			20.0
		v/c	0.855			0.204			0.19			0.757			N/A
	Olohena Rd & Kaapuni Rd	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	E			B
		Delay	7.7	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	35.1			12.3
		v/c	0.05	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.751			N/A
	Kaapuni Rd & Kaehulua Rd	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	7.6	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	11.7			0.4
		v/c	0.01	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.024			N/A
	Kuhio Hwy & Kapa`a Bypass Rd	LOS	C	N/A	A	N/A	N/A	N/A	B	N/A	N/A	N/A	N/A	N/A	A
		Delay	20.7	N/A	0.0	N/A	N/A	N/A	10.0	N/A	N/A	N/A	N/A	N/A	0.7
		v/c	0.02	N/A	N/A	N/A	N/A	N/A	0.12	N/A	N/A	N/A	N/A	N/A	N/A
<b>Legend</b>															
EBL – Makai (East) Bound Left-Turn Movement				WBL – Mauka (West) Bound Left-Turn Movement				NBL – North Bound Left-Turn Movement				SBL – South Bound Left-Turn Movement			
EBT – Makai (East) Bound Through Movement				WBT – Mauka (West) Bound Through Movement				NBT – North Bound Through Movement				SBT – South Bound Through Movement			
EBR – Makai (East) Bound Right-Turn Movement				WBR – Mauka (West) Bound Right-Turn Movement				NBR – North Bound Right-Turn Movement				SBR – South Bound Right-Turn Movement			

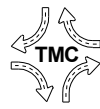


Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
Existing PM Peak Hour Traffic	Kuhio Hwy & Kukui St & Huluili St	LOS	E		A	N/A	N/A	A	A	A		A			A
		Delay	61.6		6.9	N/A	N/A	2.4	2.4	3.4		8.2			7.4
		v/c	0.39		0.22	N/A	N/A	0.18	0.18	0.36		0.48			0.48 (maximum)
	Ulu Street & Kukui Street	LOS	N/A	N/A	N/A	A	A	N/A	C			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.2	0.0	N/A	17.4			N/A	N/A	N/A	7.7
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.567			N/A	N/A	N/A	N/A
	Olohena Road & Lehua Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	E			B
		Delay	9.8	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	47			12.9
		v/c	0.31	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.81			N/A
	Olohena Road & Kahau Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			A
		Delay	9.1	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	19.0			0.9
		v/c	0.04	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.14			N/A
	Kuhio Hwy & Lehua Street	LOS	D			N/A	N/A	N/A	A	A	N/A	N/A	A	A	A
		Delay	29.2			N/A	N/A	N/A	9.0	0.0	N/A	N/A	0.0	0.0	5.0
		v/c	0.68			N/A	N/A	N/A	0.02	0.38	N/A	N/A	0.39	0.09	N/A
	Kuhio Hwy & Ohia St/Pono Kai Driveway	LOS	F			D			B	N/A	N/A	A	N/A	N/A	A
		Delay	143.4			33.1			11.3	N/A	N/A	9.0	N/A	N/A	3.0
		v/c	0.04			0.10			0.40	N/A	N/A	0.01	N/A	N/A	N/A
	Ulu Street & Ohia Street	LOS	C			B			N/A	N/A	N/A	A	N/A	N/A	A
		Delay	15.5			10.6			N/A	N/A	N/A	0.0	N/A	N/A	7.1
		v/c	0.03			0.38			N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	A			B			B			D			B
		Delay	7.9			11.5			11.5			26.6			14.2
		v/c	0.32			0.57			0.61			0.73			N/A
	Olohena Rd & Kaapuni Rd	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			B
		Delay	8.7	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	17.3			4.1
		v/c	0.05	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.39			N/A
	Kaapuni Rd & Kaehulua Rd	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	7.9	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	11.7			0.4
		v/c	0.00	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.02			N/A
	Kuhio Hwy & Kapa`a Bypass Rd	LOS	B	N/A	C	N/A	N/A	N/A	B	N/A	N/A	N/A	N/A	N/A	A
		Delay	14.0	N/A	19.0	N/A	N/A	N/A	12.7	N/A	N/A	N/A	N/A	N/A	6.7
		v/c	0.06	N/A	N/A	N/A	N/A	N/A	0.59	N/A	N/A	N/A	N/A	N/A	N/A



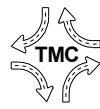


Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
AM Peak Hour Traffic Without Project	Kuhio Hwy & Kukui Street & Huluili Street	LOS	F		C	N/A	N/A	A	A	A		A			A
		Delay	117.0		29.9	N/A	N/A	1.5	1.5	3.7		6.3			8.5
		v/c	0.52		0.36	N/A	N/A	0.03	0.03	0.49		0.57			0.57 (maximum)
	Ulu Street & Kukui Street	LOS	N/A	N/A	N/A	A	A	N/A	B			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.5	0.0	N/A	12.2			N/A	N/A	N/A	2.7
		v/c	N/A	N/A	N/A	0.02	N/A	N/A	0.203			N/A	N/A	N/A	N/A
	Olohena Road & Lehua Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	8.5	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	13.1			4.6
		v/c	0.29	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.19			N/A
	Olohena Road & Kahau Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	7.7	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	14.2			0.6
		v/c	0.01	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.09			N/A
	Kuhio Hwy & Lehua Street	LOS	F			N/A	N/A	N/A	A	A	N/A	N/A	A	A	B
		Delay	104.5			N/A	N/A	N/A	9.7	0.0	N/A	N/A	0.0	0.0	19.3
		v/c	1.33			N/A	N/A	N/A	0.02	0.45	N/A	N/A	0.51	0.06	N/A
	Kuhio Hwy & Ohia St/Pono Kai Driveway	LOS	F			D			B	N/A	N/A	A	N/A	N/A	A
		Delay	65			27.4			10.00	N/A	N/A	9.8	N/A	N/A	0.7
		v/c	0.016			0.036			0.12	N/A	N/A	0.01	N/A	N/A	N/A
	Ulu Street & Ohia Street	LOS	B			A			N/A	N/A	N/A	A	N/A	N/A	A
		Delay	10.3			8.8			N/A	N/A	N/A	0.0	N/A	N/A	3.1
		v/c	0.029			0.101			N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	F			A			A			D			E
		Delay	64.9			5.4			7.4			30.1			38.7
		v/c	1.027			0.233			0.174			0.888			N/A
	Olohena Rd & Kaapuni Rd	LOS	-	-	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	-	-	N/A	N/A	N/A	N/A	N/A	N/A	N/A	11.4			7.8
		v/c	-	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.399			N/A
	Kaapuni Rd & Kaehulua Rd	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	7.6	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	12.6			0.4
		v/c	0.01	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.031			N/A
	Kuhio Hwy & Kapa`a Bypass Rd	LOS	C	N/A	A	N/A	N/A	N/A	B	N/A	N/A	N/A	N/A	N/A	A
		Delay	24.0	N/A	0.0	N/A	N/A	N/A	10.7	N/A	N/A	N/A	N/A	N/A	0.8
		v/c	0.03	N/A	N/A	N/A	N/A	N/A	0.15	N/A	N/A	N/A	N/A	N/A	N/A

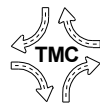


Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
PM Peak Hour Traffic Without Project	Kuhio Hwy & Kukui St & Huluili St	LOS	E		A	N/A	N/A	A	A	A		A			A
		Delay	62.1		8.6	N/A	N/A	2.7	2.7	3.9		9.6			8.3
		v/c	0.42		0.25	N/A	N/A	0.22	0.22	0.41		0.55			0.55 (maximum)
	Ulu Street & Kukui Street	LOS	N/A	N/A	N/A	A	A	N/A	C			N/A	N/A	N/A	B
		Delay	N/A	N/A	N/A	8.5	0.0	N/A	24.7			N/A	N/A	N/A	10.9
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.72			N/A	N/A	N/A	N/A
	Olohena Road & Lehua Street	LOS	B	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	F			E
		Delay	10.8	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	199.9			46.8
		v/c	0.38	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	1.30			N/A
	Olohena Road & Kahau Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			A
		Delay	9.5	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	24.3			1.1
		v/c	0.05	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.20			N/A
	Kuhio Hwy & Lehua Street	LOS	E			N/A	N/A	N/A	A	A	N/A	N/A	A	A	B
		Delay	48.4			N/A	N/A	N/A	9.3	0.0	N/A	N/A	0.0	0.0	19.3
		v/c	0.85			N/A	N/A	N/A	0.03	0.42	N/A	N/A	0.43	0.10	N/A
	Kuhio Hwy & Ohia St/Pono Kai Driveway	LOS	F			E			B	N/A	N/A	A	N/A	N/A	A
		Delay	261.5			47.7			12.80	N/A	N/A	9.3	N/A	N/A	3.5
		v/c	0.067			0.16			0.48	N/A	N/A	0.02	N/A	N/A	N/A
	Ulu Street & Ohia Street	LOS	C			B			N/A	N/A	N/A	A	N/A	N/A	A
		Delay	17.5			11.1			N/A	N/A	N/A	0.0	N/A	N/A	7.4
		v/c	0.033			0.428			N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	A			C			C			F			D
		Delay	9.8			16.8			16.9			72.9			27.7
		v/c	0.399			0.714			0.744			1.002			N/A
	Olohena Road & Kaapuni Road	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			A
		Delay	9.1	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	23.5			5.4
		v/c	0.07	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.538			N/A
	Kaapuni Road & Kaehulua Road	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	B			A
		Delay	8.1	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	12.6			0.4
		v/c	0.00	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.031			N/A
	Kuhio Hwy & Kapa`a Bypass Rd	LOS	C	N/A	D	N/A	N/A	N/A	C	N/A	N/A	N/A	N/A	N/A	A
		Delay	22.6	N/A	25.4	N/A	N/A	N/A	15.6	N/A	N/A	N/A	N/A	N/A	8.6
		v/c	0.12	N/A	N/A	N/A	N/A	N/A	0.70	N/A	N/A	N/A	N/A	N/A	N/A

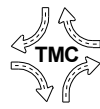


Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
AM Peak Hour Traffic With Project	Kuhio Hwy & Kukui Street & Huluili Street	LOS	F		C	N/A	N/A	A	A	A		A			A
		Delay	118.9		26.1	N/A	N/A	1.8	1.8	4.1		7.0			10.0
		v/c	0.59		0.42	N/A	N/A	0.03	0.03	0.49		0.57			0.59 (maximum)
	Ulu Street & Kukui Street	LOS	N/A	N/A	N/A	A	A	N/A	B			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.8	0.0	N/A	13.6			N/A	N/A	N/A	2.8
		v/c	N/A	N/A	N/A	0.02	N/A	N/A	0.257			N/A	N/A	N/A	N/A
	Olohena Road & Lehua Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			A
		Delay	9.2	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	19			5.4
		v/c	0.39	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.31			N/A
	Olohena Road & Kahau Street	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			A
		Delay	7.8	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	17.3			0.6
		v/c	0.01	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.13			N/A
	Kuhio Hwy & Lehua Street	LOS	F			N/A	N/A	N/A	A	A	N/A	N/A	A	A	F
		Delay	237.5			N/A	N/A	N/A	9.7	0.0	N/A	N/A	0.0	0.0	54.4
		v/c	1.43			N/A	N/A	N/A	0.02	0.45	N/A	N/A	0.51	0.05	N/A
	Kuhio Hwy & Ohia Street/Pono Kai Driveway	LOS	F			D			B	N/A	N/A	A	N/A	N/A	A
		Delay	71.7			29.0			10.2	N/A	N/A	9.8	N/A	N/A	0.8
		v/c	0.02			0.04			0.15	N/A	N/A	0.01	N/A	N/A	N/A
	Ulu Street & Ohia Street	LOS	B			A			N/A	N/A	N/A	A	N/A	N/A	A
		Delay	11			8.9			N/A	N/A	N/A	0.0	N/A	N/A	2.9
		v/c	0.03			0.12			N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	E			A			B			E			D
		Delay	42.1			5.7			14.5			40.7			31.9
		v/c	0.91			0.27			0.53			0.95			N/A
	Road A & Olohena Road	LOS	N/A	N/A	N/A	A	A	N/A	C			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	9.6	0.0	N/A	17.9			N/A	N/A	N/A	1.0
		v/c	N/A	N/A	N/A	0.00	N/A	N/A	0.17			N/A	N/A	N/A	N/A
	Olohena Road & Kaapuni Road	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	F			D
		Delay	7.8	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	106.1			35.5
		v/c	0.06	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	1.078			N/A
	Kaapuni Rd & Kaehulua Rd	LOS	A	A	N/A	N/A	B	N/A	N/A	N/A	N/A	B			A
		Delay	7.6	0.0	N/A	N/A	12.6	N/A	N/A	N/A	N/A	12.6			0.4
		v/c	0.01	-	N/A	N/A	0.03	N/A	N/A	N/A	N/A	0.03			N/A
	Phase 1 Dwy & Olohena Rd	LOS	N/A	N/A	N/A	A	A	N/A	B			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.6	0.0	N/A	12.1			N/A	N/A	N/A	0.4
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.03			N/A	N/A	N/A	N/A

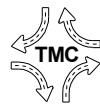


Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
AM Peak Hour Traffic With Project (Cont'd.)	Kapa`a Bypass Road & Road A	LOS	A			A			N/A			C			B
		Delay	4.8			7.3			N/A			15.1			10.1
		v/c	0.16			0.45			N/A			0.64			N/A
	Kuhio Hwy & Kapa`a Bypass Road	LOS	F	N/A	A	N/A	N/A	N/A	B	N/A	N/A	N/A	N/A	N/A	A
		Delay	51.1	N/A	0.0	N/A	N/A	N/A	10.9	N/A	N/A	N/A	N/A	N/A	1.0
		v/c	0.07	N/A	N/A	N/A	N/A	N/A	0.18	N/A	N/A	N/A	N/A	N/A	N/A
AM Peak Hour Traffic With Project - Improved	Kuhio Hwy & Kukui Street & Huluili Street	LOS	C		B	N/A	N/A	A	A	A		A			A
		Delay	32.8		11.4	N/A	N/A	0.0	3.8	7.0		8.2			.6
		v/c	0.31		0.27	N/A	N/A	N/A	0.03	0.56		0.63			0.63 (maximum)
	Kuhio Hwy & Lehua Street	LOS	F			N/A	N/A	N/A	A	A	N/A	N/A	A	A	E
		Delay	196.5			N/A	N/A	N/A	9.7	0.0	N/A	N/A	0.0	0.0	45.0
		v/c	1.34			N/A	N/A	N/A	0.02	0.45	N/A	N/A	0.51	0.06	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	C			A			B			B		A	B
		Delay	23.9			5.4			11.1			12.1		0.0	12.9
		v/c	0.79			0.25			0.46			0.61		0.13	N/A
	Olohena Rd & Kaapuni Road & Kaehulua Road	LOS	A	A		A	A		D			E			B
		Delay	7.5	0.0		8.1	0.0		34.9			43.2			19.0
		v/c	0.01	N/A		0.04	-		0.87			0.14			N/A
	Kuhio Hwy & Kapa`a Bypass Road	LOS	C	N/A	A	N/A	N/A	N/A	B	A	N/A	N/A	A	A	A
		Delay	21.8	N/A	0.0	N/A	N/A	N/A	10.9	0.0	N/A	N/A	0.0	0.0	0.9
		v/c	0.03	N/A	N/A	N/A	N/A	N/A	0.18	N/A	N/A	N/A	N/A	N/A	N/A
PM Peak Hour Traffic With Project	Kuhio Hwy & Kukui Street & Huluili Street	LOS	E		B	N/A	N/A	A	A	A		B			A
		Delay	63.3		11.1	N/A	N/A	3.0	3.0	4.2		10.5			9.4
		v/c	0.48		0.27	N/A	N/A	0.23	0.23	0.42		0.56			0.56 (maximum)
	Ulu Street & Kukui Street	LOS	N/A	N/A	N/A	A	A	N/A	E			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	8.7	0.0	N/A	49.9			N/A	N/A	N/A	22.5
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.93			N/A	N/A	N/A	N/A
	Olohena Road & Lehua Street	LOS	B	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	F			A
		Delay	12.7	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	714.0			160.2
		v/c	0.49	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	2.45			N/A
	Olohena Road & Kahau Street	LOS	B	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	E			A
		Delay	10.4	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	36.5			1.5
		v/c	0.07	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.34			N/A



Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
PM Peak Hour Traffic With Project (Cont'd.)	Kuhio Hwy & Lehua Street	LOS	F			N/A	N/A	N/A	A	A	N/A	N/A	A	A	B
		Delay	85.7			N/A	N/A	N/A	9.3	0.0	N/A	N/A	0.0	0.0	12.5
		v/c	1.03			N/A	N/A	N/A	0.03	0.42	N/A	N/A	0.43	0.14	N/A
	Kuhio Hwy & Ohia St/Pono Kai Driveway	LOS	F			B			B	N/A	N/A	A	N/A	N/A	A
		Delay	401.7			14.6			14.6	N/A	N/A	9.3	N/A	N/A	4.1
		v/c	0.10			0.04			0.58	N/A	N/A	0.02	N/A	N/A	N/A
	Ulu Street & Ohia Street	LOS	C			B			N/A	N/A	N/A	A	N/A	N/A	A
		Delay	21.7			12.1			N/A	N/A	N/A	0.0	N/A	N/A	8.1
		v/c	0.04			0.51			N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Olohena Rd	LOS	B			B			B			F			D
		Delay	11.3			14.2			13.3			84.7			29.8
		v/c	0.39			0.72			0.65			1.05			N/A
	Road A & Olohena Road	LOS	N/A	N/A	N/A	A	A	N/A	D			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	7.8	0.0	N/A	26.7			N/A	N/A	N/A	7.0
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.61			N/A	N/A	N/A	N/A
	Olohena Road & Kaapuni Road	LOS	A	A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	C			D
		Delay	9.1	0.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	24.5			5.5
		v/c	0.07	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.55			N/A
	Kaapuni Road & Kaehulua Road	LOS	A	A	N/A	N/A	B	N/A	N/A	N/A	N/A	B			A
		Delay	8.1	0.0	N/A	N/A	12.6	N/A	N/A	N/A	N/A	12.6			0.4
		v/c	0.00	-	N/A	N/A	0.03	N/A	N/A	N/A	N/A	0.03			N/A
	Phase 1 Dwy & Olohena Road	LOS	N/A	N/A	N/A	A	A	N/A	A			N/A	N/A	N/A	A
		Delay	N/A	N/A	N/A	7.6	0.0	N/A	9.3			N/A	N/A	N/A	0.2
		v/c	N/A	N/A	N/A	0.01	N/A	N/A	0.01			N/A	N/A	N/A	N/A
	Kapa`a Bypass Rd & Road A	LOS	C			C			N/A			A			B
		Delay	16.0			16.1			N/A			7.4			14.8
		v/c	0.76			0.70			75			0.30			N/A
	Kuhio Hwy & Kapa`a Bypass Road	LOS	E	N/A	A	N/A	N/A	N/A	C	N/A	N/A	N/A	N/A	N/A	B
		Delay	44.7	N/A	0.0	N/A	N/A	N/A	21.1	N/A	N/A	N/A	N/A	N/A	12.0
		v/c	0.25	N/A	N/A	N/A	N/A	N/A	0.81	N/A	N/A	N/A	N/A	N/A	N/A
PM Peak Hour Traffic With Project - Improved	Kuhio Hwy & Kukui Street & Huliuli Street	LOS	C			A	N/A	N/A	A	A		B			B
		Delay	32.0			1.8	N/A	N/A	0.1	4.0		16.7			11.4
		v/c	0.32			0.18	N/A	N/A	N/A	0.24		0.66			0.66 (maximum)
	Kuhio Hwy & Lehua Street	LOS	F			N/A	N/A	N/A	A	A	N/A	N/A	A	A	B
		Delay	65.9			N/A	N/A	N/A	9.3	0.0	N/A	N/A	0.0	0.0	12.7
		v/c	0.96			N/A	N/A	N/A	0.03	0.42	N/A	N/A	0.43	0.14	N/A



Table 7. Summary of Capacity Analysis (Cont'd.)															
Scenario	Intersection	MOE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Intersection
PM Peak Hour Traffic With Project – Improved (Cont'd.)	Kapa`a Bypass Rd & Olohena Rd	LOS	A			B			B			D		A	B
		Delay	9.1			12.5			11.2			25.1		0.0	13.7
		v/c	0.33			0.68			0.60			0.72		0.04	N/A
	Olohena Rd & Kaapuni Road & Kaehulua Road	LOS	A	A		A	A		C			E			A
		Delay	7.9	0.0		8.2	0.0		21.3			36.0			7.1
		v/c	0.00	N/A		0.23	N/A		0.48			0.12			N/A
	Kuhio Hwy & Kapa`a Bypass Road	LOS	E	N/A	A	N/A	N/A	N/A	C	A	N/A	N/A	A	A	A
		Delay	44.9	N/A	0.0	N/A	N/A	N/A	22.4	0.0	N/A	N/A	0.0	0.0	8.9
		v/c	0.25	N/A	N/A	N/A	N/A	N/A	0.81	N/A	N/A	N/A	N/A	N/A	N/A

**TRAFFIC IMPACT ANALYSIS REPORT UPDATE**

**FOR THE PROPOSED**

**HOKUA PLACE**

**KAPA`A, KAUAI, HAWAII**

**TAX MAP KEY: (4) 4-3-03: 01**

**APPENDIX A**

**TRAFFIC COUNT DATA**

Study Name Kuhio Hwy Kukui ST 3-15-17 to 3-17-17  
Start Date 3/15/17  
Start Time 3:00 PM  
Site Code Hokua Place

Start	Kukui St Makai Bound				Kukui St Mauka Bound				Kuhio Hwy Northbound				Kuhio Hwy Southbound				Intersection	
	LT-Huluili	LT-Kuhio	Thru	Right-Turn	Left-Turn	Thru	Thru-Huluili	RT-Kuhio	LT-Kukui	LT-Huluili	Thru	Right-Turn	Left-Turn	Thru	RT-Kukui	RT-Huluili	15-Min Totals	Hourly Totals
3/15/17																		
3:00 PM	4	11	6	12	0	0	0	4	4	1	121	2	3	126	14	1	309	1160
3:15 PM	1	16	1	9	0	0	0	7	4	1	89	2	1	129	16	0	276	1136
3:30 PM	10	12	0	9	0	0	0	7	5	0	86	0	3	144	12	1	289	1142
3:45 PM	9	15	4	16	1	0	0	1	11	2	99	1	1	113	13	0	286	1125
4:00 PM	7	18	6	11	0	0	0	2	7	0	96	1	3	111	18	5	285	1136
4:15 PM	4	16	4	11	0	0	0	8	7	1	97	0	1	112	20	1	282	1123
4:30 PM	3	9	1	13	0	0	0	2	8	0	110	1	2	107	15	1	272	1153
4:45 PM	2	12	1	11	0	0	0	1	5	0	103	2	3	136	17	4	297	1177
5:00 PM	4	13	4	4	0	0	0	2	7	7	82	1	2	133	10	3	272	1144
5:15 PM	12	9	5	16	0	0	0	1	4	5	109	0	2	134	9	6	312	
5:30 PM	4	3	3	6	0	0	0	2	6	6	123	2	0	133	7	1	296	
5:45 PM	2	7	5	6	0	0	0	3	1	1	108	0	1	124	5	1	264	
3/16/17																		
7:00 AM	0	8	2	8	0	0	0	2	1	1	124	1	0	184	2	2	335	1397
7:15 AM	2	6	0	9	0	0	0	0	2	1	166	3	0	182	9	0	380	1410
7:30 AM	0	6	1	15	0	0	0	0	7	5	143	1	1	153	6	1	339	1367
7:45 AM	2	3	3	15	0	0	0	0	4	4	153	1	0	151	7	0	343	1399
8:00 AM	2	11	4	9	0	0	0	1	3	1	143	2	2	165	2	3	348	1397
8:15 AM	0	0	1	3	0	0	0	4	4	1	143	4	3	172	2	0	337	
8:30 AM	0	5	3	6	0	0	0	9	7	0	153	9	1	170	7	1	371	
8:45 AM	3	9	0	8	0	0	0	9	2	2	151	5	2	143	6	1	341	
3/16/17																		
3:00 PM	7	7	2	9	1	0	0	1	5	1	93	3	2	134	10	4	279	1182
3:15 PM	2	11	2	3	0	0	0	3	10	2	129	1	2	125	9	3	302	1203
3:30 PM	2	7	4	18	0	0	0	4	7	3	105	3	2	144	14	1	314	1160
3:45 PM	3	8	5	7	0	0	0	10	2	1	96	0	1	148	5	1	287	1160
4:00 PM	3	3	4	9	0	0	0	0	3	4	113	4	2	139	10	6	300	1162
4:15 PM	2	8	1	6	0	0	0	0	3	3	111	1	2	114	8	0	259	1176
4:30 PM	1	9	1	3	0	0	0	0	4	0	136	2	1	148	7	2	314	1269
4:45 PM	6	5	2	3	0	0	0	0	10	1	114	2	3	134	8	1	289	1281
5:00 PM	7	4	1	8	0	0	0	6	4	3	119	3	2	144	10	3	314	1305
5:15 PM	4	5	2	9	0	0	0	2	15	14	137	4	0	145	8	7	352	
5:30 PM	2	6	0	0	0	0	0	2	3	1	154	3	0	145	5	5	326	
5:45 PM	0	6	1	7	0	0	0	1	3	0	134	3	1	147	6	4	313	
3/17/17																		
6:45 AM	0	4	1	6	0	0	0	2	2	0	104	1	1	167	6	0	294	1326
7:00 AM	1	3	1	8	0	0	0	3	3	0	101	2	0	179	7	0	308	1386
7:15 AM	0	4	0	11	0	0	0	2	5	0	172	2	0	170	5	0	371	1415
7:30 AM	1	10	1	12	0	0	0	3	9	1	141	2	1	167	5	0	353	1395
7:45 AM	1	7	2	12	0	0	0	2	3	0	145	2	2	164	14	0	354	1364
8:00 AM	1	9	0	14	0	0	0	4	11	2	135	3	2	149	7	0	337	
8:15 AM	3	5	1	7	0	0	0	11	5	0	146	4	2	155	12	0	351	
8:30 AM	0	6	0	6	0	0	0	2	4	0	125	3	2	164	10	0	322	

AM Peak Hour Traffic

7:15 AM	6	26	8	48	0	0	0	1	16	11	605	7	3	651	24	4	1410
PHF	0.75	1.08	N/A	1.33	N/A	N/A	N/A	N/A	2.00	2.75	0.91	0.58	N/A	0.89	0.67	N/A	0.93
PHV	8	24	0	36	0	0	0	0	8	4	664	12	0	728	36	0	1520
T Factor	0%	15%	0%	4%	N/A	N/A	N/A	0%	6%	9%	4%	0%	0%	1%	4%	0%	

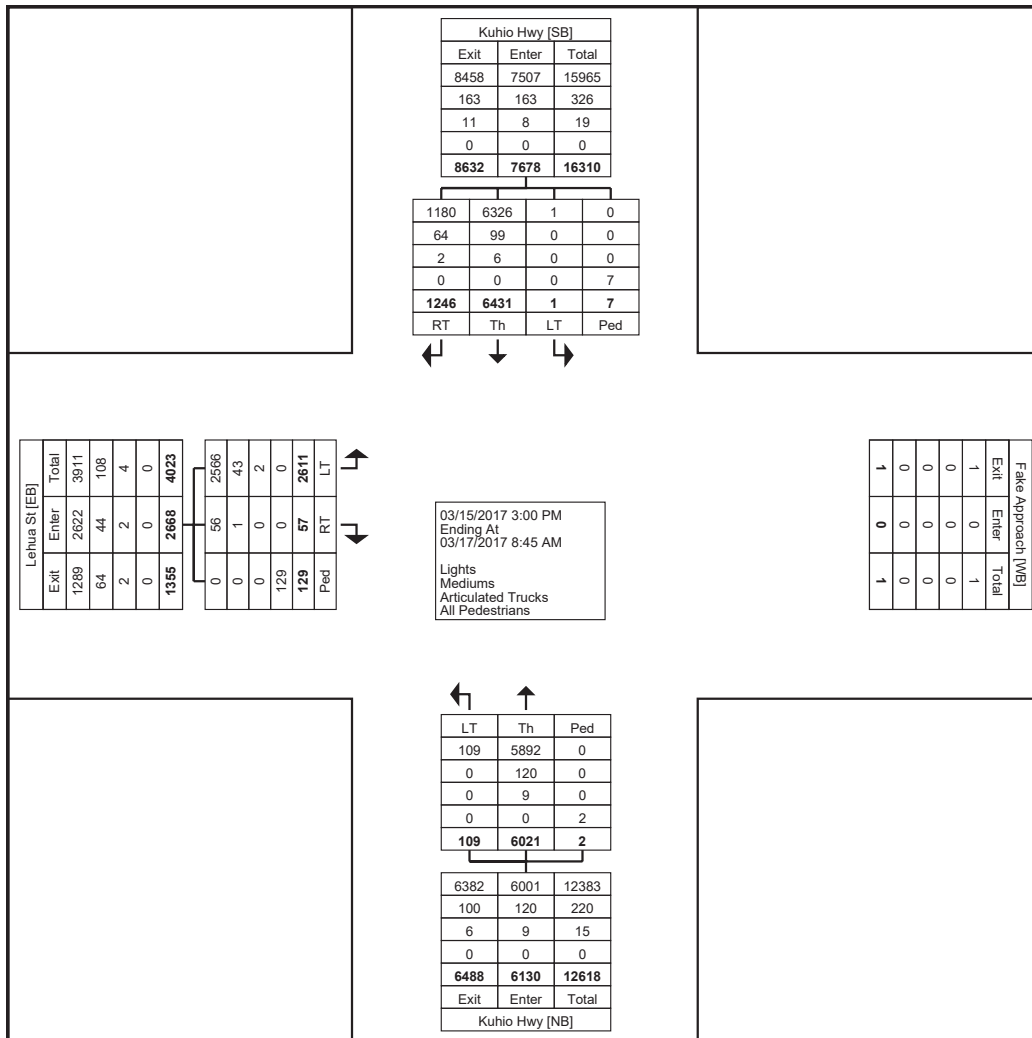
PM Peak Hour Traffic

5:00 PM	13	21	4	24	0	0	0	11	25	18	544	13	3	581	29	19	1305
PHF	0.81	1.05	0.50	0.67	N/A	N/A	N/A	1.38	0.42	0.32	0.99	0.81	N/A	1.00	0.91	0.68	0.93
PHV	16	20	8	36	0	0	0	8	60	56	548	16	0	580	32	28	1408
T Factor	0%	0%	0%	0%	N/A	N/A	N/A	0%	0%	0%	0%	0%	0%	1%	0%	0%	



## Turning Movement Data

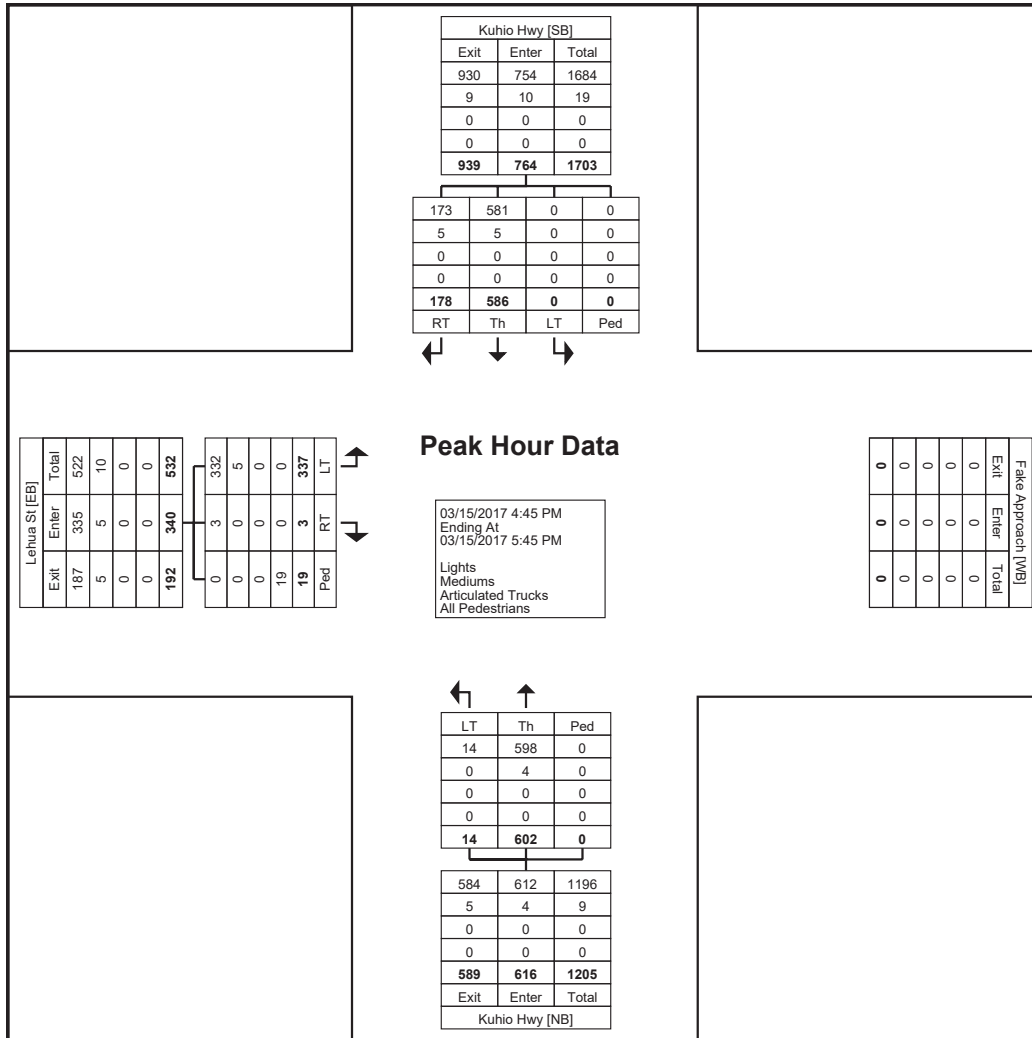
Start Time	Lehua St Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
3:00 PM	71	3	1	74	2	142	0	144	0	140	59	0	199	417
3:15 PM	68	4	14	72	4	137	0	141	0	146	51	0	197	410
3:30 PM	75	2	8	77	2	136	0	138	0	154	56	0	210	425
3:45 PM	72	1	2	73	2	148	0	150	0	134	45	0	179	402
Hourly Total	286	10	25	296	10	563	0	573	0	574	211	0	785	1654
4:00 PM	69	4	6	73	1	145	1	146	0	135	54	0	189	408
4:15 PM	72	2	1	74	5	144	1	149	0	139	37	0	176	399
4:30 PM	75	2	5	77	3	143	0	146	0	130	45	0	175	398
4:45 PM	74	0	5	74	5	160	0	165	0	164	38	0	202	441
Hourly Total	290	8	17	298	14	592	2	606	0	568	174	0	742	1646
5:00 PM	82	0	5	82	0	134	0	134	0	140	55	0	195	411
5:15 PM	85	1	6	86	7	145	0	152	0	145	51	0	196	434
5:30 PM	96	2	3	98	2	163	0	165	0	137	34	0	171	434
5:45 PM	69	2	8	71	1	161	0	162	0	131	25	0	156	389
Hourly Total	332	5	22	337	10	603	0	613	0	553	165	0	718	1668
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
7:00 AM	51	0	1	51	0	140	0	140	0	213	8	0	221	412
7:15 AM	86	0	2	86	4	168	0	172	0	191	20	0	211	469
7:30 AM	95	1	0	96	1	163	0	164	0	170	15	0	185	445
7:45 AM	64	1	2	65	1	180	0	181	0	184	15	0	199	445
Hourly Total	296	2	5	298	6	651	0	657	0	758	58	0	816	1771
8:00 AM	45	0	4	45	2	169	0	171	1	189	18	0	208	424
8:15 AM	29	1	3	30	10	145	0	155	0	194	12	0	206	391
8:30 AM	34	3	3	37	2	168	0	170	0	185	16	0	201	408
8:45 AM	41	1	10	42	5	161	0	166	0	150	21	0	171	379
Hourly Total	149	5	20	154	19	643	0	662	1	718	67	0	786	1602
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	85	2	3	87	3	131	0	134	0	153	49	0	202	423
3:15 PM	67	1	7	68	5	162	0	167	0	138	42	0	180	415
3:30 PM	71	4	4	75	6	145	0	151	0	155	55	0	210	436
3:45 PM	78	2	1	80	5	141	0	146	0	146	35	0	181	407
Hourly Total	301	9	15	310	19	579	0	598	0	592	181	0	773	1681
4:00 PM	71	0	0	71	4	148	0	152	0	163	51	0	214	437
4:15 PM	66	2	1	68	2	145	0	147	0	119	39	0	158	373
4:30 PM	68	3	0	71	1	175	0	176	0	158	45	0	203	450
4:45 PM	81	1	1	82	1	134	0	135	0	146	25	0	171	388
Hourly Total	286	6	2	292	8	602	0	610	0	586	160	0	746	1648
5:00 PM	80	1	1	81	3	140	0	143	0	163	34	0	197	421
5:15 PM	74	1	6	75	2	161	0	163	0	160	27	0	187	425
5:30 PM	50	2	0	52	4	167	0	171	0	158	28	0	186	409
5:45 PM	55	1	2	56	3	133	0	136	0	151	33	0	184	376
Hourly Total	259	5	9	264	12	601	0	613	0	632	122	0	754	1631
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6:45 AM	32	4	1	36	2	107	0	109	0	178	13	7	191	336
Hourly Total	32	4	1	36	2	107	0	109	0	178	13	7	191	336
7:00 AM	62	1	2	63	1	115	0	116	0	183	13	0	196	375
7:15 AM	62	0	1	62	0	170	0	170	0	188	12	0	200	432
7:30 AM	87	0	1	87	2	181	0	183	0	177	9	0	186	456
7:45 AM	69	0	0	69	1	173	0	174	0	195	18	0	213	456
Hourly Total	280	1	4	281	4	639	0	643	0	743	52	0	795	1719
8:00 AM	42	1	3	43	4	144	0	148	0	168	10	0	178	369
8:15 AM	35	1	2	36	1	156	0	157	0	189	21	0	210	403
8:30 AM	23	0	4	23	0	141	0	141	0	172	12	0	184	348
Grand Total	2611	57	129	2668	109	6021	2	6130	1	6431	1246	7	7678	16476
Approach %	97.9	2.1	-	-	1.8	98.2	-	-	0.0	83.8	16.2	-	-	-
Total %	15.8	0.3	-	16.2	0.7	36.5	-	37.2	0.0	39.0	7.6	-	46.6	-
Lights	2566	56	-	2622	109	5892	-	6001	1	6326	1180	-	7507	16130
% Lights	98.3	98.2	-	98.3	100.0	97.9	-	97.9	100.0	98.4	94.7	-	97.8	97.9
Mediums	43	1	-	44	0	120	-	120	0	99	64	-	163	327
% Mediums	1.6	1.8	-	1.6	0.0	2.0	-	2.0	0.0	1.5	5.1	-	2.1	2.0
Articulated Trucks	2	0	-	2	0	9	-	9	0	6	2	-	8	19
% Articulated Trucks	0.1	0.0	-	0.1	0.0	0.1	-	0.1	0.0	0.1	0.2	-	0.1	0.1
All Pedestrians	-	-	129	-	-	-	2	-	-	-	-	7	-	-
% All Pedestrians	-	-	100.0	-	-	-	100.0	-	-	-	-	100.0	-	-



Turning Movement Data Plot

### Turning Movement Peak Hour Data (4:45 PM)

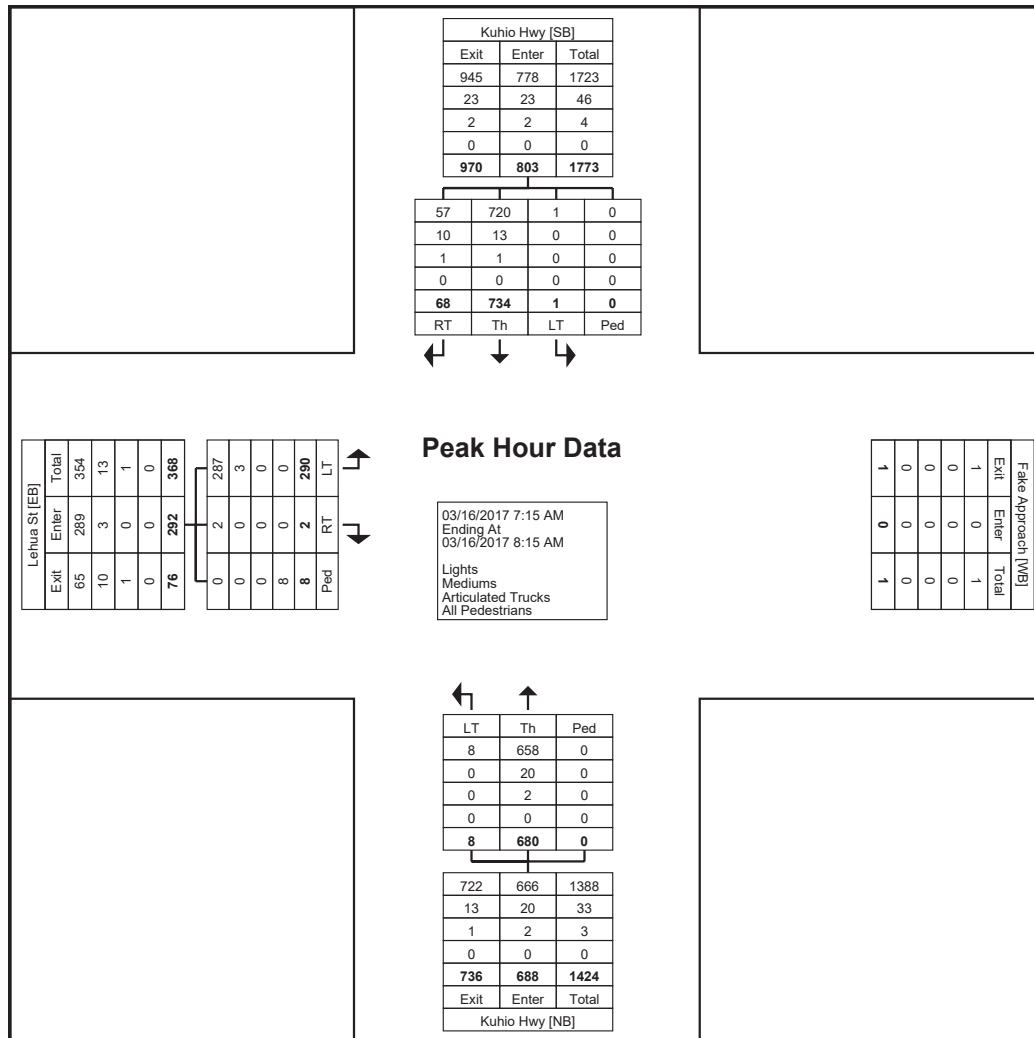
Start Time	Lehua St Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
4:45 PM	74	0	5	74	5	160	0	165	0	164	38	0	202	441
5:00 PM	82	0	5	82	0	134	0	134	0	140	55	0	195	411
5:15 PM	85	1	6	86	7	145	0	152	0	145	51	0	196	434
5:30 PM	96	2	3	98	2	163	0	165	0	137	34	0	171	434
Total	337	3	19	340	14	602	0	616	0	586	178	0	764	1720
Approach %	99.1	0.9	-	-	2.3	97.7	-	-	0.0	76.7	23.3	-	-	-
Total %	19.6	0.2	-	19.8	0.8	35.0	-	35.8	0.0	34.1	10.3	-	44.4	-
PHF	0.878	0.375	-	0.867	0.500	0.923	-	0.933	0.000	0.893	0.809	-	0.946	0.975
Lights	332	3	-	335	14	598	-	612	0	581	173	-	754	1701
% Lights	98.5	100.0	-	98.5	100.0	99.3	-	99.4	-	99.1	97.2	-	98.7	98.9
Mediums	5	0	-	5	0	4	-	4	0	5	5	-	10	19
% Mediums	1.5	0.0	-	1.5	0.0	0.7	-	0.6	-	0.9	2.8	-	1.3	1.1
Articulated Trucks	0	0	-	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
All Pedestrians	-	-	19	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (4:45 PM)

### Turning Movement Peak Hour Data (7:15 AM)

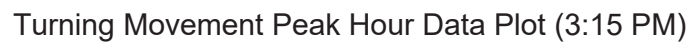
Start Time	Lehua St Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:15 AM	86	0	2	86	4	168	0	172	0	191	20	0	211	469
7:30 AM	95	1	0	96	1	163	0	164	0	170	15	0	185	445
7:45 AM	64	1	2	65	1	180	0	181	0	184	15	0	199	445
8:00 AM	45	0	4	45	2	169	0	171	1	189	18	0	208	424
Total	290	2	8	292	8	680	0	688	1	734	68	0	803	1783
Approach %	99.3	0.7	-	-	1.2	98.8	-	-	0.1	91.4	8.5	-	-	-
Total %	16.3	0.1	-	16.4	0.4	38.1	-	38.6	0.1	41.2	3.8	-	45.0	-
PHF	0.763	0.500	-	0.760	0.500	0.944	-	0.950	0.250	0.961	0.850	-	0.951	0.950
Lights	287	2	-	289	8	658	-	666	1	720	57	-	778	1733
% Lights	99.0	100.0	-	99.0	100.0	96.8	-	96.8	100.0	98.1	83.8	-	96.9	97.2
Mediums	3	0	-	3	0	20	-	20	0	13	10	-	23	46
% Mediums	1.0	0.0	-	1.0	0.0	2.9	-	2.9	0.0	1.8	14.7	-	2.9	2.6
Articulated Trucks	0	0	-	0	0	2	-	2	0	1	1	-	2	4
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.3	-	0.3	0.0	0.1	1.5	-	0.2	0.2
All Pedestrians	-	-	8	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (7:15 AM)

### Turning Movement Peak Hour Data (3:15 PM)

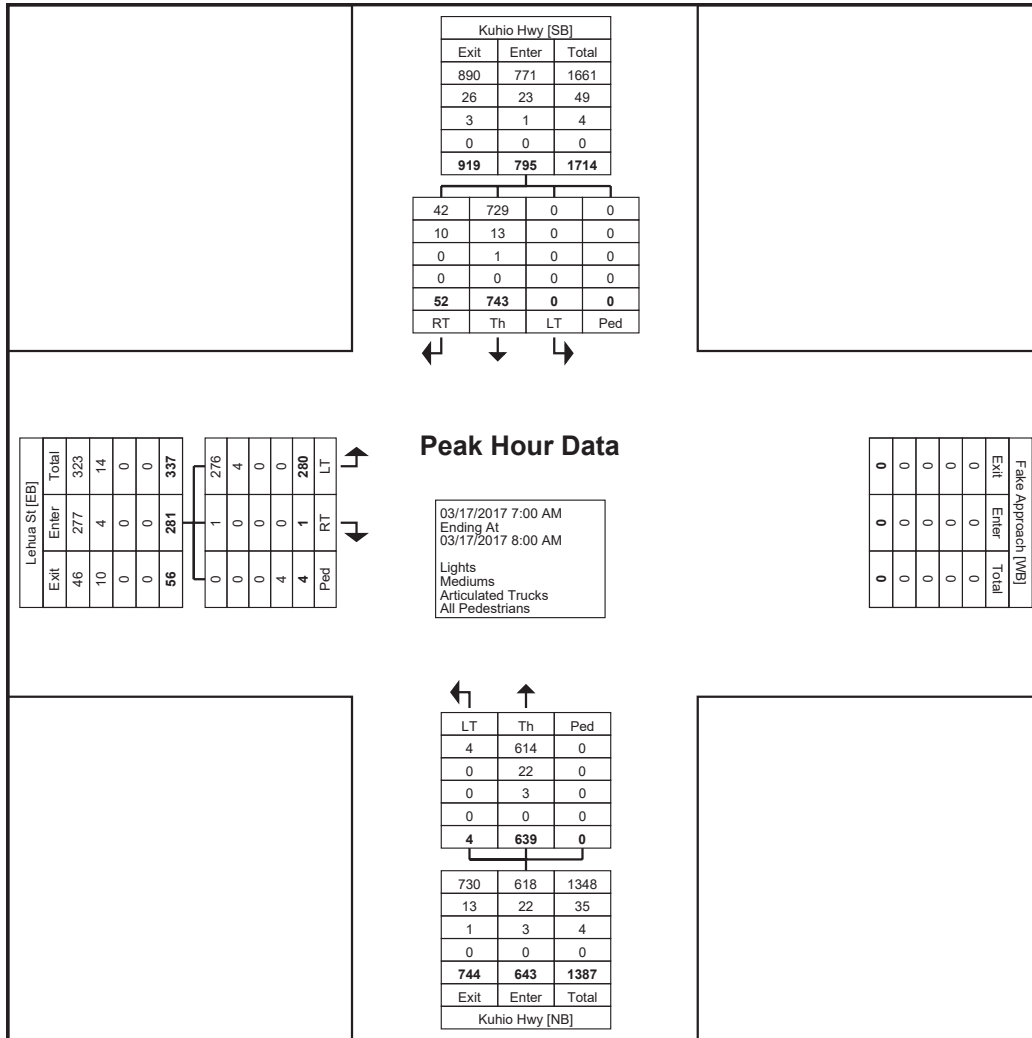
Start Time	Lehua St Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
3:15 PM	67	1	7	68	5	162	0	167	0	138	42	0	180	415
3:30 PM	71	4	4	75	6	145	0	151	0	155	55	0	210	436
3:45 PM	78	2	1	80	5	141	0	146	0	146	35	0	181	407
4:00 PM	71	0	0	71	4	148	0	152	0	163	51	0	214	437
Total	287	7	12	294	20	596	0	616	0	602	183	0	785	1695
Approach %	97.6	2.4	-	-	3.2	96.8	-	-	0.0	76.7	23.3	-	-	-
Total %	16.9	0.4	-	17.3	1.2	35.2	-	36.3	0.0	35.5	10.8	-	46.3	-
PHF	0.920	0.438	-	0.919	0.833	0.920	-	0.922	0.000	0.923	0.832	-	0.917	0.970
Lights	285	7	-	292	20	592	-	612	0	590	177	-	767	1671
% Lights	99.3	100.0	-	99.3	100.0	99.3	-	99.4	-	98.0	96.7	-	97.7	98.6
Mediums	2	0	-	2	0	4	-	4	0	11	6	-	17	23
% Mediums	0.7	0.0	-	0.7	0.0	0.7	-	0.6	-	1.8	3.3	-	2.2	1.4
Articulated Trucks	0	0	-	0	0	0	-	0	0	1	0	-	1	1
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.2	0.0	-	0.1	0.1
All Pedestrians	-	-	12	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-





### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Lehua St Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:00 AM	62	1	2	63	1	115	0	116	0	183	13	0	196	375
7:15 AM	62	0	1	62	0	170	0	170	0	188	12	0	200	432
7:30 AM	87	0	1	87	2	181	0	183	0	177	9	0	186	456
7:45 AM	69	0	0	69	1	173	0	174	0	195	18	0	213	456
Total	280	1	4	281	4	639	0	643	0	743	52	0	795	1719
Approach %	99.6	0.4	-	-	0.6	99.4	-	-	0.0	93.5	6.5	-	-	-
Total %	16.3	0.1	-	16.3	0.2	37.2	-	37.4	0.0	43.2	3.0	-	46.2	-
PHF	0.805	0.250	-	0.807	0.500	0.883	-	0.878	0.000	0.953	0.722	-	0.933	0.942
Lights	276	1	-	277	4	614	-	618	0	729	42	-	771	1666
% Lights	98.6	100.0	-	98.6	100.0	96.1	-	96.1	-	98.1	80.8	-	97.0	96.9
Mediums	4	0	-	4	0	22	-	22	0	13	10	-	23	49
% Mediums	1.4	0.0	-	1.4	0.0	3.4	-	3.4	-	1.7	19.2	-	2.9	2.9
Articulated Trucks	0	0	-	0	0	3	-	3	0	1	0	-	1	4
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.5	-	0.5	-	0.1	0.0	-	0.1	0.2
All Pedestrians	-	-	4	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



Study Name Kuhio Hwy Ulu St Ohia St 3-15-17 to 3-17-17  
Start Date 3/15/17  
Start Time 3:00 PM  
Site Code Hokua Place

	Ohia St Eastbound					Pono Kai Dwy Westbound					Kuhio Hwy Northbound					Kuhio Hwy Southbound					Ulu St Southeast Bound					Intersection		
Start	LT-Ulu	LT-Kuhio	Thru	RT-Kuhio	RT-Ulu	LT-Kuhio	LT-Ulu	Thru	RT-Ulu	RT-Kuhio	UT-Ulu	LT-Ohia	LT-Ulu	Thru	Right-Turn	Left-Turn	Thru	RT-Ulu	RT-Ohia	UT-Ulu	UT-Kuhio	LT-Dwy	RT-Kuhio	Thru	RT-Ohia	15-Min Totals	Hourly Totals	
3/15/17																												
3:00 PM	2	0	0	0	0	0	0	0	0	0	3	0	0	74	154	6	0	154	0	0	2	0	0	0	55	0	450	1699
3:15 PM	4	0	0	0	0	0	0	0	0	0	3	1	0	61	124	18	3	128	0	0	5	0	0	0	74	3	424	1656
3:30 PM	3	0	0	0	0	0	0	0	0	0	2	0	1	77	106	4	3	138	0	0	3	0	0	0	67	0	404	1632
3:45 PM	1	0	0	0	2	0	0	0	0	0	0	0	1	75	123	11	2	136	0	0	4	0	0	0	63	3	421	1640
4:00 PM	3	0	1	0	0	0	0	0	0	0	2	0	0	66	118	8	1	139	0	2	2	0	0	0	63	2	407	1673
4:15 PM	2	1	0	0	0	0	0	0	0	0	1	0	0	55	121	12	1	126	0	0	3	0	0	0	76	2	400	1674
4:30 PM	0	0	1	0	0	0	0	0	0	0	3	0	0	66	137	12	0	125	0	0	1	0	1	0	65	1	412	1715
4:45 PM	1	0	0	0	0	0	0	0	0	0	2	1	0	70	136	14	1	166	0	0	1	0	0	0	62	0	454	1734
5:00 PM	1	0	0	0	0	1	0	0	0	0	1	0	0	91	120	10	3	132	0	1	3	0	0	0	44	1	408	1670
5:15 PM	0	1	0	0	1	0	0	0	0	0	2	1	1	78	131	12	1	156	0	1	1	0	0	0	54	1	441	
5:30 PM	2	0	0	0	2	0	0	0	0	0	2	0	0	68	157	9	2	152	0	3	0	0	0	0	34	0	431	
5:45 PM	2	0	0	0	1	0	0	0	0	0	3	0	1	64	131	8	1	138	0	1	1	0	0	0	35	4	390	
3/16/17																												
7:00 AM	2	0	0	0	2	0	0	0	0	0	3	0	0	16	135	1	0	200	0	3	2	0	0	0	46	3	413	1664
7:15 AM	1	1	0	0	1	0	0	0	0	0	0	3	24	162	1	0	182	0	0	2	0	2	0	0	39	2	420	1673
7:30 AM	5	4	0	0	2	0	0	0	0	0	0	0	0	23	165	5	2	162	0	0	0	0	0	0	50	1	419	1683
7:45 AM	1	0	0	0	1	0	0	0	0	0	2	0	0	27	157	3	1	159	0	1	2	0	0	0	57	1	412	1710
8:00 AM	0	0	0	0	0	0	0	0	0	0	3	0	0	32	158	4	4	180	0	2	0	0	0	0	38	1	422	1743
8:15 AM	0	0	1	0	2	0	0	0	0	0	2	0	0	27	164	2	1	195	0	0	1	0	0	0	34	1	430	
8:30 AM	1	0	0	0	3	0	0	0	0	0	1	0	0	22	187	5	0	173	0	0	1	0	0	0	51	2	446	
8:45 AM	2	0	0	0	1	1	0	0	0	0	3	0	0	29	188	4	2	165	0	0	1	0	0	0	47	2	445	
3/16/17																												
3:00 PM	1	0	0	0	1	1	0	0	0	0	1	1	0	57	124	8	0	152	0	0	0	0	0	0	42	2	390	1643
3:15 PM	2	0	0	0	0	0	0	0	0	0	2	0	0	76	145	13	0	140	0	0	0	0	0	0	39	1	418	1705
3:30 PM	0	0	0	0	2	0	0	0	0	0	2	0	0	67	142	10	3	164	0	0	0	0	0	0	38	1	429	1688
3:45 PM	1	0	0	0	0	0	0	0	0	0	1	0	0	81	118	9	4	155	0	0	0	0	0	0	36	1	406	1732
4:00 PM	3	0	0	0	0	0	0	0	0	0	1	0	0	71	147	6	3	162	0	0	0	0	0	0	57	2	452	1743
4:15 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	72	137	10	0	131	0	0	2	0	0	0	48	0	401	1714
4:30 PM	2	0	0	0	0	0	0	0	0	0	3	0	0	95	163	3	3	152	0	0	1	0	0	0	50	1	473	1756
4:45 PM	0	0	0	0	0	1	0	0	0	0	2	0	0	76	153	5	1	139	0	0	1	0	0	0	39	0	417	1704
5:00 PM	2	1	0	0	0	0	0	0	0	0	4	0	0	60	141	6	1	162	0	0	0	0	0	0	46	0	423	1695
5:15 PM	2	0	0	0	0	1	0	0	0	0	2	2	2	71	140	6	1	159	0	1	2	0	0	0	53	1	443	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	58	163	5	1	150	0	0	0	0	0	0	43	0	421	
5:45 PM	1	0	0	0	1	0	0	0	0	0	2	0	1	53	139	5	1	156	0	0	1	0	0	0	47	1	408	
3/17/17																												
7:00 AM	2	1	0	0	1	0	0	0	0	0	1	0	1	19	123	0	1	185	0	2	0	0	0	0	48	2	386	1668
7:15 AM	1	1	0	0	0	0	0	0	0	0	0	0	0	25	182	2	3	172	0	1	0	1	0	0	45	1	434	1680
7:30 AM	5	1	0	0	1	0	0	0	0	0	0	0	0	21	162	2	2	176	0	1	0	0	0	0	50	0	421	1676
7:45 AM	4	1	0	0	0	1	0	0	0	0	2	0	0	25	145	3	1	172	0	0	3	0	0	0	67	3	427	1655
8:00 AM	0	1	0	0	0	0	0	0	0	0	0	1	0	24	148	4	1	170	0	1	1	0	0	0	45	2	398	1660
8:15 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	27	174	7	0	159	0	1	0	0	0	0	56	2	430	
8:30 AM	0	2	0	0	0	1	0	0	0	0	2	0	1	30	126	1	3	178	0	0	1	0	0	0	52	3	400	
8:45 AM	0	2	0	0	1	0	0	0	0	0	4	0	2	21	166	4	1	181	0	0	2	0	0	0	46	2	432	

AM Peak Hour Traffic		3/16/17																								
8:00 AM	3	0	1	0	6	1	0	0	0	9	0	0	110	697	15	7	713	0	2	3	0	0	0	170	6	1743
PHF	0.75	N/A	N/A	N/A	0.50	N/A	N/A	N/A	N/A	2.25	N/A	N/A	1.25	0.93	0.75	N/A	1.03	N/A	N/A	0.75	N/A	N/A	N/A	0.83	0.75	0.98
PHV	4	0	0	0	12	0	0	0	0	4	0	0	88	748	20	0	692	0	0	4	0	0	0	204	8	1784
T Factor	0%	N/A	0%	N/A	17%	0%	N/A	N/A	N/A	11%	N/A	N/A	5%	4%	0%	0%	3%	N/A	0%	0%	N/A	N/A	N/A	4%	17%	
													5	29	0		20									
														4%			3%									
PM Peak Hour Traffic		3/16/17																								
4:30 PM	6	1	0	0	2	0	0	0	0	11	2	2	302	597	20	6	612	0	1	4	0	0	0	188	2	1756
PHF	0.75	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.92	N/A	N/A	0.79	0.92	1.67	0.50	1.01	N/A	N/A	1.00	N/A	N/A	N/A	0.94	0.50	0.93
PHV	8	0	0	0	0	0	0	0	0	12	0	0	380	652	12	12	608	0	0	4	0	0	0	200	4	1892
T Factor	0%	0%	N/A	N/A	N/A	0%	N/A	N/A	N/A	0%	0%	0%	1%	1%	0%	0%	0%	N/A	0%	0%	N/A	N/A	N/A	1%	0%	

**Study Name Olohena Rd Lehua St Kahau St 3-13-17 to 3-15-17**

**Start Date 03/13/2017**

**Start Time 3:00 PM**

**Site Code Hokua Place**

Start	Olohena Rd Eastbound			Olohena Rd Westbound			Kahau St Southbound			Lehua St Southwest Bound			Intersection	
3/13/17	LT-Kahau	LT-Lehua	Thru	Thru	RT-Kahau	RT-Lehua	UT-Lehua	LT-Olohena	Right-Turn	LT-Olohena	RT-Olohena	UT-Kahau	15-Min Totals	Hourly Totals
3:00 PM	12	55	88	79	3	12	9	8	16	8	38	9	337	1326
3:15 PM	12	90	70	52	9	10	3	9	6	18	44	11	334	1308
3:30 PM	9	80	53	86	7	21	2	2	6	12	51	8	337	1277
3:45 PM	7	60	63	86	7	13	4	3	5	12	51	7	318	1212
4:00 PM	18	68	56	77	8	11	5	8	4	8	42	14	319	1177
4:15 PM	22	59	42	82	9	12	0	4	3	8	43	19	303	
4:30 PM	19	49	65	72	3	7	4	6	3	8	33	3	272	
4:45 PM	9	71	38	81	7	11	2	7	6	11	33	7	283	
3/14/17														
6:30 AM	3	23	36	15	2	0	2	0	0	2	5	4	92	573
6:45 AM	2	40	41	13	7	0	5	4	0	0	10	4	126	698
7:00 AM	4	52	46	25	2	1	1	2	4	1	8	3	149	787
7:15 AM	6	84	57	29	5	3	0	5	1	3	12	1	206	818
7:30 AM	4	81	58	35	0	3	11	3	6	2	13	1	217	762
7:45 AM	4	66	94	28	2	1	0	3	3	0	13	1	215	
8:00 AM	2	38	81	28	1	3	3	2	2	0	18	2	180	
8:15 AM	8	35	49	31	3	4	1	2	5	1	8	3	150	
3/14/17														
3:30 PM	8	61	57	28	4	12	5	5	3	6	25	13	227	1158
3:45 PM	14	66	53	56	8	11	2	7	3	12	40	7	279	1229
4:00 PM	24	78	44	70	10	19	1	9	5	15	46	6	327	1223
4:15 PM	14	55	50	84	10	13	3	12	3	12	54	15	325	1149
4:30 PM	14	81	53	61	3	10	1	3	4	5	56	7	298	1079
4:45 PM	14	66	56	76	3	8	1	7	4	5	26	7	273	
5:00 PM	10	65	45	72	5	8	0	3	4	0	37	4	253	
5:15 PM	9	76	37	71	6	9	0	2	5	8	30	2	255	
3/15/17														
6:30 AM	4	18	27	9	5	0	2	2	1	2	6	6	82	602
6:45 AM	4	48	54	23	8	0	2	7	2	1	7	6	162	748
7:00 AM	1	53	54	23	1	1	1	3	1	0	9	1	148	829
7:15 AM	7	81	59	16	1	4	9	4	8	2	17	2	210	839
7:30 AM	2	95	59	35	5	6	0	2	5	1	18	0	228	793
7:45 AM	2	90	89	27	2	3	2	1	6	0	20	1	243	
8:00 AM	4	36	58	29	2	2	2	1	2	4	17	1	158	
8:15 AM	1	42	62	21	3	1	0	4	7	4	17	2	164	

**AM Peak Hour Traffic**

3/15/17

7:15 AM	15	302	265	107	10	15	13	8	21	7	72	4	839
PHF	1.88	0.84	0.74	0.99	1.25	1.25	1.63	2.00	0.88	N/A	0.90	1.00	0.86
Peak Flow	8	360	356	108	8	12	8	4	24	7	80	4	972
T Factor	0%	1%	4%	3%	10%	0%	8%	50%	19%	0%	7%	25%	

**PM Peak Hour Traffic**

3/13/17

3:00 PM	40	285	274	303	26	56	18	22	33	50	184	35	1326
PHF	1.11	0.89	1.29	0.88	0.93	0.67	2.25	2.75	1.38	1.04	0.90	1.09	0.98
Peak Flow	36	320	212	344	28	84	8	8	24	48	204	32	1348
T Factor	13%	3%	4%	1%	0%	2%	0%	0%	6%	2%	2%	0%	

The Traffic Management Consultant  
1188 Bishop Street, Suite 1907

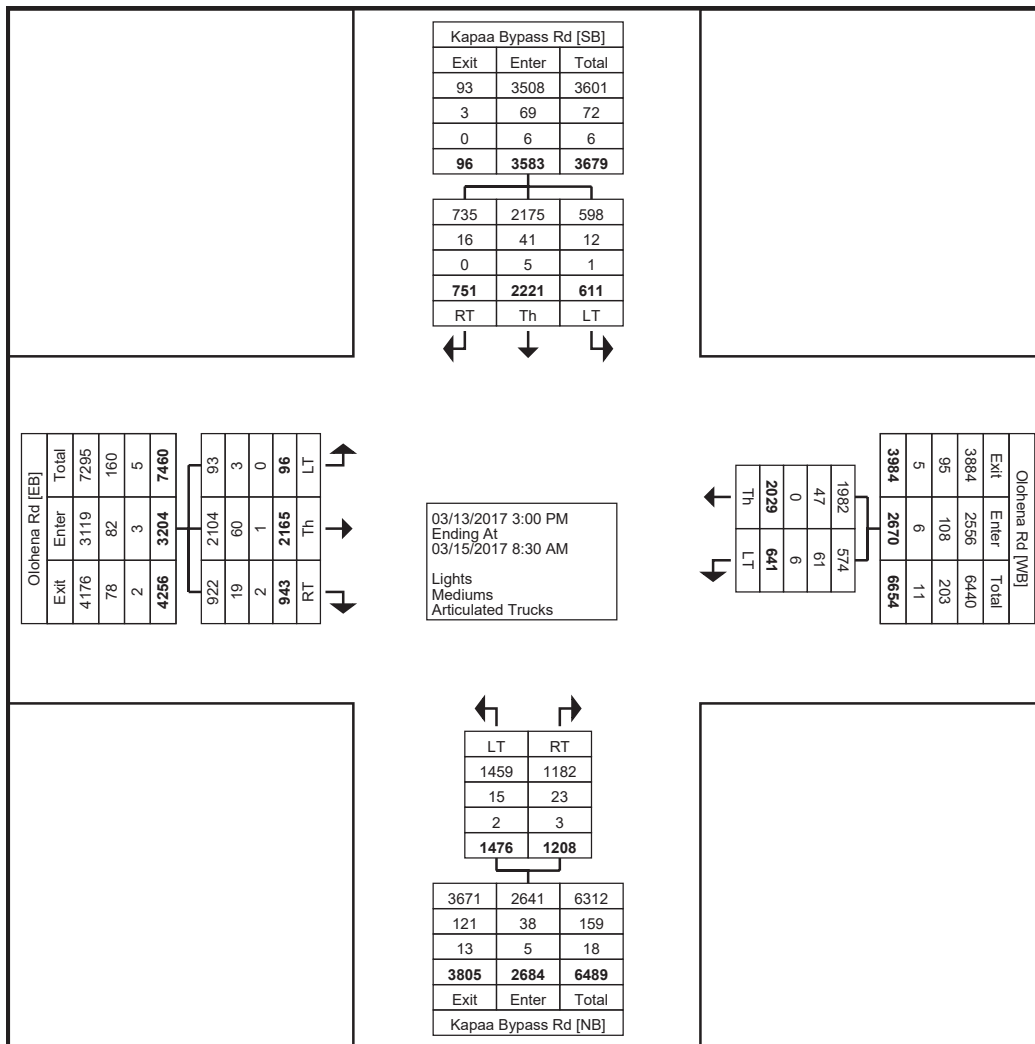
Honolulu, Hawaii, United States 96813  
808-536-0223 tmchawaii@aol.com

Count Name: Olohena Rd  
Kapaa Bypass 3-13-17 to 3-15-17

Site Code: Hokua Place  
Start Date: 03/13/2017  
Page No: 1

## Turning Movement Data

Start Time	Olohena Rd Eastbound				Olohena Rd Westbound			Kapaa Bypass Rd Northbound			Kapaa Bypass Rd Southbound				Int. Total
	Left-Turn	Thru	Right-Turn	App. Total	Left-Turn	Thru	App. Total	Left-Turn	Right-Turn	App. Total	Left-Turn	Thru	Right-Turn	App. Total	
3:00 PM	2	106	33	141	33	102	135	64	38	102	21	32	10	63	441
3:15 PM	4	65	17	86	29	73	102	67	65	132	33	63	28	124	444
3:30 PM	4	50	18	72	42	98	140	57	72	129	14	69	19	102	443
3:45 PM	1	49	11	61	32	118	150	77	57	134	19	46	29	94	439
Hourly Total	11	270	79	360	136	391	527	265	232	497	87	210	86	383	1767
4:00 PM	1	47	17	65	24	114	138	82	78	160	15	66	17	98	461
4:15 PM	2	48	17	67	10	126	136	83	64	147	16	61	10	87	437
4:30 PM	2	35	11	48	16	105	121	89	76	165	13	57	19	89	423
4:45 PM	3	40	12	55	17	109	126	80	64	144	11	41	19	71	396
Hourly Total	8	170	57	235	67	454	521	334	282	616	55	225	65	345	1717
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6:30 AM	0	46	37	83	7	16	23	5	4	9	10	40	4	54	169
6:45 AM	3	61	45	109	7	17	24	3	5	8	20	66	9	95	236
Hourly Total	3	107	82	192	14	33	47	8	9	17	30	106	13	149	405
7:00 AM	3	86	76	165	11	28	39	10	2	12	14	96	24	134	350
7:15 AM	3	107	59	169	10	33	43	23	12	35	30	119	31	180	427
7:30 AM	7	116	40	163	10	47	57	25	9	34	30	95	56	181	435
7:45 AM	8	112	34	154	10	35	45	26	13	39	36	101	41	178	416
Hourly Total	21	421	209	651	41	143	184	84	36	120	110	411	152	673	1628
8:00 AM	2	77	27	106	13	32	45	10	11	21	24	91	22	137	309
8:15 AM	1	65	21	87	12	32	44	12	5	17	20	68	14	102	250
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	3	142	48	193	25	64	89	22	16	38	44	159	36	239	559
3:30 PM	2	42	19	63	18	37	55	41	61	102	14	48	19	81	301
3:45 PM	3	46	6	55	37	86	123	93	84	177	14	46	20	80	435
Hourly Total	5	88	25	118	55	123	178	134	145	279	28	94	39	161	736
4:00 PM	2	38	17	57	36	104	140	66	74	140	22	70	44	136	473
4:15 PM	2	51	15	68	42	104	146	68	58	126	11	62	23	96	436
4:30 PM	4	38	12	54	43	85	128	77	76	153	22	65	21	108	443
4:45 PM	4	47	15	66	31	84	115	98	77	175	11	42	20	73	429
Hourly Total	12	174	59	245	152	377	529	309	285	594	66	239	108	413	1781
5:00 PM	5	41	18	64	21	106	127	98	69	167	15	55	28	98	456
5:15 PM	2	36	12	50	29	92	121	96	82	178	10	43	24	77	426
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	7	77	30	114	50	198	248	194	151	345	25	98	52	175	882
6:30 AM	0	38	43	81	8	9	17	3	1	4	9	51	6	66	168
6:45 AM	2	79	56	137	9	26	35	10	7	17	18	58	10	86	275
Hourly Total	2	117	99	218	17	35	52	13	8	21	27	109	16	152	443
7:00 AM	2	90	59	151	10	26	36	10	2	12	13	98	16	127	326
7:15 AM	1	108	62	171	17	24	41	26	6	32	24	107	45	176	420
7:30 AM	4	116	34	154	16	47	63	27	8	35	32	111	50	193	445
7:45 AM	10	126	45	181	13	45	58	25	12	37	39	83	47	169	445
Hourly Total	17	440	200	657	56	142	198	88	28	116	108	399	158	665	1636
8:00 AM	3	73	34	110	13	39	52	16	8	24	16	99	16	131	317
8:15 AM	4	86	21	111	15	30	45	9	8	17	15	72	10	97	270
Grand Total	96	2165	943	3204	641	2029	2670	1476	1208	2684	611	2221	751	3583	12141
Approach %	3.0	67.6	29.4	-	24.0	76.0	-	55.0	45.0	-	17.1	62.0	21.0	-	-
Total %	0.8	17.8	7.8	26.4	5.3	16.7	22.0	12.2	9.9	22.1	5.0	18.3	6.2	29.5	-
Lights	93	2104	922	3119	574	1982	2556	1459	1182	2641	598	2175	735	3508	11824
% Lights	96.9	97.2	97.8	97.3	89.5	97.7	95.7	98.8	97.8	98.4	97.9	97.9	97.9	97.9	97.4
Mediums	3	60	19	82	61	47	108	15	23	38	12	41	16	69	297
% Mediums	3.1	2.8	2.0	2.6	9.5	2.3	4.0	1.0	1.9	1.4	2.0	1.8	2.1	1.9	2.4
Articulated Trucks	0	1	2	3	6	0	6	2	3	5	1	5	0	6	20
% Articulated Trucks	0.0	0.0	0.2	0.1	0.9	0.0	0.2	0.1	0.2	0.2	0.2	0.2	0.0	0.2	0.2



Turning Movement Data Plot

The Traffic Management Consultant  
1188 Bishop Street, Suite 1907

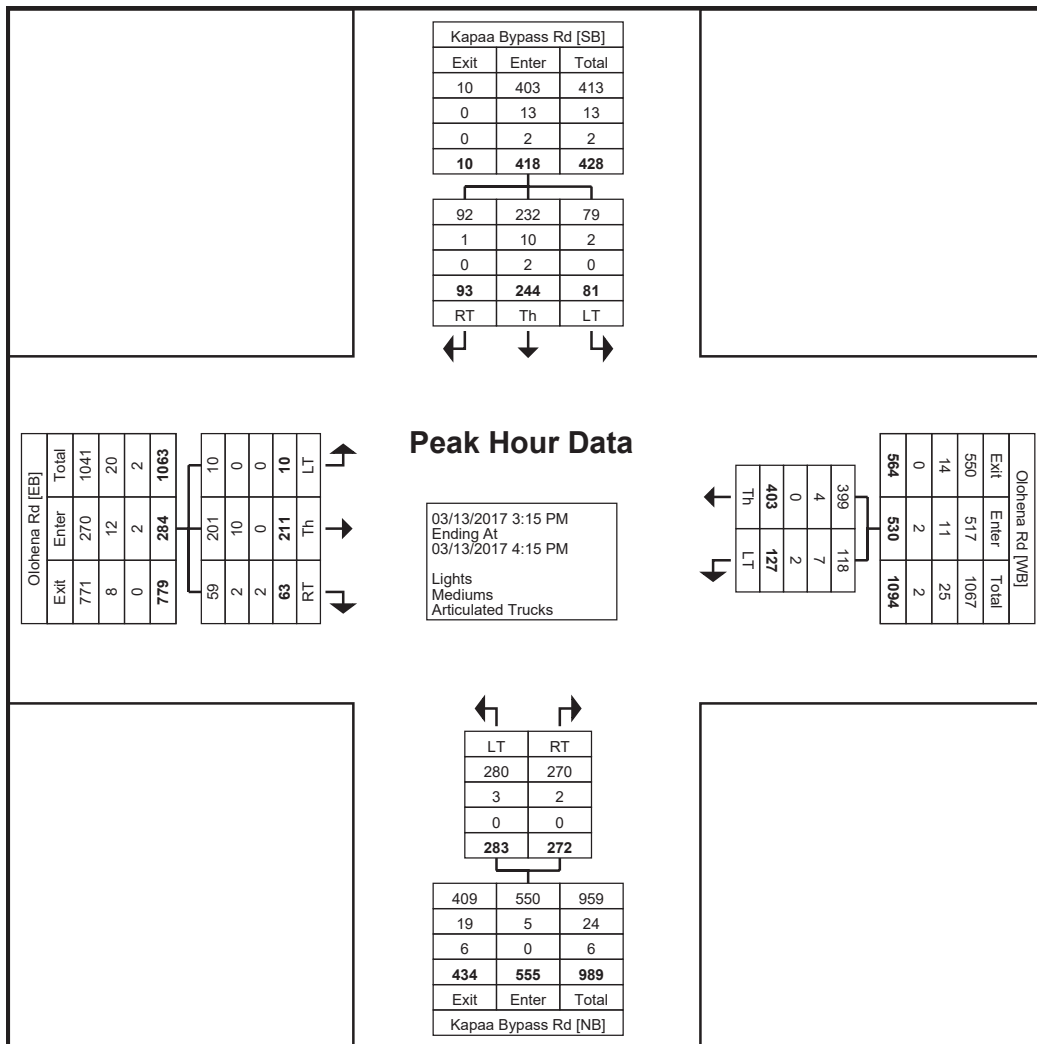
Honolulu, Hawaii, United States 96813  
808-536-0223 tmchawaii@aol.com

Count Name: Oloheua Rd  
Kapaa Bypass 3-13-17 to 3-15-17

Site Code: Hokua Place  
Start Date: 03/13/2017  
Page No: 3

### Turning Movement Peak Hour Data (3:15 PM)

Start Time	Oloheua Rd Eastbound				Oloheua Rd Westbound			Kapaa Bypass Rd Northbound			Kapaa Bypass Rd Southbound				Int. Total
	Left-Turn	Thru	Right-Turn	App. Total	Left-Turn	Thru	App. Total	Left-Turn	Right-Turn	App. Total	Left-Turn	Thru	Right-Turn	App. Total	
3:15 PM	4	65	17	86	29	73	102	67	65	132	33	63	28	124	444
3:30 PM	4	50	18	72	42	98	140	57	72	129	14	69	19	102	443
3:45 PM	1	49	11	61	32	118	150	77	57	134	19	46	29	94	439
4:00 PM	1	47	17	65	24	114	138	82	78	160	15	66	17	98	461
Total	10	211	63	284	127	403	530	283	272	555	81	244	93	418	1787
Approach %	3.5	74.3	22.2	-	24.0	76.0	-	51.0	49.0	-	19.4	58.4	22.2	-	-
Total %	0.6	11.8	3.5	15.9	7.1	22.6	29.7	15.8	15.2	31.1	4.5	13.7	5.2	23.4	-
PHF	0.625	0.812	0.875	0.826	0.756	0.854	0.883	0.863	0.872	0.867	0.614	0.884	0.802	0.843	0.969
Lights	10	201	59	270	118	399	517	280	270	550	79	232	92	403	1740
% Lights	100.0	95.3	93.7	95.1	92.9	99.0	97.5	98.9	99.3	99.1	97.5	95.1	98.9	96.4	97.4
Mediums	0	10	2	12	7	4	11	3	2	5	2	10	1	13	41
% Mediums	0.0	4.7	3.2	4.2	5.5	1.0	2.1	1.1	0.7	0.9	2.5	4.1	1.1	3.1	2.3
Articulated Trucks	0	0	2	2	2	0	2	0	0	0	0	2	0	2	6
% Articulated Trucks	0.0	0.0	3.2	0.7	1.6	0.0	0.4	0.0	0.0	0.0	0.0	0.8	0.0	0.5	0.3



Turning Movement Peak Hour Data Plot (3:15 PM)



The Traffic Management Consultant  
1188 Bishop Street, Suite 1907

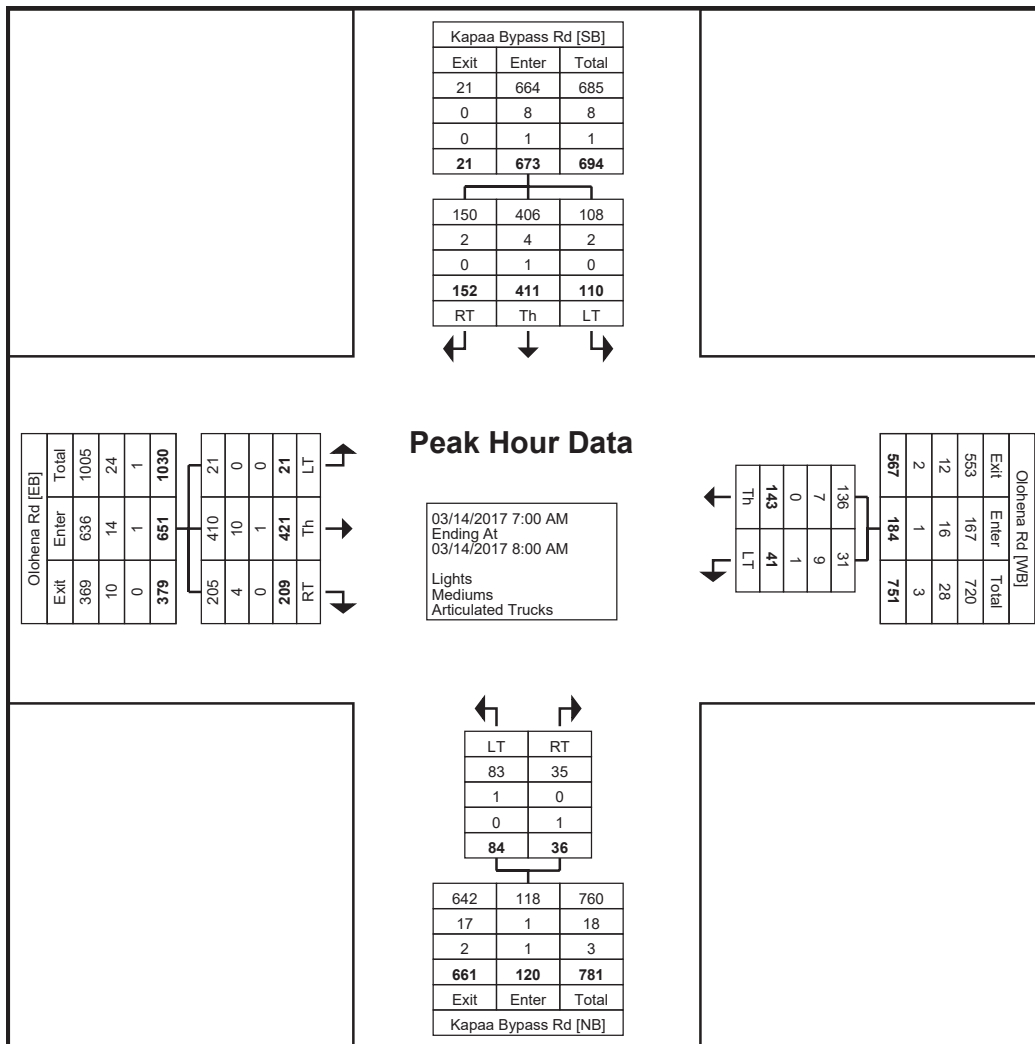
Honolulu, Hawaii, United States 96813  
808-536-0223 tmchawaii@aol.com

Count Name: Oloheua Rd  
Kapaa Bypass 3-13-17 to 3-15-17

Site Code: Hokua Place  
Start Date: 03/13/2017  
Page No: 5

### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Oloheua Rd Eastbound				Oloheua Rd Westbound			Kapaa Bypass Rd Northbound			Kapaa Bypass Rd Southbound				Int. Total
	Left-Turn	Thru	Right-Turn	App. Total	Left-Turn	Thru	App. Total	Left-Turn	Right-Turn	App. Total	Left-Turn	Thru	Right-Turn	App. Total	
7:00 AM	3	86	76	165	11	28	39	10	2	12	14	96	24	134	350
7:15 AM	3	107	59	169	10	33	43	23	12	35	30	119	31	180	427
7:30 AM	7	116	40	163	10	47	57	25	9	34	30	95	56	181	435
7:45 AM	8	112	34	154	10	35	45	26	13	39	36	101	41	178	416
Total	21	421	209	651	41	143	184	84	36	120	110	411	152	673	1628
Approach %	3.2	64.7	32.1	-	22.3	77.7	-	70.0	30.0	-	16.3	61.1	22.6	-	-
Total %	1.3	25.9	12.8	40.0	2.5	8.8	11.3	5.2	2.2	7.4	6.8	25.2	9.3	41.3	-
PHF	0.656	0.907	0.688	0.963	0.932	0.761	0.807	0.808	0.692	0.769	0.764	0.863	0.679	0.930	0.936
Lights	21	410	205	636	31	136	167	83	35	118	108	406	150	664	1585
% Lights	100.0	97.4	98.1	97.7	75.6	95.1	90.8	98.8	97.2	98.3	98.2	98.8	98.7	98.7	97.4
Mediums	0	10	4	14	9	7	16	1	0	1	2	4	2	8	39
% Mediums	0.0	2.4	1.9	2.2	22.0	4.9	8.7	1.2	0.0	0.8	1.8	1.0	1.3	1.2	2.4
Articulated Trucks	0	1	0	1	1	0	1	0	1	1	0	1	0	1	4
% Articulated Trucks	0.0	0.2	0.0	0.2	2.4	0.0	0.5	0.0	2.8	0.8	0.0	0.2	0.0	0.1	0.2



Turning Movement Peak Hour Data Plot (7:00 AM)

The Traffic Management Consultant  
1188 Bishop Street, Suite 1907

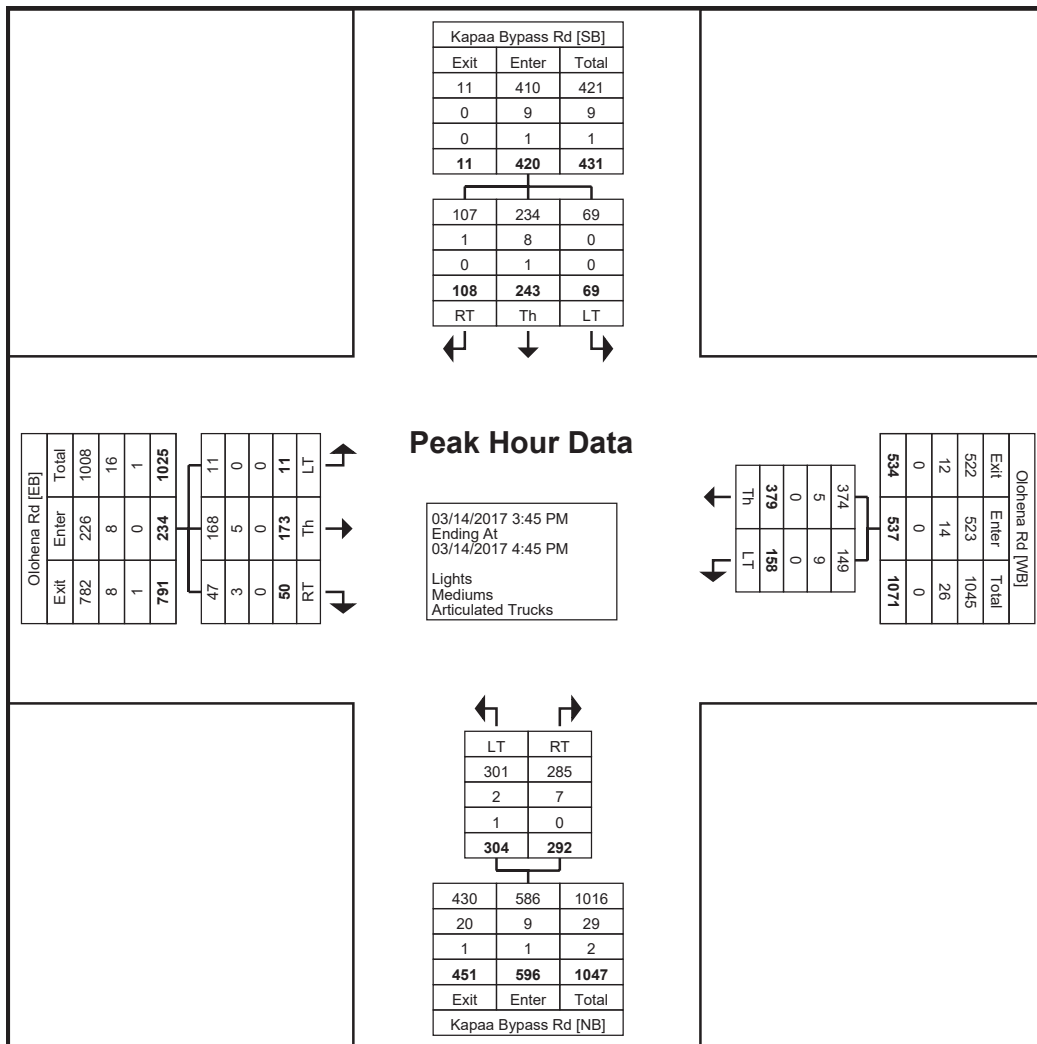
Honolulu, Hawaii, United States 96813  
808-536-0223 tmchawaii@aol.com

Count Name: Olohehena Rd  
Kapaa Bypass 3-13-17 to 3-15-17

Site Code: Hokua Place  
Start Date: 03/13/2017  
Page No: 7

### Turning Movement Peak Hour Data (3:45 PM)

Start Time	Olohehena Rd Eastbound				Olohehena Rd Westbound			Kapaa Bypass Rd Northbound			Kapaa Bypass Rd Southbound				Int. Total
	Left-Turn	Thru	Right-Turn	App. Total	Left-Turn	Thru	App. Total	Left-Turn	Right-Turn	App. Total	Left-Turn	Thru	Right-Turn	App. Total	
3:45 PM	3	46	6	55	37	86	123	93	84	177	14	46	20	80	435
4:00 PM	2	38	17	57	36	104	140	66	74	140	22	70	44	136	473
4:15 PM	2	51	15	68	42	104	146	68	58	126	11	62	23	96	436
4:30 PM	4	38	12	54	43	85	128	77	76	153	22	65	21	108	443
Total	11	173	50	234	158	379	537	304	292	596	69	243	108	420	1787
Approach %	4.7	73.9	21.4	-	29.4	70.6	-	51.0	49.0	-	16.4	57.9	25.7	-	-
Total %	0.6	9.7	2.8	13.1	8.8	21.2	30.1	17.0	16.3	33.4	3.9	13.6	6.0	23.5	-
PHF	0.688	0.848	0.735	0.860	0.919	0.911	0.920	0.817	0.869	0.842	0.784	0.868	0.614	0.772	0.945
Lights	11	168	47	226	149	374	523	301	285	586	69	234	107	410	1745
% Lights	100.0	97.1	94.0	96.6	94.3	98.7	97.4	99.0	97.6	98.3	100.0	96.3	99.1	97.6	97.6
Mediums	0	5	3	8	9	5	14	2	7	9	0	8	1	9	40
% Mediums	0.0	2.9	6.0	3.4	5.7	1.3	2.6	0.7	2.4	1.5	0.0	3.3	0.9	2.1	2.2
Articulated Trucks	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2
% Articulated Trucks	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0	0.2	0.0	0.4	0.0	0.2	0.1



Turning Movement Peak Hour Data Plot (3:45 PM)

The Traffic Management Consultant  
1188 Bishop Street, Suite 1907

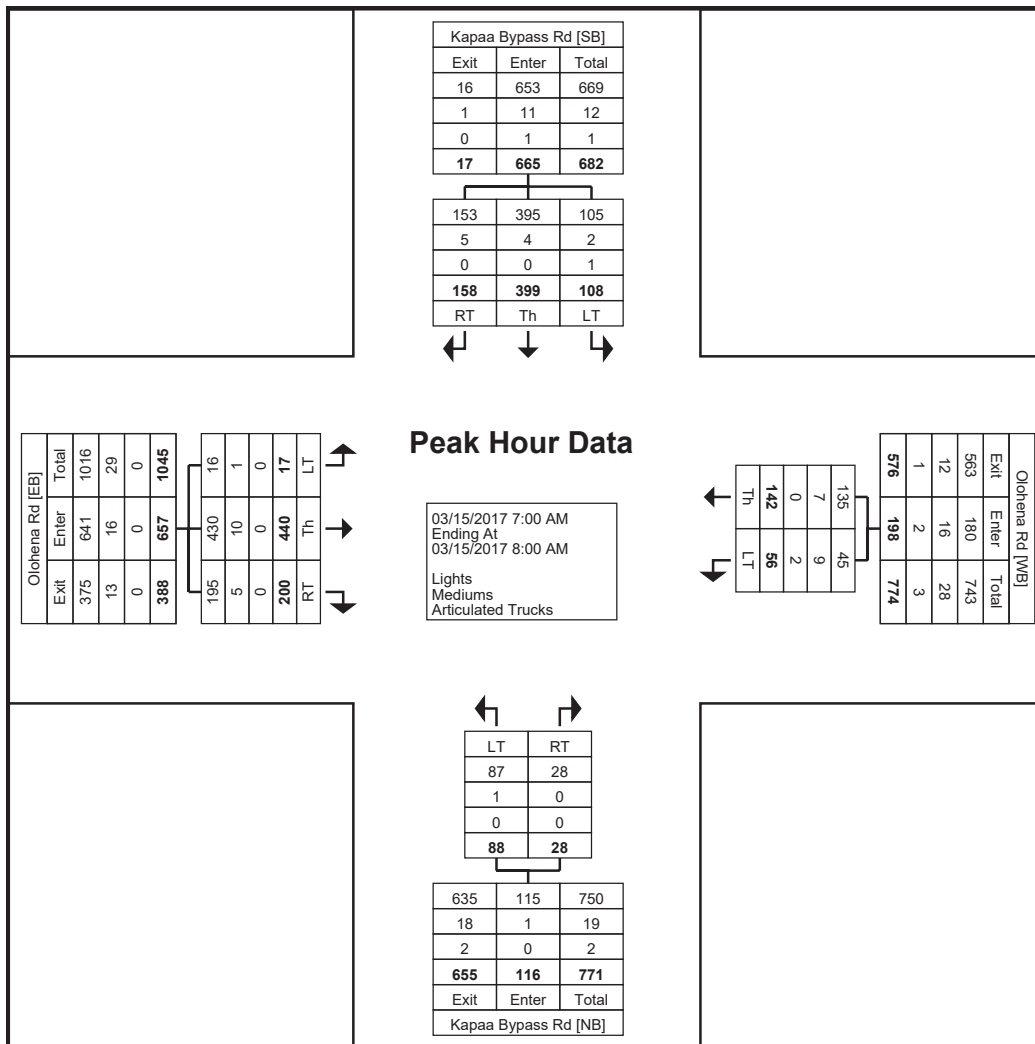
Honolulu, Hawaii, United States 96813  
808-536-0223 tmchawaii@aol.com

Count Name: Olohehena Rd  
Kapaa Bypass 3-13-17 to 3-15-17

Site Code: Hokua Place  
Start Date: 03/13/2017  
Page No: 9

### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Olohehena Rd Eastbound				Olohehena Rd Westbound			Kapaa Bypass Rd Northbound			Kapaa Bypass Rd Southbound				Int. Total
	Left-Turn	Thru	Right-Turn	App. Total	Left-Turn	Thru	App. Total	Left-Turn	Right-Turn	App. Total	Left-Turn	Thru	Right-Turn	App. Total	
7:00 AM	2	90	59	151	10	26	36	10	2	12	13	98	16	127	326
7:15 AM	1	108	62	171	17	24	41	26	6	32	24	107	45	176	420
7:30 AM	4	116	34	154	16	47	63	27	8	35	32	111	50	193	445
7:45 AM	10	126	45	181	13	45	58	25	12	37	39	83	47	169	445
Total	17	440	200	657	56	142	198	88	28	116	108	399	158	665	1636
Approach %	2.6	67.0	30.4	-	28.3	71.7	-	75.9	24.1	-	16.2	60.0	23.8	-	-
Total %	1.0	26.9	12.2	40.2	3.4	8.7	12.1	5.4	1.7	7.1	6.6	24.4	9.7	40.6	-
PHF	0.425	0.873	0.806	0.907	0.824	0.755	0.786	0.815	0.583	0.784	0.692	0.899	0.790	0.861	0.919
Lights	16	430	195	641	45	135	180	87	28	115	105	395	153	653	1589
% Lights	94.1	97.7	97.5	97.6	80.4	95.1	90.9	98.9	100.0	99.1	97.2	99.0	96.8	98.2	97.1
Mediums	1	10	5	16	9	7	16	1	0	1	2	4	5	11	44
% Mediums	5.9	2.3	2.5	2.4	16.1	4.9	8.1	1.1	0.0	0.9	1.9	1.0	3.2	1.7	2.7
Articulated Trucks	0	0	0	0	2	0	2	0	0	0	1	0	0	1	3
% Articulated Trucks	0.0	0.0	0.0	0.0	3.6	0.0	1.0	0.0	0.0	0.0	0.9	0.0	0.0	0.2	0.2



Turning Movement Peak Hour Data Plot (7:00 AM)

**Study Name Olohehena Rd Kaapuni Rd Kaehulu Rd 3-13-17 to 3-15-17**

**Start Date 03/13/2017**

**Start Time 3:30 PM**

**Site Code Hokua Place**

Start	Olohehena Rd Eastbound			Olohehena Rd Westbound			Kaehulua Rd Southbound			Kaapuni Rd Southeast Bound			Intersection	
	LT-Kaapuni	LT-Kaehulu	Thru	Thru	RT-Kaapuni	RT-Kaehulu	LT-Olohehena	RT-Olohehena	RT-Kaapuni	LT-Kaehulu	LT-Olohehena	RT-Olohehena	15-Min Totals	Hourly Totals
3/13/17														
3:30 PM	4	5	33	38	70	3	0	0	0	0	28	5	186	811
3:45 PM	1	0	28	51	80	6	2	0	0	0	28	8	204	802
4:00 PM	4	0	37	54	84	3	2	0	0	0	21	5	210	797
4:15 PM	7	2	24	50	77	9	1	0	0	0	36	5	211	775
4:30 PM	6	0	18	49	64	5	0	0	1	0	25	9	177	744
4:45 PM	8	4	23	51	72	4	0	4	0	0	26	7	199	
5:00 PM	4	2	36	44	62	1	0	1	0	0	30	8	188	
5:15 PM	5	2	20	53	65	6	2	0	1	0	19	7	180	
3/14/17														
6:30 AM	4	1	36	10	6	0	2	2	0	0	52	5	118	702
6:45 AM	4	1	61	7	7	0	0	0	0	1	65	2	148	789
7:00 AM	3	1	79	14	15	2	2	0	0	1	78	3	198	835
7:15 AM	11	6	104	10	24	2	3	0	0	2	72	4	238	782
7:30 AM	3	9	86	17	28	4	1	1	0	5	45	6	205	674
7:45 AM	5	6	53	24	32	2	2	0	0	0	60	10	194	
8:00 AM	5	4	37	30	13	2	4	1	0	1	45	3	145	
8:15 AM	5	2	45	16	17	1	1	0	0	0	40	3	130	
3/14/17														
3:30 PM	6	2	30	33	35	2	0	3	1	2	42	4	160	800
3:45 PM	9	2	22	51	60	3	0	3	2	0	26	11	189	833
4:00 PM	8	5	29	69	64	6	1	2	0	1	29	15	229	858
4:15 PM	5	4	26	68	68	2	4	0	0	0	31	14	222	855
4:30 PM	10	0	19	60	63	3	0	2	1	2	24	9	193	834
4:45 PM	11	4	27	38	85	4	2	0	0	1	31	11	214	
5:00 PM	9	0	32	58	81	6	5	1	1	0	30	3	226	
5:15 PM	5	4	17	58	73	4	3	3	3	0	26	5	201	
3/15/17														
6:30 AM	2	1	33	9	2	0	2	3	0	0	49	2	103	658
6:45 AM	2	0	64	8	12	0	1	1	0	0	64	1	153	763
7:00 AM	2	2	83	11	9	2	2	0	0	0	73	9	193	824
7:15 AM	8	7	97	9	20	1	2	0	0	0	61	4	209	761
7:30 AM	10	7	82	21	23	4	1	0	0	0	54	6	208	699
7:45 AM	11	3	65	22	33	5	3	0	0	0	60	12	214	
8:00 AM	3	1	44	20	14	5	0	1	0	0	40	2	130	
8:15 AM	7	0	48	19	13	1	1	1	0	0	51	6	147	

**AM Peak Hour Traffic**

			3/14/17											
7:00 AM	22	22	322	65	99	10	8	1	0	8	255	23	835	
PHF	0.50	0.92	0.77	1.63	1.03	1.25	0.67	N/A	N/A	1.00	0.89	1.44	0.88	
PHV	44	24	416	40	96	8	12	1	0	8	288	16	952	
T Factor	9%	0%	1%	0%	2%	0%	0%	0%	N/A	0%	2%	0%		

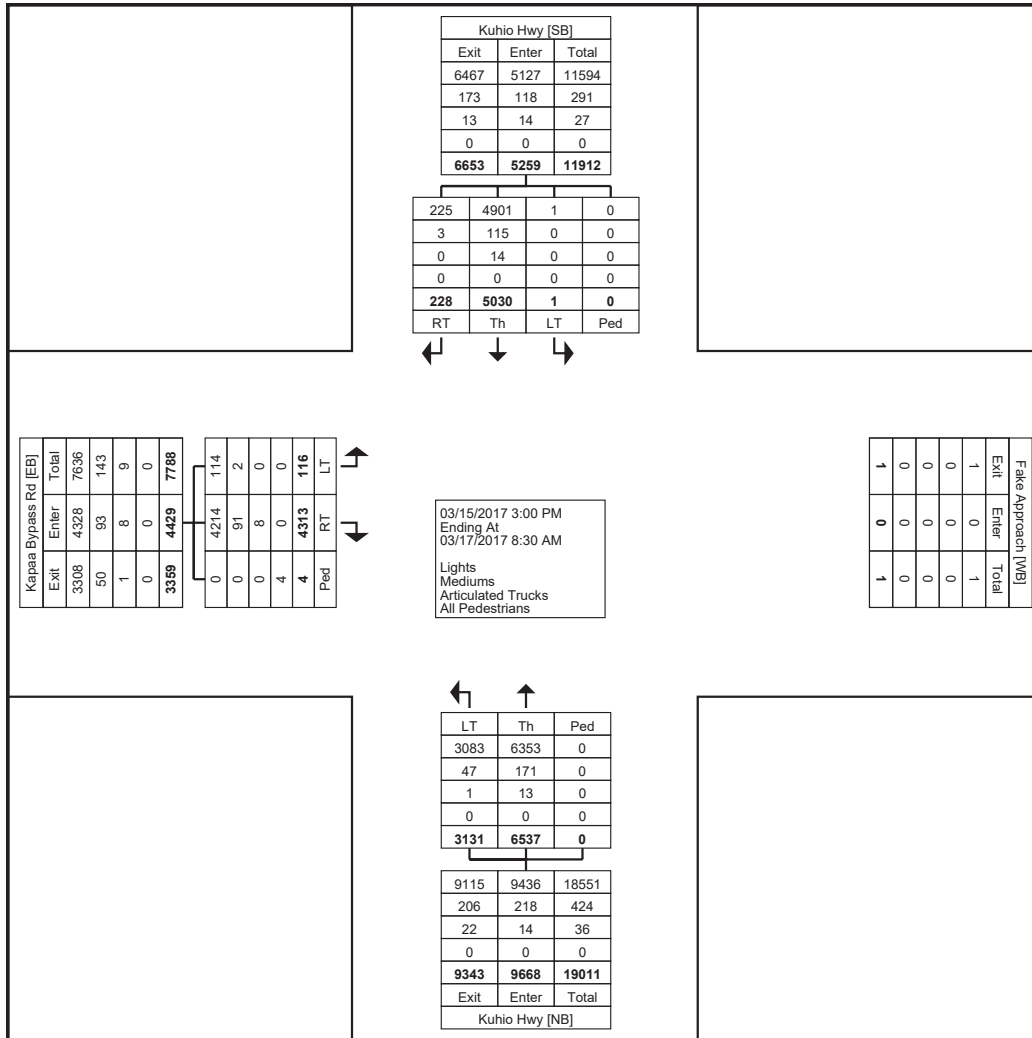
**PM Peak Hour Traffic**

			3/14/17											
4:00 PM	34	13	101	235	280	15	7	4	1	4	115	49	858	
PHF	1.06	0.65	0.87	0.85	1.09	0.63	1.75	0.50	N/A	1.00	0.99	0.82	0.94	
PHV	32	20	116	276	256	24	4	8	1	4	116	60	916	
T Factor	0%	0%	2%	0%	1%	0%	0%	0%	0%	0%	2%	0%		

## Turning Movement Data

Start Time	Kapaa Bypass Rd Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
3:00 PM	1	105	0	106	99	191	0	290	0	106	5	0	111	507
3:15 PM	3	100	0	103	122	210	0	332	0	88	7	0	95	530
3:30 PM	8	93	0	101	120	207	0	327	0	73	8	0	81	509
3:45 PM	8	104	0	112	148	201	0	349	0	88	21	0	109	570
Hourly Total	20	402	0	422	489	809	0	1298	0	355	41	0	396	2116
4:00 PM	1	108	0	109	168	161	0	329	0	91	16	0	107	545
4:15 PM	9	94	0	103	154	172	0	326	0	97	14	0	111	540
4:30 PM	6	90	0	96	166	187	0	353	0	112	19	0	131	580
4:45 PM	2	95	0	97	146	176	0	322	0	112	15	0	127	546
Hourly Total	18	387	0	405	634	696	0	1330	0	412	64	0	476	2211
5:00 PM	5	88	0	93	149	232	0	381	0	138	27	0	165	639
5:15 PM	2	91	0	93	149	192	0	341	0	152	25	0	177	611
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	7	179	0	186	298	424	0	722	0	290	52	0	342	1250
6:30 AM	0	78	0	78	14	124	0	138	0	203	0	0	203	419
6:45 AM	2	116	0	118	8	124	0	132	0	190	1	0	191	441
Hourly Total	2	194	0	196	22	248	0	270	0	393	1	0	394	860
7:00 AM	1	161	0	162	20	129	0	149	0	233	0	0	233	544
7:15 AM	1	184	0	185	25	155	0	180	0	200	1	0	201	566
7:30 AM	2	152	0	154	24	152	0	176	0	167	0	0	167	497
7:45 AM	1	155	1	156	33	180	0	213	0	135	0	0	135	504
Hourly Total	5	652	1	657	102	616	0	718	0	735	1	0	736	2111
8:00 AM	0	150	0	150	24	187	0	211	0	132	1	0	133	494
8:15 AM	3	131	0	134	21	177	0	198	0	165	0	0	165	497
8:30 AM	3	130	0	133	33	191	0	224	0	161	1	0	162	519
8:45 AM	1	108	0	109	25	209	0	234	0	189	0	0	189	532
Hourly Total	7	519	0	526	103	764	0	867	0	647	2	0	649	2042
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	5	103	0	108	97	217	0	314	0	96	6	0	102	524
3:15 PM	8	117	0	125	131	156	0	287	0	84	9	0	93	505
3:30 PM	6	83	0	89	138	227	0	365	1	76	8	0	85	539
3:45 PM	2	87	1	89	119	182	0	301	0	76	7	0	83	473
Hourly Total	21	390	1	411	485	782	0	1267	1	332	30	0	363	2041
4:00 PM	2	122	0	124	126	152	0	278	0	96	7	0	103	505
4:15 PM	6	109	1	115	136	158	0	294	0	95	6	0	101	510
4:30 PM	6	96	1	102	143	174	0	317	0	78	2	0	80	499
4:45 PM	5	93	0	98	138	181	0	319	0	83	6	0	89	506
Hourly Total	19	420	2	439	543	665	0	1208	0	352	21	0	373	2020
5:00 PM	2	98	0	100	146	204	0	350	0	85	3	0	88	538
5:15 PM	4	113	0	117	121	159	0	280	0	92	2	0	94	491
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	6	211	0	217	267	363	0	630	0	177	5	0	182	1029
6:30 AM	0	82	0	82	11	115	0	126	0	185	0	0	185	393
6:45 AM	0	89	0	89	10	126	0	136	0	164	3	0	167	392
Hourly Total	0	171	0	171	21	241	0	262	0	349	3	0	352	785
7:00 AM	1	131	0	132	17	133	0	150	0	219	1	0	220	502
7:15 AM	3	168	0	171	32	158	0	190	0	182	3	0	185	546
7:30 AM	1	125	0	126	40	146	0	186	0	166	2	0	168	480
7:45 AM	1	123	0	124	30	165	0	195	0	138	0	0	138	457
Hourly Total	6	547	0	553	119	602	0	721	0	705	6	0	711	1985
8:00 AM	4	116	0	120	20	169	0	189	0	150	0	0	150	459
8:15 AM	1	125	0	126	28	158	0	186	0	133	2	0	135	447
Grand Total	116	4313	4	4429	3131	6537	0	9668	1	5030	228	0	5259	19356
Approach %	2.6	97.4	-	-	32.4	67.6	-	-	0.0	95.6	4.3	-	-	-
Total %	0.6	22.3	-	22.9	16.2	33.8	-	49.9	0.0	26.0	1.2	-	27.2	-
Lights	114	4214	-	4328	3083	6353	-	9436	1	4901	225	-	5127	18891
% Lights	98.3	97.7	-	97.7	98.5	97.2	-	97.6	100.0	97.4	98.7	-	97.5	97.6
Mediums	2	91	-	93	47	171	-	218	0	115	3	-	118	429
% Mediums	1.7	2.1	-	2.1	1.5	2.6	-	2.3	0.0	2.3	1.3	-	2.2	2.2
Articulated Trucks	0	8	-	8	1	13	-	14	0	14	0	-	14	36
% Articulated Trucks	0.0	0.2	-	0.2	0.0	0.2	-	0.1	0.0	0.3	0.0	-	0.3	0.2
All Pedestrians	-	-	4	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-

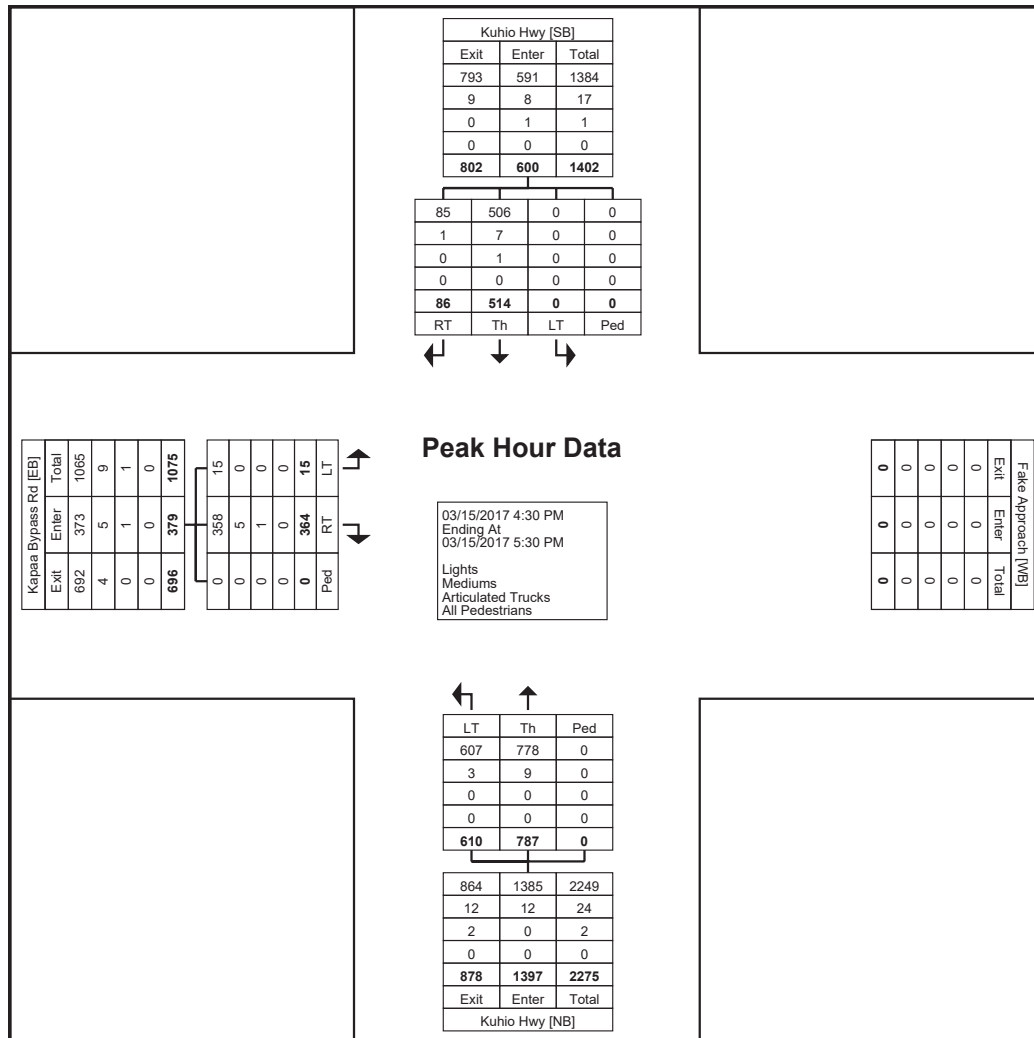




Turning Movement Data Plot

### Turning Movement Peak Hour Data (4:30 PM)

Start Time	Kapaa Bypass Rd Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
4:30 PM	6	90	0	96	166	187	0	353	0	112	19	0	131	580
4:45 PM	2	95	0	97	146	176	0	322	0	112	15	0	127	546
5:00 PM	5	88	0	93	149	232	0	381	0	138	27	0	165	639
5:15 PM	2	91	0	93	149	192	0	341	0	152	25	0	177	611
Total	15	364	0	379	610	787	0	1397	0	514	86	0	600	2376
Approach %	4.0	96.0	-	-	43.7	56.3	-	-	0.0	85.7	14.3	-	-	-
Total %	0.6	15.3	-	16.0	25.7	33.1	-	58.8	0.0	21.6	3.6	-	25.3	-
PHF	0.625	0.958	-	0.977	0.919	0.848	-	0.917	0.000	0.845	0.796	-	0.847	0.930
Lights	15	358	-	373	607	778	-	1385	0	506	85	-	591	2349
% Lights	100.0	98.4	-	98.4	99.5	98.9	-	99.1	-	98.4	98.8	-	98.5	98.9
Mediums	0	5	-	5	3	9	-	12	0	7	1	-	8	25
% Mediums	0.0	1.4	-	1.3	0.5	1.1	-	0.9	-	1.4	1.2	-	1.3	1.1
Articulated Trucks	0	1	-	1	0	0	-	0	0	1	0	-	1	2
% Articulated Trucks	0.0	0.3	-	0.3	0.0	0.0	-	0.0	-	0.2	0.0	-	0.2	0.1
All Pedestrians	-	-	0	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (4:30 PM)

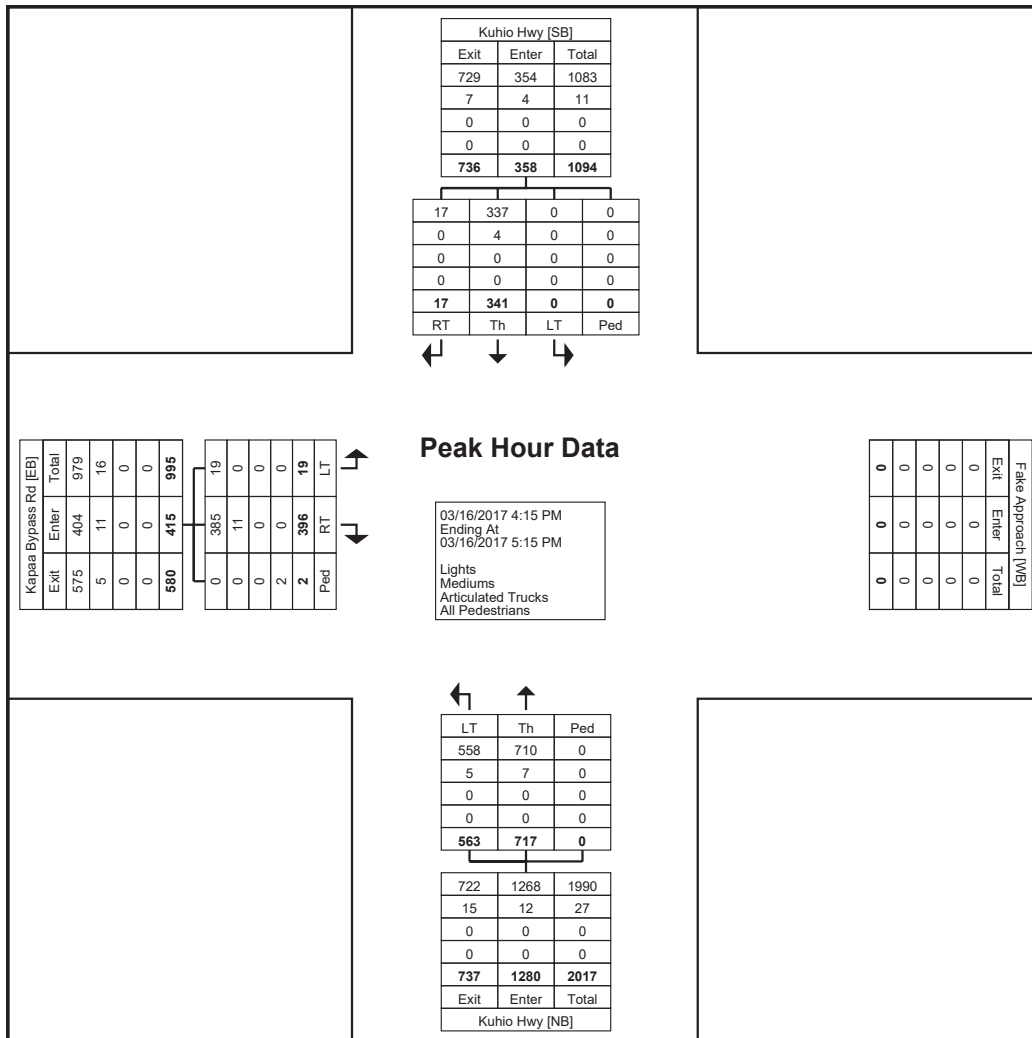
### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Kapaa Bypass Rd Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:00 AM	1	161	0	162	20	129	0	149	0	233	0	0	233	544
7:15 AM	1	184	0	185	25	155	0	180	0	200	1	0	201	566
7:30 AM	2	152	0	154	24	152	0	176	0	167	0	0	167	497
7:45 AM	1	155	1	156	33	180	0	213	0	135	0	0	135	504
Total	5	652	1	657	102	616	0	718	0	735	1	0	736	2111
Approach %	0.8	99.2	-	-	14.2	85.8	-	-	0.0	99.9	0.1	-	-	-
Total %	0.2	30.9	-	31.1	4.8	29.2	-	34.0	0.0	34.8	0.0	-	34.9	-
PHF	0.625	0.886	-	0.888	0.773	0.856	-	0.843	0.000	0.789	0.250	-	0.790	0.932
Lights	4	635	-	639	100	600	-	700	0	711	1	-	712	2051
% Lights	80.0	97.4	-	97.3	98.0	97.4	-	97.5	-	96.7	100.0	-	96.7	97.2
Mediums	1	14	-	15	2	16	-	18	0	23	0	-	23	56
% Mediums	20.0	2.1	-	2.3	2.0	2.6	-	2.5	-	3.1	0.0	-	3.1	2.7
Articulated Trucks	0	3	-	3	0	0	-	0	0	1	0	-	1	4
% Articulated Trucks	0.0	0.5	-	0.5	0.0	0.0	-	0.0	-	0.1	0.0	-	0.1	0.2
All Pedestrians	-	-	1	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-



### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Kapaa Bypass Rd Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
4:15 PM	6	109	1	115	136	158	0	294	0	95	6	0	101	510
4:30 PM	6	96	1	102	143	174	0	317	0	78	2	0	80	499
4:45 PM	5	93	0	98	138	181	0	319	0	83	6	0	89	506
5:00 PM	2	98	0	100	146	204	0	350	0	85	3	0	88	538
Total	19	396	2	415	563	717	0	1280	0	341	17	0	358	2053
Approach %	4.6	95.4	-	-	44.0	56.0	-	-	0.0	95.3	4.7	-	-	-
Total %	0.9	19.3	-	20.2	27.4	34.9	-	62.3	0.0	16.6	0.8	-	17.4	-
PHF	0.792	0.908	-	0.902	0.964	0.879	-	0.914	0.000	0.897	0.708	-	0.886	0.954
Lights	19	385	-	404	558	710	-	1268	0	337	17	-	354	2026
% Lights	100.0	97.2	-	97.3	99.1	99.0	-	99.1	-	98.8	100.0	-	98.9	98.7
Mediums	0	11	-	11	5	7	-	12	0	4	0	-	4	27
% Mediums	0.0	2.8	-	2.7	0.9	1.0	-	0.9	-	1.2	0.0	-	1.1	1.3
Articulated Trucks	0	0	-	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
All Pedestrians	-	-	2	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-

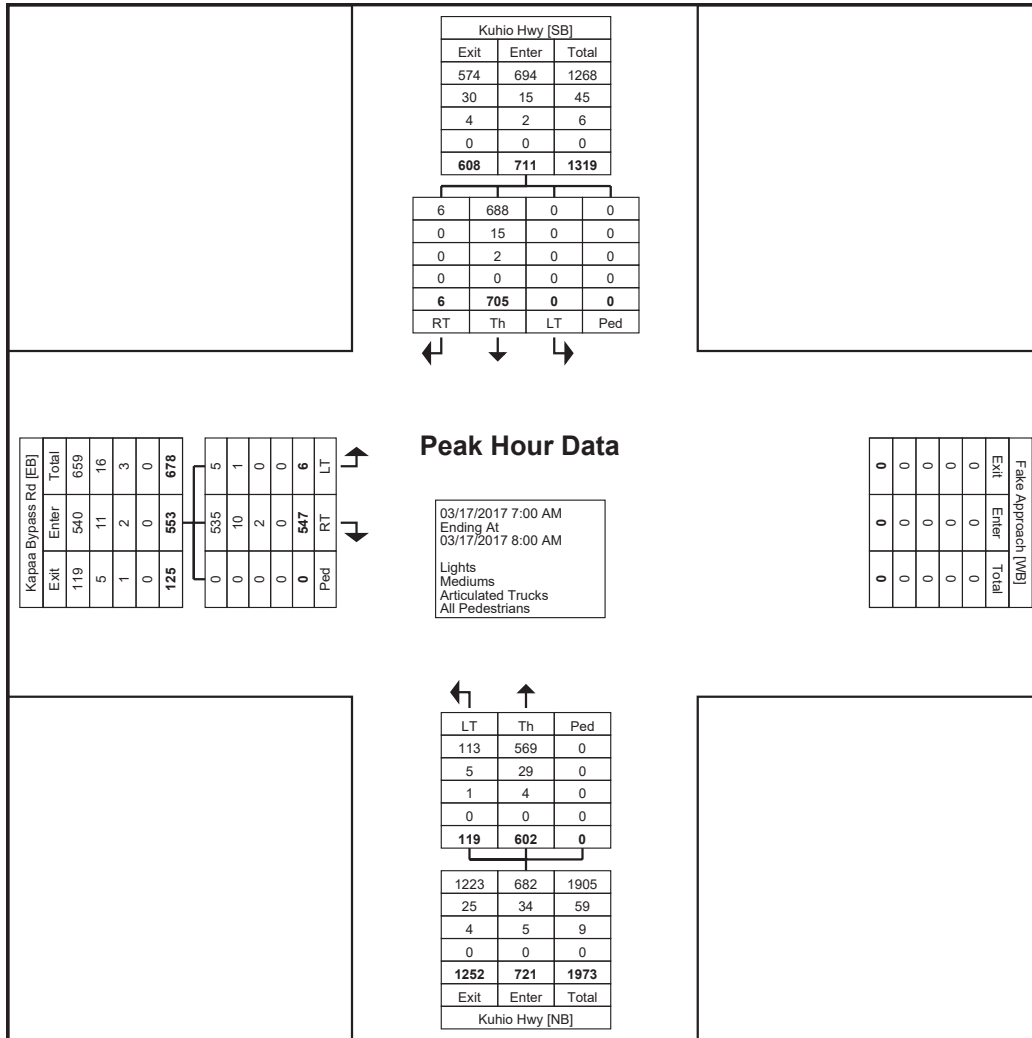


Turning Movement Peak Hour Data Plot (4:15 PM)

### Turning Movement Peak Hour Data (7:00 AM)

Start Time	Kapaa Bypass Rd Eastbound				Kuhio Hwy Northbound				Kuhio Hwy Southbound					Int. Total
	Left-Turn	Right-Turn	Peds	App. Total	Left-Turn	Thru	Peds	App. Total	Left-Turn	Thru	Right-Turn	Peds	App. Total	
7:00 AM	1	131	0	132	17	133	0	150	0	219	1	0	220	502
7:15 AM	3	168	0	171	32	158	0	190	0	182	3	0	185	546
7:30 AM	1	125	0	126	40	146	0	186	0	166	2	0	168	480
7:45 AM	1	123	0	124	30	165	0	195	0	138	0	0	138	457
Total	6	547	0	553	119	602	0	721	0	705	6	0	711	1985
Approach %	1.1	98.9	-	-	16.5	83.5	-	-	0.0	99.2	0.8	-	-	-
Total %	0.3	27.6	-	27.9	6.0	30.3	-	36.3	0.0	35.5	0.3	-	35.8	-
PHF	0.500	0.814	-	0.808	0.744	0.912	-	0.924	0.000	0.805	0.500	-	0.808	0.909
Lights	5	535	-	540	113	569	-	682	0	688	6	-	694	1916
% Lights	83.3	97.8	-	97.6	95.0	94.5	-	94.6	-	97.6	100.0	-	97.6	96.5
Mediums	1	10	-	11	5	29	-	34	0	15	0	-	15	60
% Mediums	16.7	1.8	-	2.0	4.2	4.8	-	4.7	-	2.1	0.0	-	2.1	3.0
Articulated Trucks	0	2	-	2	1	4	-	5	0	2	0	-	2	9
% Articulated Trucks	0.0	0.4	-	0.4	0.8	0.7	-	0.7	-	0.3	0.0	-	0.3	0.5
All Pedestrians	-	-	0	-	-	-	0	-	-	-	-	0	-	-
% All Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-





Turning Movement Peak Hour Data Plot (7:00 AM)